SUBDIVISION AND DEVELOPMENT APPEAL BOARD FOR ROCKY VIEW COUNTY

File:	1 - PRDP20240118 Bird	
Appellant(s)	lichard and Cathryn Bird	
Applicant(s)	Dean Thomas Design Group (Ryland Cook)	
Owner(s)	John & Claudine Lang-Hodge	

Exhibit	Description	Pages
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1 - PRDP20223318

Exhibit 1 - Notice of Appeal

NOTICE OF APPEAL

Subdivision & Development Appeal Board for Rocky View County

Enforcement Appeal Committee for Rocky View County

LEGISLATIVE AND

INTERGOVERNMENTAL

Page 1 of 2

Name of Appellant				
Bird Rich	ard & Cath	r val		
Appellant Property Address 7 Clear Monstain R.		Municipality Rocky View Const	Province	Postal Code 732-359
Mailing Address (if different than above)		Municipality	Province	Postal Code
Primary Phone # Altern	nate Phone #	Email Address		
PROPERTY UNDER APPEAL				
Address 242253 Westbla	ss Road	Legal Land Description (Lot, Block, Pla Lot 4, Block 2 Development Permit, Subdivision Ap		
04618044		Application Number		
I AM APPEALING THE DECISION I	SSUED BY			
Development Authority	Subdivision A	Authority 🗆 E	nforcement Se	rvices
which are situ development, inc development inclu clean fill. We o Modifies the ex height relative to	luding our les regradin bject to an icting Nature	primary reside. ug, excavation ny such develo al elevation of to the Nature	nce. The and place present the lo al conto	proposed ement of is it t, its

hearing. Your name, legal land description, street address, and reasons for appeal will be made available to the public in accordance with section 40(1)(c) of the FOIP Act. Your personal contact information, including your phone number and email address, may be redacted prior to your appeal being made available to the public. If you have questions regarding the collection, use or disclosure of this information, please contact a Legislative Officer at 403-230-1401.

7 Clear Mountain Rise SW Calgary, AB T3Z 3J9

March 7, 2024

Board Clerk c/o Legislative & Intergovernmental Services 262075 Rocky View Point Rocky View County AB T4A 0X2

Dear Sir:

Please find enclosed our Notice of Appeal to the approval of a development permit, Application Number PRDP20240118, of which we were just advised, together with the requisite cheque for \$250. The reasons for our appeal of and objection to this development are set out in the Notice form but essentially involve the anticipated adverse impacts to the natural rural character of our neighbourhood, to the natural beauty of the existing skyline, and to the value of our down slope home and properties, especially as a result of relaxing the minimum top-of-bank setback.

Yours truly,

J. Richard Bird

SUBDIVISION AND DEVELOPMENT APPEAL BOARD FOR ROCKY VIEW COUNTY

262075 Rocky View Point Rocky View County, Alberta T4A 0X2 403-230-1401 | sdab@rockyview.ca

NOTICE OF HEARING

Issued: March 18, 2024

An appeal has been filed with the Subdivision and Development Appeal Board for Rocky View County ("Board") against the Development Authority's decision to conditionally-approve a development permit application for the single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

INFORMATION ABOUT THE PROPERTY UNDER APPEAL	
File:	04618044; PRDP20240118
Location:	242253 Westbluff Road located approximately 1.61 km (1 mile) south of Springbank Road and on the west side of Westbluff Road
Appellant(s):	Richard and Cathryn Bird
Applicant(s):	Dean Thomas Design Group (Ryland Cook)
Owner(s):	John & Claudine Lang-Hodge

APPEAL HEARING INFORMATION

Further information about the appeal will be available in the Board's agenda six days before the hearing on www.rockyview.ca.

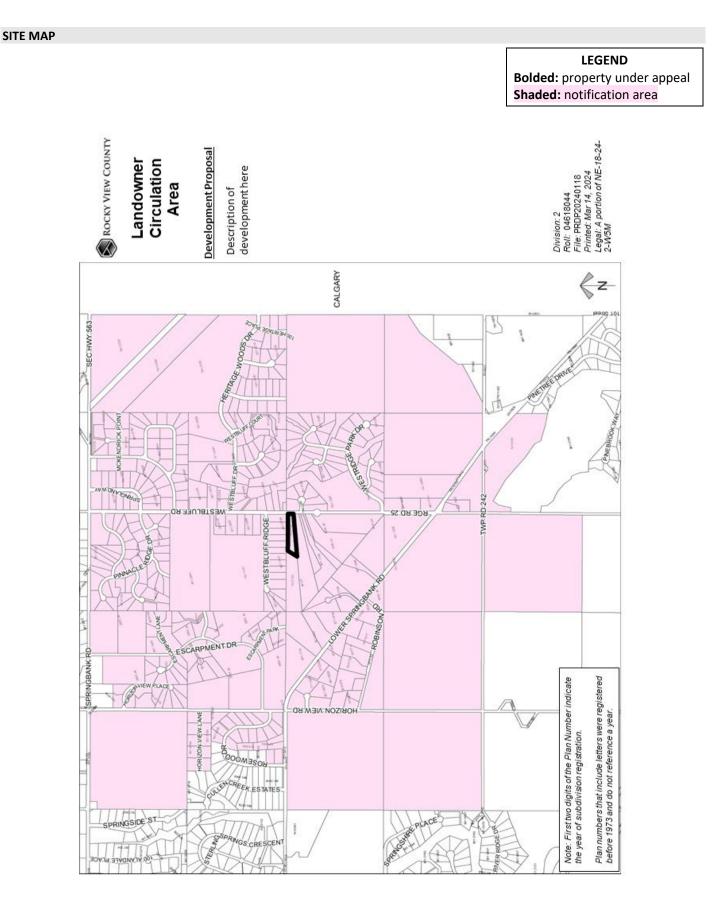
Date and time:	March 28, 2024 at or after 9:00 a.m.
Location:	Council Chambers - County Hall 262075 Rocky View Point, Rocky View County, Alberta T4A 0X2

HOW TO PARTICIPATE IN THE HEARING

If you feel you are affected by this appeal, you can provide a submission or present at the hearing as noted below.

Before the hearing:	 In your submission, clearly state how you are affected and include where you live in relation to the property under appeal. Submissions are due by 9:00 a.m. the last business day before the hearing. It is at the Board's discretion whether late submissions are accepted. Submissions can be provided by: email to sdab@rockyview.ca; or mail to the SDAB Clerk at 262075 Rocky View Point, Rocky View County, Alberta T4A 0X2
At the hearing:	Add your name to the sign-in sheet to present to the Board at the hearing

If you have questions about the development permit application, contact Planning Services at <u>development@rockyview.ca</u>. For inquiries about the hearing procedure, contact the Board clerk at <u>sdab@rockyview.ca</u>.



Submissions may be made available to the public on www.rockyview.ca in accordance with section 40(1)(c) of the *Freedom of Information and Protection of Privacy Act* (*'FOIP Act'*). Personal information contained in your submission is collected under section 33(c) of the *FOIP Act* for the purpose of public participation in the Board's decision-making process. Your name, legal land description, street address, and any opinions provided in your submission will be made available to the public and form part of the public record. Your personal contact information, including your phone number and email address, may be redacted prior to making your submission available to the public. If you have questions regarding the collection, use or disclosure of this information, please contact a Legislative Officer at 403-230-1401.



PLANNING

TO:	Subdivision and Development Appeal Board	
DATE:	March 28, 2024	DIVISION: 2
Roll #:	04618044	APPLICATION: PRDP20240118
SUBJECT:	Development Item – Single-lot Regrading, Excavation, and Placement of Clean Fill, with Variances	

PROPOSAL: Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement

LOCATION: Located approximately 1.61 kilometres (1.00 mile) south of Springbank Road and on the west side of Westbluff Road.

DECISION: Approval

DECISION DATE:	APPEAL DATE:	ADVERTISED DATE:
March 5, 2024	March 13, 2024	March 5, 2024

APPEAL:

Submitted by an adjacent landowner with respect to concerns surrounding building height, visual impacts, property values, and the natural physical landscape of the subject parcel and the neighbourhood.

'See attached exhibits'

ANALYSIS:

The application is for single-lot regrading, excavation, and placement of clean fill, to accommodate the construction of a new Dwelling, Single Detached along with other associated site improvements.

Dwelling, Single Detached is listed as a permitted use under the Residential, Rural District, and is exempt from requiring a Development Permit as per Section 92 J) of *Land Use Bylaw C-8000-2020* (LUB), unless relaxations are requested. The relaxation to the top-of-bank setback requirement, and the additional single-lot regrading, excavation, and placement of fill beyond the allowable parameters of Section 92 v) of the LUB, are seen as essential to the construction of the dwelling. Subsequent technical reports shall be required as prior-to-release conditions to demonstrate the suitability of the proposed development.

Due to the thorough review process undertaken to conditionally approve the application, and the required technical reports which will be required prior to release of the development permit, it is Administration's position that the proposed development would not unduly interfere with the amenities of the neighbourhood, or materially interfere with or affect the use, enjoyment, or value of neighbouring parcels of land.

Respectfully submitted,

Justin Rebello



PLANNING

TO:	Subdivision and Development Appeal Board	
DATE:	March 28, 2024	DIVISION: 2
Roll #:	04618044	APPLICATION: PRDP20240118
SUBJECT:	Staff Report – Single-lot Regrading, Excavation, and Placement of Clean Fill, with Variances	

EXECUTIVE SUMMARY:

The application is for Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

The proposed regrading, excavation, and placement of clean fill is to accommodate the foundation of the dwelling. Due to the topography of the subject parcel in combination with the selected location of the proposed dwelling, the application is requesting a relaxation to the minimum top-of-bank setback requirement. The relaxations are deemed as necessary to accommodate the construction of the proposed dwelling. The Applicant/Owner chose the location of the proposed dwelling to effectively manage stormwater drainage given the size of the home. The dwelling meets all minimum setback requirements and maximum building height requirements of the Residential, Rural District.

The proposed development meets the definition of a Dwelling, Single Detached and is a listed use under the Residential, Rural District. Due to the thorough review process undertaken to conditionally approve the application, and the required technical reports which will be required prior to release of the development permit, it is Administration's position that the proposed development would not unduly interfere with the amenities of the neighbourhood, or materially interfere with or affect the use, enjoyment, or value of neighbouring parcels of land.

ADMINISTRATION DECISION:

Approval, subject to conditions.

OVERVIEW:

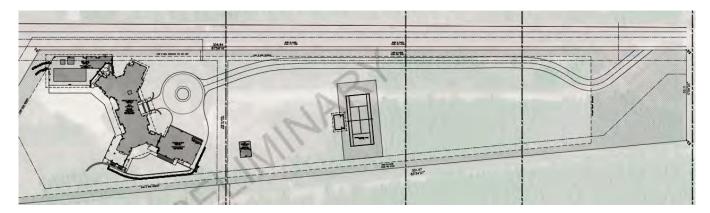
Applicant	Dean Thomas Design Group (Ryland Cook)	
Landowner	Lang-Hodge, John & Claudine	
Subject Site(s)	242253 WESTBLUFF ROAD	
Site Area	1.90 hectares (4.69 acres)	
Proposal	Single-lot regrading, excavation, and placement of clean fill, with variance to minimum top-of-bank setback requirement	
Surrounding Uses	Residential	
Applicable Regulations	Land Use Bylaw C-8000-2020, Municipal Development Plan, Central Springbank Area Structure Plan, County Servicing Standards	



Figure 1 – Site Location (Regional Context)



Figure 2 – Site Plan (Intended Use Areas)



POLICY/LAND USE BYLAW REVIEW:

Central Springbank Area Structure Plan (ASP):

As per Map 2 – Current Land Use of the ASP, subject parcel is located within the Residential Use area.

As per Map 11 – Infill Residential, subject parcel is located with the Conceptual Plan Boundary.

2.3) Physical Development Guidelines

• As the submitted application has taken site-specific conditions into consideration, and the design and appearance of the proposed dwelling appear cohesive with adjacent dwellings, it is Administration's position that the subject application is consistent with the Physical Development Guidelines of the ASP.



2.8) Utilities

Potable water is to be provided via the local water co-op, and wastewater is to be serviced via an
on-site Private Sewage Treatment System (PSTS). Should potable water capacity not be available
via the water co-op, potable water is to be provided via a groundwater well. Therefore, it is in the
opinion of Administration that the subject application is consistent with the Utilities Policies of the
ASP.

2.9) Residential Development

• Subject application is for a Dwelling, Single Detached, does not pose a negative visual impact on adjacent lands, does not obstruct existing viewsheds, and takes into account the natural topography into consideration. Therefore, it is Administration's position that the subject application is consistent with the Residential Development Policies of the ASP.

Municipal Development Plan (MDP):

Country Residential Communities.

- 10.1) Development within Greater Bragg Creek, Bearspaw, North and Central Springbank, Elbow Valley, Balzac East (Sharp Hills/Butte Hills), Cochrane North, and Glenbow Ranch shall conform to their relevant area structure plan.
 - Subject application is consistent with the Policies of the Central Springbank ASP, therefore the proposal is consistent with Section 10.1 of the MDP.

Land Use Bylaw C-8000-2020 (LUB):

"Dwelling, Single Detached" means a dwelling which is supported on a permanent foundation or basement and has a minimum GFA of 37.10 sq. m (399.34 sq. ft.).

• Subject dwelling is to be constructed on a basement foundation and shall meet the minimum GFA of 37.10 sq. m (399.34 sq. ft.).

R-RUR Residential, Rural District

317) PURPOSE: To provide for residential uses in a rural setting on parcels which can accommodate limited agricultural pursuits.

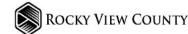
318) PERMITTED USES: Dwelling, Single Detached

320) MAXIMUM DENSITY:

- a) A maximum of two Dwelling Units one Dwelling, Single Detached and one other Dwelling Unit where the other Dwelling Unit is not a Dwelling, Single Detached.
 - Subject dwelling meets the definition of Dwelling, Single Detached and shall serve as the principal dwelling on the subject parcel. The proposed dwelling does not contain an Accessory Dwelling Unit (secondary suite). No concerns in respect to maximum density.

321) MAXIMUM BUILDING HEIGHT:

- b) All others: 12.0 m (39.37 ft.)
 - Maximum: 12.00 m (39.37 ft.)
 - Proposed (with pool house height included): 10.38 m (34.06 ft.)
 - Proposed (without pool house height included): 11.74 m (38.52 ft.)



323) MINIMUM SETBACKS:

- Front yard setback requirement: 45.00 m (147.64 ft.)
- Proposed front yard setback: Lots
- Side yard setback requirement (S1): 3.00 m (9.84 ft.)
- Proposed side yard setback (S1): 3.02 m (9.91 ft.)
- Side yard setback requirement (S2): 3.00 m (9.84 ft.)
- Proposed side yard setback (S2): 10.58 m (34.71 ft.)
- Rear yard setback requirement: 7.00 m (22.97 ft.)
- Proposed rear yard setback: 7.60 m (24.93 ft.)

DEVELOPMENT PERMITS NOT REQUIRED

- 92) A Development Permit is not required for the following development, provided it complies with all applicable provisions of the Bylaw, and does not require a variance:
- v) Stripping, Grading, Excavation and Fill

The placing of up to 1.00 m (3.28 ft.) of fill and topsoil adjacent to or within 15.00 m (49.21 ft.) of a building under construction that has a valid Building Permit, during the course of the construction to be used to establish approved final grades.

 Proposed placement of fill exceeds above allowable parameters, therefore Development Permit is required.

The excavation up to 2.00 m (6.56 ft.) adjacent to or within 15.00 m (49.21 ft.) of a building under construction that has a valid Building Permit, during the course of the construction to be used to establish approved final grades.

• Proposed excavation exceeds above allowable parameters, therefore Development Permit is required.

Section 190) The Development Authority may, at their discretion, reduce the setback requirements if the applicant provides a Geotechnical Study, prepared by a qualified engineer, that provides satisfactory proof of bank stability.

• Included as prior-to-release condition of approval.

County Servicing Standards:

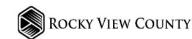
302) SLOPE STABILITY ASSESSMENT REPORT

The County requires a Slope Stability Assessment by a Geotechnical Engineer, for slopes 15% or greater, and greater than 2.00 meters in vertical height. These areas can be considered as part of the developable acre area if a Geotechnical Engineer can certify the stability of the slopes prior to, during and after development.

• Included as prior-to-release condition of approval.

305) DEEP FILL REPORTS

When the constructed depth of fill exceeds 1.2 meters a "Deep Fill" report is required. The report shall be completed by a Geotechnical Engineer that includes general recommendations for different types of building foundations, as well as include and summarize compaction testing of fill.



Potential fill areas must be identified as part of the development approval application. Following development approval, all deep-fill placements must have a record of compaction testing.

Fill for building foundations must be compacted to a minimum of 98% Standard Proctor Maximum Dry Density (SPMDD). Specifications for fill for roadway subgrades are addressed in Section 400.0. Areas outside of roadways or foundations must be compacted to a minimum of 95% SPMDD.

• Included as prior-to-release condition of approval.

704.2.5) Site-Specific Stormwater Implementation Plan (SSIP)

A SSIP is a drainage and servicing plan that is generally prepared in support of Development Permits or small residential subdivisions of less than 10 lots on a site-specific basis.

• Included as prior-to-release condition of approval.

CONSTRUCTION MANAGEMENT

1101) Stock Piling of Materials and Stripping and Grading

During site preparation and/or construction of roads and buildings, care shall be taken to mitigate potential impact from erosion and sedimentation. Prior to undertaking any site preparation, the developer of the works shall submit to the County a Construction Management Plan.

• Included as prior-to-release condition of approval.

VARIANCE SUMMARY:

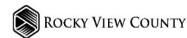
Variance	Requirement	Proposed	Percentage (%)
Section 189) Minimum Top-of-Bank setback requirement	20.00 m (65.62 ft.)	Varies: To a maximum of 0.00 m (0.00 ft.)	100%

DISCUSSION:

Based on the size, location, style of home, and context of the subject parcel, the proposed Dwelling, Single Detached requires grading, excavation, and placement of fill greater than the DP-exempt limits allowed in Section 90 v) of the Land Use Bylaw. Due to the topography of the subject parcel in combination with the selected location of the proposed dwelling, the application is requesting a relaxation to the minimum top-of-bank setback requirement. It is also to be noted that the proposed dwelling would be relatively parallel with other adjacent dwellings in the area, therefore form and massing are not of concern given the context of the area.

The applicant provided a Geotechnical Slope Assessment dated 2016 that was conducted on the subject parcel for a previous Development Permit Application that was not pursued by the then-owner of the parcel. Engineering Services reviewed the Assessment and determined that the Applicant/Owner shall submit a Geotechnical Memo confirming whether the Assessment remains applicable to the subject application or not, and if necessary, provide additional recommendations with respect to slope stability.

Based on a desktop review, no environmentally sensitive features were observed on the subject parcel. A joint review was conducted with Building Services who determined that based on the submitted floor plans, the basement of the dwelling does not meet the definition of an Accessory Dwelling Unit (secondary suite), therefore it was not included as part of this approval.



The subject parcel is accessed off a mutual driveway which has an Access Easement Agreement registered on title under Instrument # 151 190 264 and corresponding survey plan # 151 2153. This aspect shall also be addressed via a permanent condition of approval to avoid potential future access issues/disputes.

Potable water is to be provided via piped service from Westridge Water Co-op, as per condition #4 of subdivision file # 2009-RV-061. A letter was provided by the then-applicant's engineer stating that connection infrastructure would be installed. Sewage is to be serviced via a packaged PSTS and enforced via the existing SISA registered on title. Servicing is to be addressed via prior-to-release and permanent conditions of approval.

A site inspection was conducted by the File Manager on March 15, 2024. No immediate issues/concerns were noted. No construction had commenced at the time of inspection. The file manager observed the existing road approach the mutual driveway, and the existing stormwater infrastructure (i.e. culvert). The topography of the land looked to be consistent with the submitted site plan, which illustrated sloping from the east steeply down towards the west. The parcel appears to be well screened from the south via existing mature trees, and adjacent dwellings are constructed relatively parallel to the building site of the proposed dwelling.

Respectfully submitted,

Dominic Kazmierczak

Concurrence,

Matthew Boscariol

Manager Planning Executive Director Community Development Services

ATTACHMENTS:

ATTACHMENT 'A': Development Permit Report Conditions ATTACHMENT 'B': Application Information



ATTACHMENT 'A': DEVELOPMENT PERMIT REPORT CONDITIONS

Description:

- That the construction of a Dwelling, Single Detached, may commence on the subject lands, in accordance with the approved site plan and drawings, as prepared by Dean Thomas Design Group, Project Name: 242253 Westbluff RD, Rocky View County, AB, Dwgs: A0.1 – A8.4, dated February 2, 2024, as amended, and conditions of approval including:
 - i. Single-lot regrading, excavation, and placement of clean fill for the Dwelling, Single Detached, in accordance with the approved site plan and drawings;
 - ii. Single-lot regrading, excavation, and placement of clean fill for the attached pool house and attached garage, in accordance with the approved site plan and drawings;
 - iii. Single-lot regrading, excavation, and placement of clean fill for the personal use tennis court, in accordance with the approved site plan and drawings;
 - *iv.* That the minimum top-of-bank setback requirement for the Dwelling, Single Detached, attached pool house and attached garage shall be relaxed *in accordance with the approved site plan and required technical studies.*

Prior to Release:

- 2. That prior to release of this permit, the Applicant/Owner shall submit a Geotechnical Memo, prepared by a qualified professional, confirming that the *Geotechnical Slope Assessment* prepared by Parkland Geo, dated August 10, 2016, Project No. CA0241-01 adequately proves bank stability for the proposed Dwelling, Single Detached, in accordance with Section 190 of the County's *Land Use Bylaw C-8000-2020* (LUB) and the County's Servicing Standards. The Geotechnical Memo shall:
 - i. Confirm that the Assessment adequately addresses slope stability, sewage disposal, water table levels, construction materials for roads, water servicing, stormwater drainage and any other relevant developmental constraints.
 - ii. Provide any additional recommendations for slope stability including registration of any required easements and/or restrictive covenants, if deemed necessary by the Development Authority.
- 3. That prior to release of this permit, the Applicant/Owner shall submit a Deep Fills Report, prepared by a qualified professional, in accordance with the County's Servicing Standards, for all placed areas of clean fill greater than 1.20 m (3.93 ft.) in depth.
- 4. That prior to release of this permit, the Applicant/Owner shall submit a limited scope Site-Specific Stormwater Implementation Plan (SSIP) prepared by a qualified professional, in accordance with Springbank Drainage Strategies and the County's Servicing Standards. The SSIP shall include:
 - i. A grading plan that illustrates the original ground profile; the depth of proposed fill; the total amount of soil to be imported/exported from the site; and analysis of the pre- and post-construction grades to determine whether there are any impacts to adjacent properties or the public road network.
 - ii. Confirmation of pre- and post-construction conditions associated with site stormwater storage, unit area site releases, volume control target, and offsite drainage.
 - iii. Recommendations for Erosion and Sediment Control (ESC) mitigation measures.

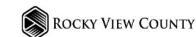


- 5. That prior to release of this permit, the Applicant/Owner shall submit a Construction Management Plan (CMP) addressing noise mitigation measures, traffic accommodation, sedimentation and dust control, erosion and weed control, construction practices, waste management, hazardous material containment and all other relevant construction management details.
- 6. That prior to release of this permit, the Applicant/Owner shall submit written confirmation of capacity availability from Westridge Water Utilities for piped water services for the subject development, in accordance with the approved subdivision Transmittal of Decision 2009-RV-061, Section 2.8.2 of the *Central Springbank Area Structure Plan*, and the County's Servicing Standards.
 - i. That if capacity remains available via Westridge Water Utilities, the subject lands shall connect to the piped water supply with confirmation/documentation provided to the satisfaction of the Development Authority; and
 - ii. That if capacity is not available via Westridge Water Utilities, the Applicant/Owner shall propose an acceptable alternative water supply for the subject development, to the satisfaction of the Development Authority.
- 7. That prior to release of this permit, the Applicant/Owner shall contact County Road Operations with haul details for materials and equipment needed during construction/site development to confirm if Road Use Agreements or permits shall be required for any hauling along the County road system and to confirm the presence of County road ban restrictions.
 - i. The Applicant/Owner shall also discuss any requirements or improvements that may be required for the approach of Westbluff Road. If required, a New Road Approach application shall be submitted to County Road Operations
 - ii. Written confirmation shall be received from County Road Operations confirming the status of this condition. Any required agreement or permits shall be obtained unless otherwise noted by County Road Operations.

Permanent:

- 8. That if the prior to release conditions have not been met by **September 30, 2024**, or the approved extension date, then this approval is null and void and the Development Permit shall not be issued.
- 9. That any plan, technical submission, agreement, matter, or understanding submitted and approved as part of the application, in response to a Prior to Release condition, including the required Geotechnical Report, Deep Fills Report, SSIP, and CMP, shall be implemented, and adhered to in perpetuity and also includes:
 - i. The Development Agreement for Site Improvements/Services Agreement (SISA), as registered on title, Instrument No. 151 190 262, as agreed upon between the landowner(s) and Rocky View County.
- 10. That the Applicant/Owner shall submit compaction testing to the County, verifying that the fill areas greater than 1.20 m. (3.93 ft.) in depth were placed in accordance with the overlying technical accepted by the County.
- 11. That the dwelling unit shall not be used as a *Vacation Rental* or for commercial purposes at any time, unless approved by a Development Permit.
- 12. That the pool house and tennis court shall not be used for commercial purposes at any time unless approved by a Development Permit.
- 13. That this approval does not include an Accessory Dwelling Unit.





- 14. That there shall be a minimum of two (2) dedicated on-site parking stalls for the subject dwelling unit at all times.
- 15. That the Applicant/Owner shall take whatever means necessary to prevent visible dust associated with the development from escaping the site and having adverse effects on adjacent roadways and properties.
- 16. That no topsoil shall be removed from the site. All topsoil shall be retained on-site and shall be respread on-site and seeded to grass or landscaped after building construction is complete, as part of site restoration.
- 17. That access to the subject parcel shall be via the existing mutual approach and driveway, as shown on the approved site plan and drawings.
 - i. That the existing Access Easement (Instrument #151 190 264) shall remain registered on title, and shall not be discharged from title, unless an alternative physical and legal access acceptable to the County, has been approved for the subject parcel.
- 18. That the Applicant/Owner shall be responsible for rectifying any adverse effect on adjacent lands and access/driveway area from drainage alteration, including stormwater implications from the proposed development. Post-development drainage shall not exceed pre-development drainage.
 - i. That any lot regrading and placement of clean fill shall not direct any additional overland surface drainage nor negatively impact existing drainage patterns in the County's road right-of-way of Westbluff Road.
- 19. That all on-site lighting, including private, site security and parking area lighting, shall be designed to conserve energy, reduce glare, and reduce uplight, in accordance with Sections 225 227 of the County's *Land Use Bylaw C-8000-2020*. All lighting shall be full cut-off (shielded) and be located and arranged so that no direct rays of light are directed at any adjoining properties, that may interfere with the use and enjoyment of neighbouring lands or interfere with the effectiveness of any traffic control devices or the vision/safety of motorists.
- 20. That if the development authorized by this Development Permit is not commenced with reasonable diligence within twelve (12) months from the date of issue and completed within twenty-four (24) months of the issue, the permit is deemed to be null and void, unless an extension to this permit shall first have been granted by the Development Officer.

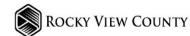
Advisory:

- That the Applicant/Owner shall obtain a Building Permit and any applicable sub-trade permits through the County's Building Services department, prior to any construction taking place, using the appropriate checklists and application forms. Compliance with the *National Energy Code* is also required.
- That the subject development shall conform to the County's *Noise Control Bylaw C-8067-2020* and *Road Use Agreement Bylaw C-8323-2022*, in perpetuity.
- That the site shall remain free of Regulated, Prohibited Noxious or Noxious Weeds and the site shall be maintained in accordance with the *Alberta Weed Control Act* [Statutes of Alberta, 2008 Chapter W-5.1, November 16, 2022].
- That there shall be adequate water & sanitary sewer servicing provided for the subject dwelling unit.





- That it is the Applicant/Owner's responsibility to obtain and display a distinct municipal address in accordance with the County's *Municipal Addressing Bylaw (Bylaw C-7562-2016)*, for the subject dwelling unit, to facilitate accurate emergency response. The municipal address for the subject dwelling unit is 242253 WESTBLUFF ROAD.
- That during construction, all construction and building materials shall be maintained on-site in a neat and orderly manner. Any debris or garbage shall be stored/placed in garbage bins and disposed of at an approved disposal facility.
- That any other federal, provincial, or County permits, approvals, and/or compliances, are the sole responsibility of the Applicant/Owner.

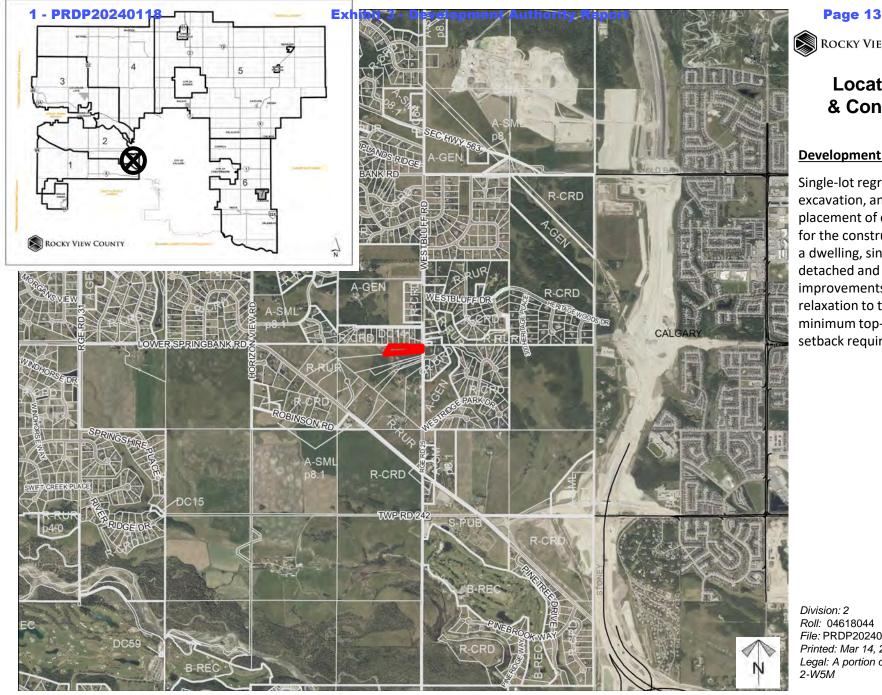


ATTACHMENT 'B': APPLICATION INFORMATION

APPLICANT: Dean Thomas Design Group (Ryland Cook)		OWNER: Lang-Hodge, John & Claudine
DATE APPLICATION RECEIVED: January 1, 2024		DATE DEEMED COMPLETE: January 22, 2024
GROSS AREA: 1.90 hectares (4.70 acres)		LEGAL DESCRIPTION: Lot 4, Block 2, Plan 1512150; NE-18-24-02-05
APPEAL BOARD	: Subdivision and Development	Appeal Board
HISTORY:		
August 10, 2017.	August 10, 2017: Development Permit PRDP20164836 issued for the construction of an Accessory Building (was not constructed).	
August 9, 2017:	Building Permit PRBD20150582 issued for the construction of a Dwelling, Single Detached (was not constructed).	
June 20, 2016:	Development Permit PRDP20152342 issued for the construction of a Dwelling, Single Detached (was not constructed).	
April 28, 2015:	Boundary Adjustment Application PL20140166 to adjust the boundaries between a \pm 5.70 hectare (14.09 acre) parcel and a \pm 1.78 hectare (4.4 acre) parcel in order to create a \pm 1.90 hectare (4.70 acre) parcel and a \pm 5.58 hectare (13.78 acre) parcel approved by the Subdivision Authority.	
July 27, 2010:	Subdivision Application 2009-RV-061 to create a +/- 1.78 hectare (+/- 4.41 acre) parcel with a +/- 5.71 hectare (+/- 14.10 acre) remainder approved by the Subdivision Authority.	

The application was circulated to a number of internal and external agencies and, where appropriate, conditions of approval have been proposed based on these comments.

At the time this report was prepared, no letters of support nor opposition were received from adjacent landowners, excepting the appeal. It is to be noted that one (1) letter of concern in respect to stormwater management was received and has been included in the agenda package for the Board's review.



File: PRDP20240118 Printed: Mar 14, 2024 Legal: A portion of NE-18-24-

Page 13 of 117

ROCKY VIEW COUNTY

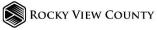
Location & Context

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

Development Authority Report

Page 14 of 117



Site Aerial

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

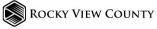




-Exhibit 3--Development-Authority-Report-

-

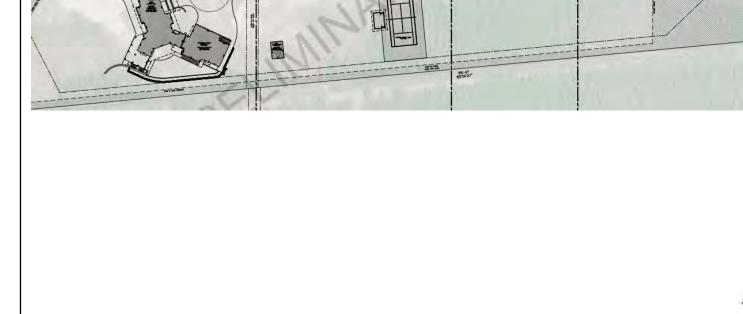
Page 15 of 117



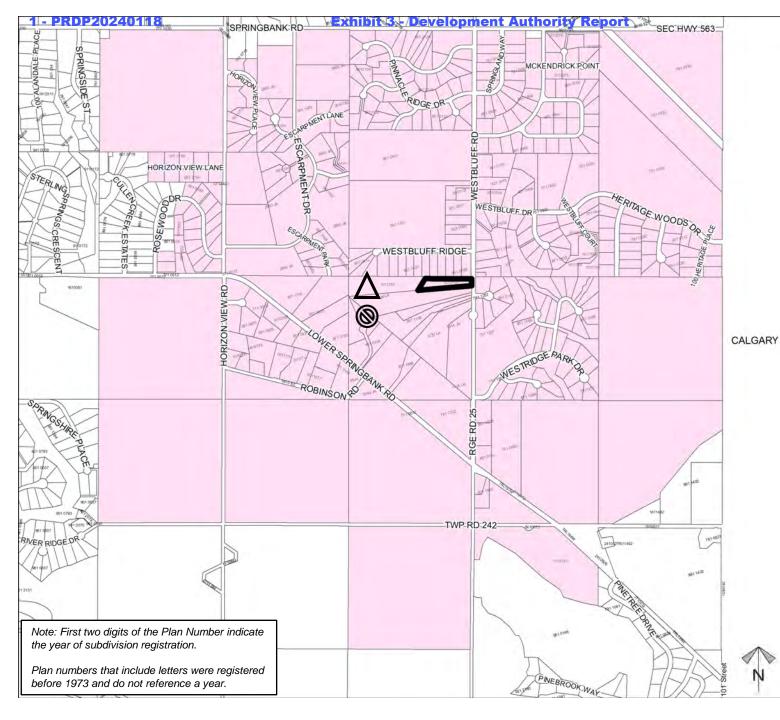
Site Plan

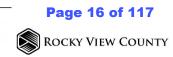
Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.



-





Landowner Circulation Area

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

Legend

Support



Not Support



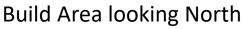
Concern



1---PRDP20240118-

Exhibit 3--Development Authority Report

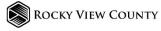




2021 02 - 5.14 23

Build Area looking South





Site Photos

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.



Bottom of Slope looking East

Build Area looking West

Exhibit 3--Development Authority-Report-

1---PRDP20240118-



From Westbluff Road Looking West



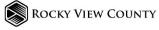
Adjacent Dwelling located West



Bottom of Slope looking West

Adjacent Stormwater Culvert

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Site Photos

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.

1 - PRDP20240118

Exhibit 3 - Development Authority Report

NOTICE OF APPEAL

Subdivision & Development Appeal Board for Rocky View County

Enforcement Appeal Committee for Rocky View County

Page 19 of 117

APPELLANT INFORMATION				
Name of Appellant Bird Richar	1 + Cath	e val		
Appellant Property Address 7 Clear Monstain Rise		Municipality Rocky View Conati	Province	Postal Code 732-359
Mailing Address (if different than above)		Municipality	Province	Postal Code
Primary Phone # Alternate	Phone #	Email Address		
PROPERTY UNDER APPEAL				
Address 242253 Westblags	F Road	Legal Land Description (Lot, Block, P Lot 4 Block 7		
Property Roll # 04618044		Lot 4, Block 2 Development Permit, Subdivision A Application Numbe		
AM APPEALING THE DECISION ISSU	JED BY			
Development Authority	Subdivision	Authority 🛛 I	Enforcement Se	rvices
REASONS FOR APPEAL (include as much of	detail as possible as to why	you are appealing the decision, attach	a separate page if req	uired)
We are the owner which are situan development, include clean sill. We ob Modifies the exis height relative to a land or existing would detract free impair the Nature properties, and a development would a bank setback, which exists to prevent and there is no jus	ted down ding our s regradi ject to a ting Natur djacent lo drainage	slope, below primary reside Ng, excavation Ny such develo al elevation of ts, the Natur patterns. Any s	the proposed the proposed the proposed the place of the local place of the local conton and the proposed the local conton and the proposed the place of the proposed the proposed the place of the place	posed proposed ement of is it t, its ups of the sicutions v Neighborhou

for Rocky View County and will be used to process your appeal and create a public record of the appeal hearing. Your name, legal land description, street address, and reasons for appeal will be made available to the public in accordance with section 40(1)(c) of the *FOIP Act*. Your personal contact information, including your phone number and email address, may be redacted prior to your appeal being made available to the public. If you have questions regarding the collection, use or disclosure of this information, please contact a Legislative Officer at 403-230-1401.

MAR 1 3 2024

7 Clear Mountain Rise SW Calgary, AB T3Z 3J9

March 7, 2024

Board Clerk c/o Legislative & Intergovernmental Services 262075 Rocky View Point Rocky View County AB T4A 0X2

Dear Sir:

Please find enclosed our Notice of Appeal to the approval of a development permit, Application Number PRDP20240118, of which we were just advised, together with the requisite cheque for \$250. The reasons for our appeal of and objection to this development are set out in the Notice form but essentially involve the anticipated adverse impacts to the natural rural character of our neighbourhood, to the natural beauty of the existing skyline, and to the value of our down slope home and properties, especially as a result of relaxing the minimum top-of-bank setback.

Yours truly,

J. Richard Bird



> 403-230-1401 questions@rockyview.ca www.rockyview.ca

THIS IS NOT A DEVELOPMENT PERMIT

Please note that the appeal period *must* end before this permit can be issued and that any Prior to Release conditions (if listed) *must* be completed.

NOTICE OF DECISION

Dean Thomas Design Group (Ryland Cook)

Page 1 of 4

Tuesday, March 5, 2024

Roll: 04618044

RE: Development Permit #PRDP20240118

Lot 4, Block 2, Plan 1512150, NE-18-24-02-05; (242253 WESTBLUFF ROAD)

The Development Permit application for single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-ofbank setback requirement has been **conditionally-approved** by the Development Officer subject to the listed conditions below (**PLEASE READ ALL CONDITIONS**):

Description:

- 1. That the construction of a Dwelling, Single Detached, may commence on the subject lands, in accordance with the approved site plan and drawings, as prepared by Dean Thomas Design Group, Project Name: 242253 Westbluff RD, Rocky View County, AB, Dwgs: A0.1 A8.4, dated February 2, 2024, as amended, and conditions of approval including:
 - i. Single-lot regrading, excavation, and placement of clean fill for the Dwelling, Single Detached, in accordance with the approved site plan and drawings;
 - ii. Single-lot regrading, excavation, and placement of clean fill for the attached pool house and attached garage, in accordance with the approved site plan and drawings;
 - iii. Single-lot regrading, excavation, and placement of clean fill for the personal use tennis court, in accordance with the approved site plan and drawings;
 - iv. That the minimum top-of-bank setback requirement for the Dwelling, Single Detached, attached pool house and attached garage shall be relaxed *in accordance with the approved site plan and required technical studies.*

Prior to Release:

- That prior to release of this permit, the Applicant/Owner shall submit a Geotechnical Report including Slope Stability Analysis, prepared by a qualified professional, in accordance with Section 190 of the County's *Land Use Bylaw C-8000-2020* (LUB) and the County's Servicing Standards, to prove bank stability for the proposed Dwelling, Single Detached. The Geotechnical Report shall address:
 - i. Slope stability, sewage disposal, water table levels, construction materials for roads, water servicing, stormwater drainage and any other relevant developmental constraints.
 - ii. Recommendations for slope stability including registration of any required easements and/or restrictive covenants.



> 403-230-1401 questions@rockyview.ca www.rockyview.ca

Dean Thomas Design Group (Ryland Cook) **#PRDP20240118** Page 2 of 4

- 3. That prior to release of this permit, the Applicant/Owner shall submit a Deep Fills Report, prepared by a qualified professional, in accordance with the County's Servicing Standards, for all placed areas of clean fill greater than 1.20 m (3.93 ft.) in depth.
- 4. That prior to release of this permit, the Applicant/Owner shall submit a limited scope Site-Specific Stormwater Implementation Plan (SSIP) prepared by a qualified professional, in accordance with Springbank Drainage Strategies and the County's Servicing Standards. The SSIP shall include:
 - i. A grading plan that illustrates the original ground profile; the depth of proposed fill; the total amount of soil to be imported/exported from the site; and analysis of the pre- and post-construction grades to determine whether there are any impacts to adjacent properties or the public road network.
 - ii. Confirmation of pre- and post-construction conditions associated with site stormwater storage, unit area site releases, volume control target, and offsite drainage.
 - iii. Recommendations for Erosion and Sediment Control (ESC) mitigation measures.
- 5. That prior to release of this permit, the Applicant/Owner shall submit a Construction Management Plan (CMP) addressing noise mitigation measures, traffic accommodation, sedimentation and dust control, erosion and weed control, construction practices, waste management, hazardous material containment and all other relevant construction management details.
- 6. That prior to release of this permit, the Applicant/Owner shall submit written confirmation of capacity availability from Westridge Water Utilities for piped water services for the subject development, in accordance with the approved subdivision Transmittal of Decision 2009-RV-061, Section 2.8.2 of the Central Springbank Area Structure Plan, and the County's Servicing Standards.
 - i. That if capacity remains available via Westridge Water Utilities, the subject lands shall connect to the piped water supply with confirmation/documentation provided to the satisfaction of the Development Authority; and
 - ii. That if capacity is not available via Westridge Water Utilities, the Applicant/Owner shall propose an acceptable alternative water supply for the subject development, to the satisfaction of the Development Authority.
- 7. That prior to release of this permit, the Applicant/Owner shall contact County Road Operations with haul details for materials and equipment needed during construction/site development to confirm if Road Use Agreements or permits shall be required for any hauling along the County road system and to confirm the presence of County road ban restrictions.
 - i. The Applicant/Owner shall also discuss any requirements or improvements that may be required for the approach of Westbluff Road. If required, a New Road Approach application shall be submitted to County Road Operations.
 - ii. Written confirmation shall be received from County Road Operations confirming the status of this condition. Any required agreement or permits shall be obtained unless otherwise noted by County Road Operations.

Permanent:

8. That if the prior to release conditions have not been met by **September 30, 2024**, or the approved extension date, then this approval is null and void and the Development Permit shall not be issued.



> 403-230-1401 questions@rockyview.ca www.rockyview.ca

Dean Thomas Design Group (Ryland Cook) **#PRDP20240118** Page 3 of 4

- 9. That any plan, technical submission, agreement, matter, or understanding submitted and approved as part of the application, in response to a Prior to Release condition, including the required Geotechnical Report, Deep Fills Report, SSIP, and CMP, shall be implemented, and adhered to in perpetuity and also includes:
 - i. The Development Agreement for Site Improvements/Services Agreement (SISA), as registered on title, Instrument No. 151 190 262, as agreed upon between the landowner(s) and Rocky View County.
- 10. That the Applicant/Owner shall submit compaction testing to the County, verifying that the fill areas greater than 1.20 m. (3.93 ft.) in depth were placed in accordance with the overlying technical accepted by the County.
- 11. That the dwelling unit shall not be used as a *Vacation Rental* or for commercial purposes at any time, unless approved by a Development Permit.
- 12. That the pool house and tennis court shall not be used for commercial purposes at any time, unless approved by a Development Permit.
- 13. That this approval does not include an Accessory Dwelling Unit.
- 14. That there shall be a minimum of two (2) dedicated on-site parking stalls for the subject dwelling unit at all times.
- 15. That the Applicant/Owner shall take whatever means necessary to prevent visible dust associated with the development escaping the site and having adverse effects on adjacent roadways and properties.
- 16. That no topsoil shall be removed from the site. All topsoil shall be retained on-site and shall be re-spread on-site and seeded to grass or landscaped after building construction is complete, as part of site restoration.
- 17. That access to the subject parcel shall be via the existing mutual approach and driveway, as shown on the approved site plan and drawings.
 - i. That the existing Access Easement (Instrument #151 190 264) shall remain registered on title, and shall not be discharged from title, unless an alternative physical and legal access acceptable to the County, has been approved for the subject parcel.
- 18. That the Applicant/Owner shall be responsible for rectifying any adverse effect on adjacent lands and access/driveway area from drainage alteration, including stormwater implications from the proposed development. Post-development drainage shall not exceed pre-development drainage.
 - i. That any lot regrading and placement of clean fill shall not direct any additional overland surface drainage nor negatively impact existing drainage patterns in the County's road right-of-way of Westbluff Road.
- 19. That all on-site lighting, including private, site security and parking area lighting, shall be designed to conserve energy, reduce glare, and reduce uplight, in accordance with Sections 225 227 of the County's Land Use Bylaw C-8000-2020. All lighting shall be full cut-off (shielded) and be located and arranged so that no direct rays of light are directed at any adjoining properties, that may interfere with the use and enjoyment of neighbouring lands or interfere with the effectiveness of any traffic control devices or the vision/safety of motorists.



> 403-230-1401 questions@rockyview.ca www.rockyview.ca

Dean Thomas Design Group (Ryland Cook) **#PRDP20240118** Page 4 of 4

20. That if the development authorized by this Development Permit is not commenced with reasonable diligence within twelve (12) months from the date of issue and completed within twenty-four (24) months of the issue, the permit is deemed to be null and void, unless an extension to this permit shall first have been granted by the Development Officer.

Advisory:

- That the Applicant/Owner shall obtain a Building Permit and any applicable sub-trade permits through the County's Building Services department, prior to any construction taking place, using the appropriate checklists and application forms. Compliance with the *National Energy Code* is also required.
- That the subject development shall conform to the County's *Noise Control Bylaw C-8067-2020* and *Road Use Agreement Bylaw C-8323-2022*, in perpetuity.
- That the site shall remain free of Regulated, Prohibited Noxious or Noxious Weeds and the site shall be maintained in accordance with the *Alberta Weed Control Act* [Statutes of Alberta, 2008 Chapter W-5.1, November 16, 2022].
- That there shall be adequate water & sanitary sewer servicing provided for the subject dwelling unit.
- That it is the Applicant/Owner's responsibility to obtain and display a distinct municipal address in accordance with the County's *Municipal Addressing Bylaw (Bylaw C-7562-2016)*, for the subject dwelling unit, to facilitate accurate emergency response. The municipal address for the subject dwelling unit is 242253 WESTBLUFF ROAD.
- That during construction, all construction and building materials shall be maintained on-site in a neat and orderly manner. Any debris or garbage shall be stored/placed in garbage bins and disposed of at an approved disposal facility.
- That any other federal, provincial, or County permits, approvals, and/or compliances, are the sole responsibility of the Applicant/Owner.

If Rocky View County does not receive any appeal(s) from you or from an adjacent/nearby landowner(s) by **Tuesday, March 26, 2024**, a Development Permit may be issued, unless there are specific conditions which need to be met prior to release. If an appeal is received, then a Development Permit will not be issued unless and until the decision to approve the Development Permit has been determined by the Subdivision and Development Appeal Board.

Regards,

Development Authority Phone: 403-230-1401 Email: <u>development@rockyview.ca</u>

ROCKY VIEW COUNTY DEVELOPMENT PERMIT APPLICATION				FOR OFFICE USE ONLY					
				APPLICATION NO. ROLL NO: RENEWAL OF FEES PAID		PRDP20240118 04618044 - \$600.00			
			_	DATE OF RE	CEIPT	01/03/2024			
APPLICANT/OWNER								-	
Applicant Name: Ryland Cook			-		land(@deanthoi	mas.	ca	
Business/Organization Name (if appli			Grou	р	_	1			-
Mailing Address: 1109 Olympic Wa		1	-		_	Postal Co	de: T2	2G 1E	9
Telephone (Primary): 403 829 9285 Alternative: 4(:403	719 6641					
Landowner Name(s) per title (if not the	e Applicant): John a	and Claudi	ne La	ng-Hodg	е				
Business/Organization Name (if appli	cable):								
Mailing Address:						Postal Co	de:		
Telephone (Primary):		Email							
LEGAL LAND DESCRIPTION - Subj	ect site								
All/part of: NE 1/4 Section: 18	Township: 24	Range:	02	West of:	5	Meridian	Divi	sion:	1.21
All parts of : Lot 4	Block: 2	Plan:	151 2	2150	Parc	el Area (ac/	ha):	4.69	ac
APPLICATION FOR - List use and so	cope of work	m setback	and			rict: R-RUR	(Resi	dentia	l, Rura
APPLICATION FOR - List use and so Stripping and grading activity in	cope of work n excess of 15.0			fill above	e 1.0	m			l, Rura
Municipal Address: 242253 Westblu APPLICATION FOR - List use and so Stripping and grading activity in Variance Rationale included: YES NO SITE INFORMATION	cope of work n excess of 15.0	m setback ist Included: ₪		fill above	e 1.0				l, Rura
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RDP20240118	Exhibit 3 - Development Authorit	y Report	Page 26 of		
ROCKY VIEW CO	OUNTY				
	00111	FOR	OFFICE USE ONLY		
STRIPPING. GRA	ADING, EXCAVATION	APPLICATION NO.	PRDP20240118		
AND/OR FILL INFO		ROLL NO.	04618044		
AND/UNTILL INFO	RMATION SHEET	DISTRICT			
DETAILS		APPLICATION	N FOR:		
otal area of work (m ² / ft ² / ac.)		🛛 Site Strippi	-		
Length (m / ft.)		□ Grading □ Re-contouring			
Width (m / ft.)		Excavation (cut-to-fill)			
Height (m / ft.)		. ,	on of artificial waterbody		
Volume (m ³ / ft ³ .)			ing dugouts)		
Number of truckloads (approx.)		□ Stockpiling □ Other:			
Slope factor (if applicable)					
ESCRIPTION OF WORK	of the work proposed (include cover lette				
aterbodies etc,) if applicable:	drainage patterns or environmentally sen any rubble or hazardous substances:	sitive areas (i.e.	riparian, wetland, other		
	, in addition to DP Checklist - General				
Pre-development and Pos	ed with the application (select if provide at-development grading plans vater Management Plan, Fill Management as ALL of the following:		ity Report may be required		
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Stripping, Grading, Excavation and/or Fill – Information Sheet

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LAND TITLE CERTIFICATE

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	EN	CUMBRANCES, LIENS & INTERESTS PAGE 2			
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NUMBER	DATE (D/M/Y)	PARTICULARS			
151 190 259	28/07/2015	UTILITY RIGHT OF WAY			
		GRANTEE - ENMAX POWER CORPORATION.			
		AS TO PORTION OR PLAN:1512151			
151 190 261	28/07/2015	UTILITY RIGHT OF WAY			
		GRANTEE - ROCKY VIEW COUNTY.			
		AS TO PORTION OR PLAN:1512152			
151 190 262	28/07/2015	CAVEAT			
		RE : DEVELOPMENT AGREEMENT PURSUANT TO MUN GOVERNMENT ACT	NICIPAL		
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		911 - 32ND AVENUE NE			
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		OVER AND FOR BENEFIT: SEE INSTRUMENT			
151 234 990	11/09/2015	RESTRICTIVE COVENANT			
		OVER AND FOR BENEFIT OF: SEE INSTRUMENT			
		AS TO PORTION DESCRIBED			

TOTAL INSTRUMENTS: 006

THE REGISTRAR OF TITLES CERTIFIES THIS TO BE AN ACCURATE REPRODUCTION OF THE CERTIFICATE OF TITLE REPRESENTED HEREIN THIS 22 DAY OF DECEMBER, 2023 AT 10:59 A.M.

ORDER NUMBER: 49209514

CUSTOMER FILE NUMBER:



END OF CERTIFICATE

THIS ELECTRONICALLY TRANSMITTED LAND TITLES PRODUCT IS INTENDED FOR THE SOLE USE OF THE ORIGINAL PURCHASER, AND NONE OTHER, SUBJECT TO WHAT IS SET OUT IN THE PARAGRAPH BELOW.

THE ABOVE PROVISIONS DO NOT PROHIBIT THE ORIGINAL PURCHASER FROM INCLUDING THIS UNMODIFIED PRODUCT IN ANY REPORT, OPINION, APPRAISAL OR OTHER ADVICE PREPARED BY THE ORIGINAL PURCHASER AS PART OF THE ORIGINAL PURCHASER APPLYING PROFESSIONAL, CONSULTING OR TECHNICAL EXPERTISE FOR THE BENEFIT OF CLIENT(S).



1109 Olympic Way SE Calgary, Alberta T2G 1B9 **deanthomas.ca**

DEVELOPMENT PERMIT PACKAGE FOR STRIPPING/GRADING/FILL

Proposed Development: 242253 Westbluff Drive Rockyview County, AB Lot 4 Block 2 Plan 151 2150

Existing Land Use: R-RUR (to remain)

Scope of Work on Subject Property:

- Development of site to include large main house, greenhouse, and sports court area
- Large cut area required to achieve walkout grade
- Cut area at rear to be used to fill at front elevation
- Engineered fill over 1.0m required at south portion of lot
- Extent of grade manipulation to exceed 15.0m offset

Dear Rockyview County,

Thank you for accepting our application for stripping/grading and fill. The intention of this application is to capture the extent of grade manipulation required to achieve effective water management design on a large-scale home. The nature of the lot dictates a significant amount of cut grade at the rear, to be used as fill at the front of the property. Also given the slope of the lot to the south, and the configuration of the yard in this area, we will require engineered fill over 1.0m. Beyond this, the extent grading exceeds the required 15.0m offset. We will be involving a civil engineer in the project to complete any required slope-stability, and geotechnical reports, and will be provided at the earliest possible date. Further information surrounding volumes of materials and truckloads etc. will be determined via these reports.

Thank you for your review and please contact us if you have any questions.

The following page has several site photos of the area.

Ryland Cook Director of Production Dean Thomas Design Group ryland@deanthomas.ca 403 829 9285









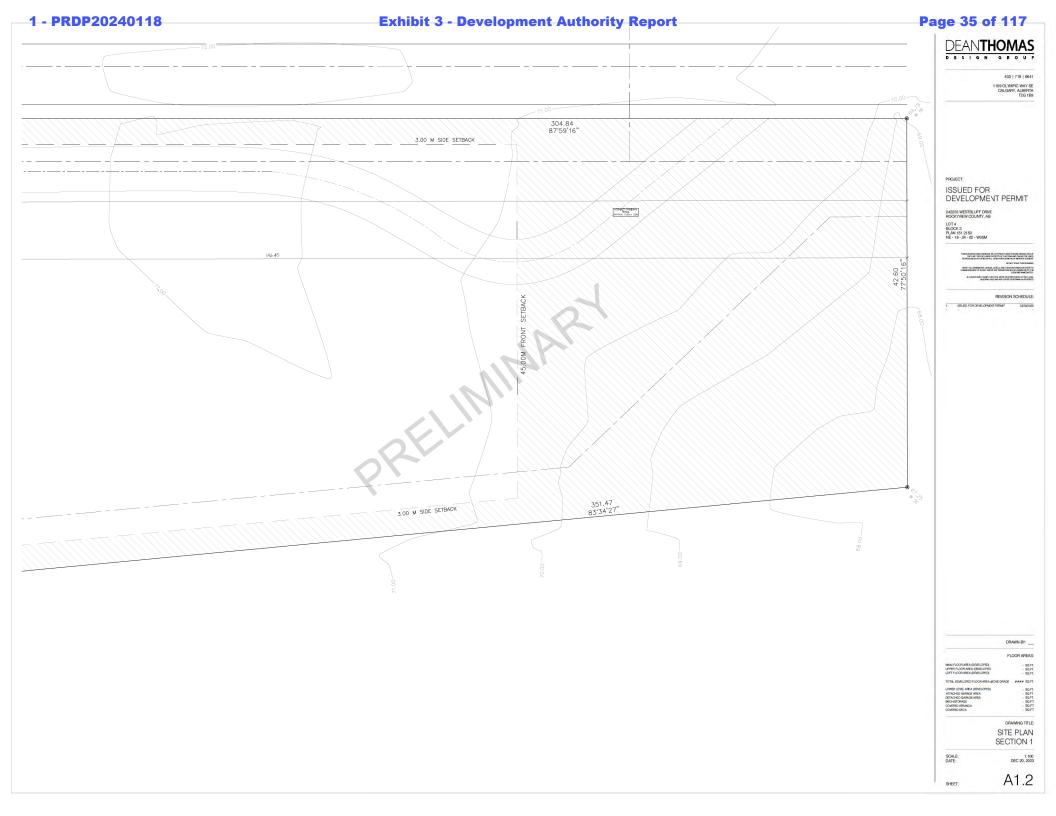


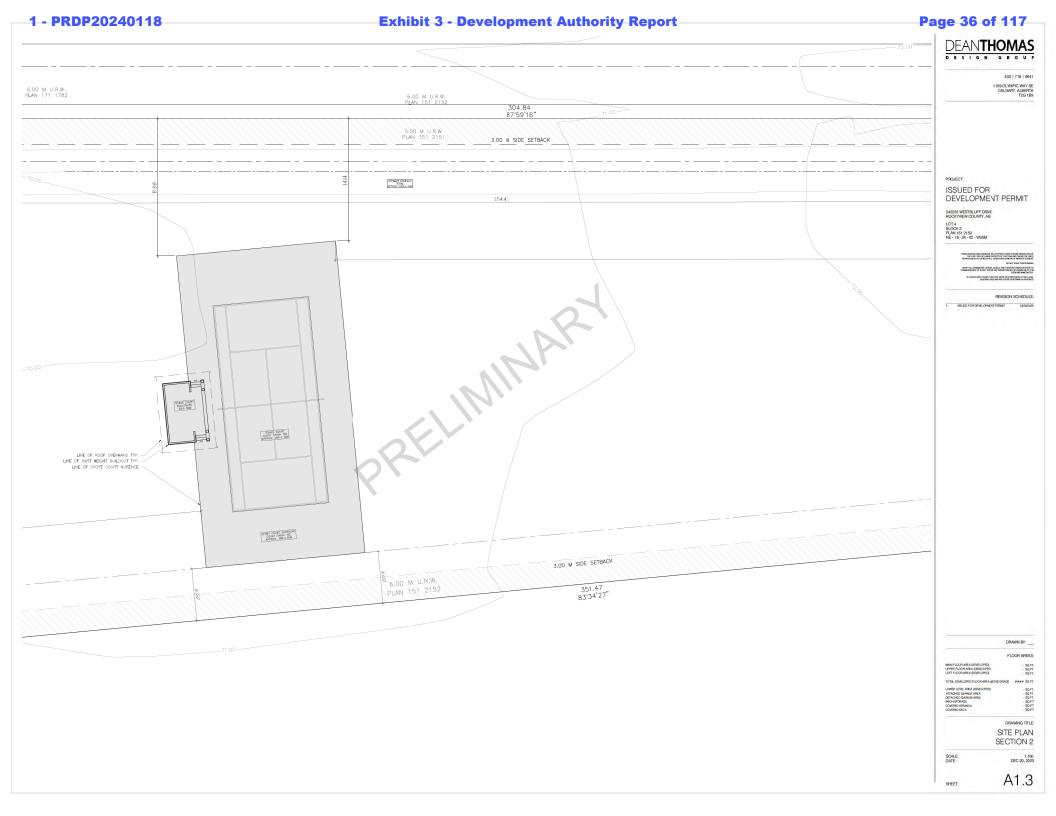


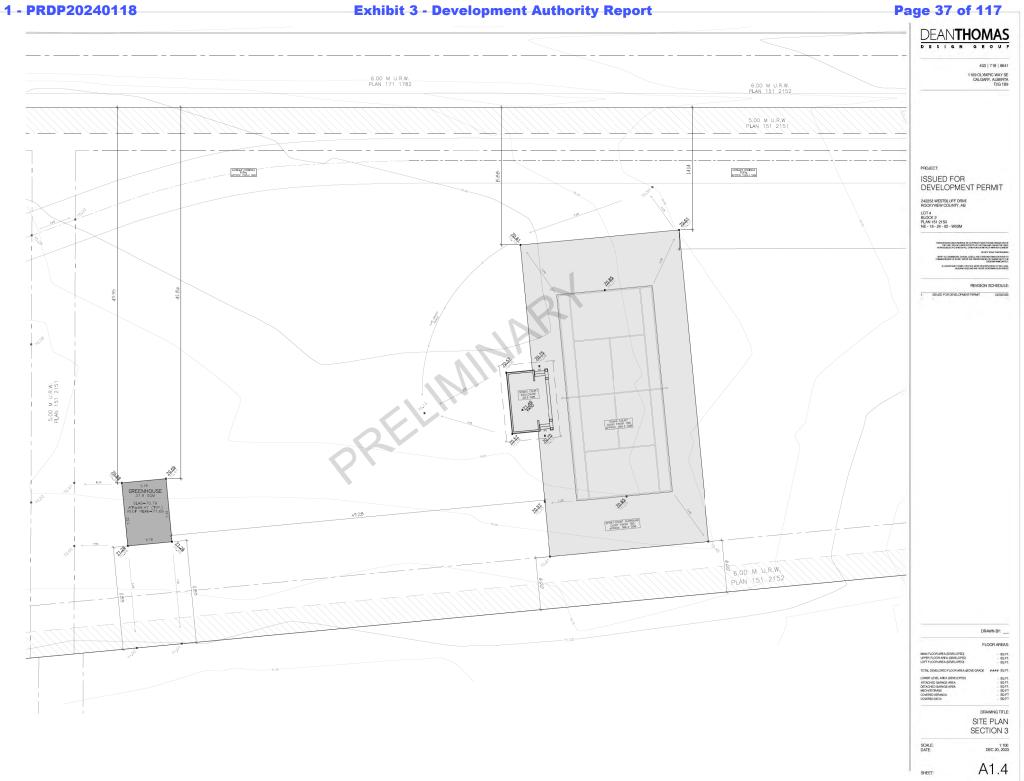
Exhibit 3 - Development Authority Report

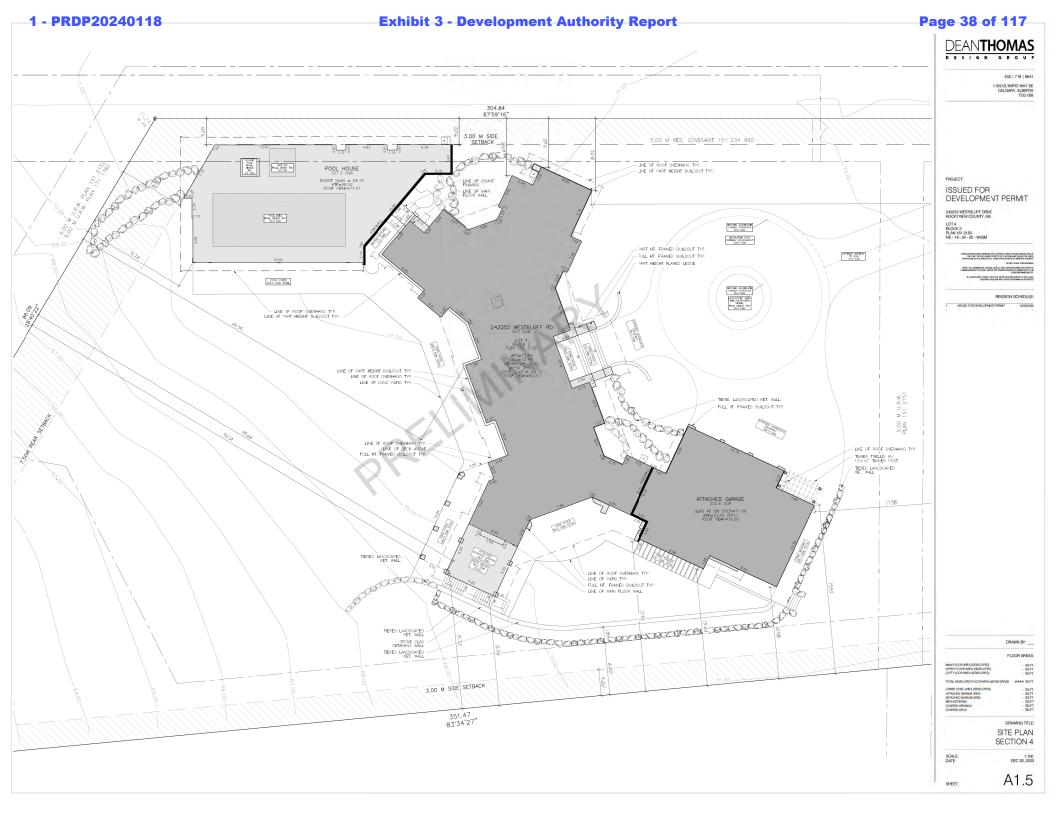
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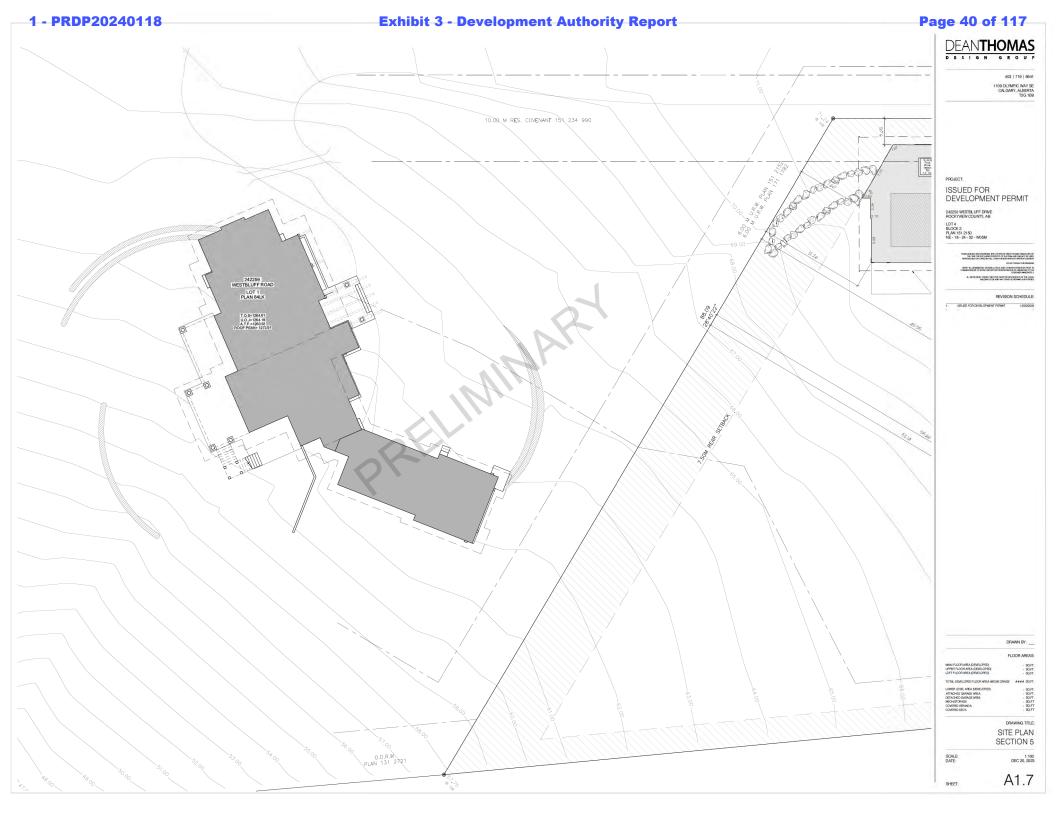




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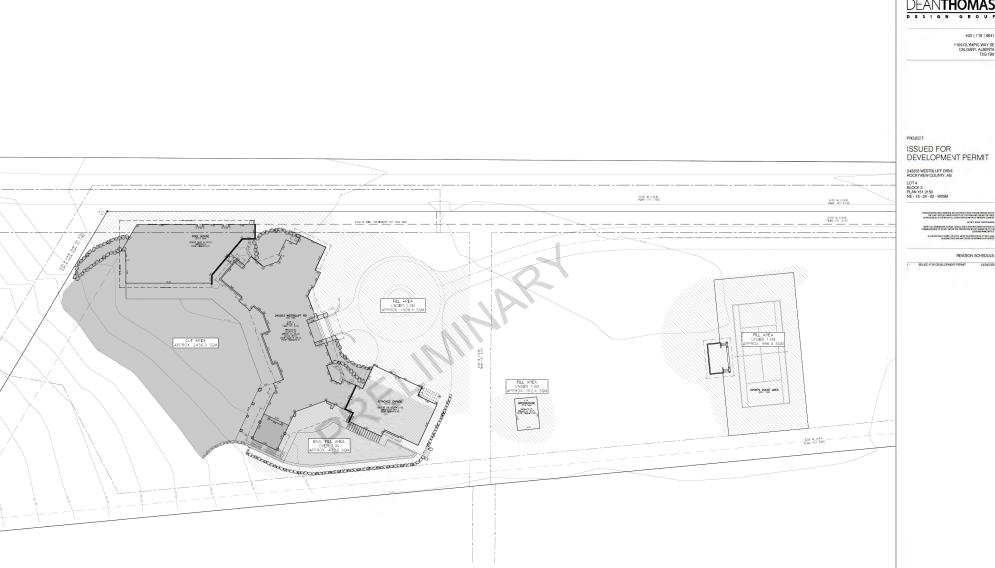
Page 41 of 117

DEANTHOMAS

403 | 719 | 6641 1109 OLYMPIC WAY SE CALGARY, ALBERTA T2G 1B9

REVISION SCHEDULE:

12/20/2029



DRAWN BY: FLOOR AREAS: - 90.FT. - 90.FT. - 90.FT. MAIN FLOOR AREA (DEVEL) UPPER FLOOR AREA (DEVEL) LOFT FLOOR AREA (DEVEL) TOTAL DEVELOPED FLOOR AREA JEONE GRADE #### SQ.FT. LOWER LEVEL AREA (DEVEL ATTACHED GARAGE AREA DETACHED GARAGE AREA MECHESTORAGE COVERED VERANDA COVERED VERANDA COVERED DECK - 90.FT. - 90.FT. - 90.FT. - 90.FT - 90.FT - 90.FT DRAWING TITLE: CUT AND FILL PLAN SCALE: DATE: 1:400 DEC 20, 2023 A1.8

SHEET:



ISSUED FOR PRICING REVISIONS: FEBRUARY 2, 2024

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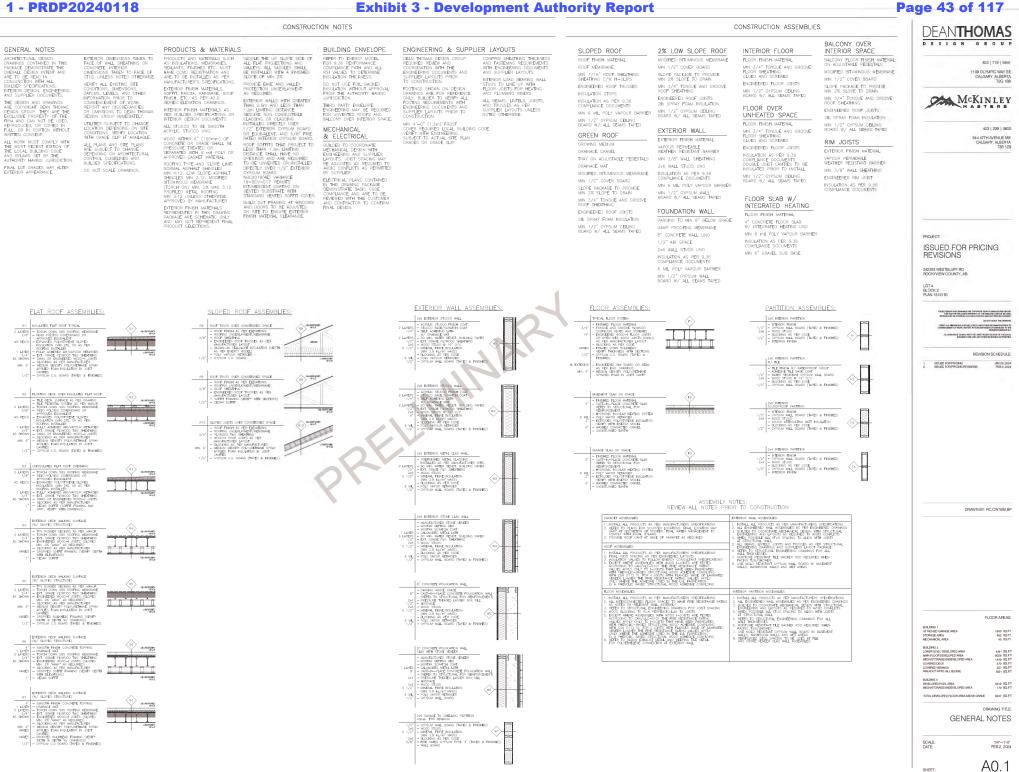


Exhibit 3 - Development Authority Report

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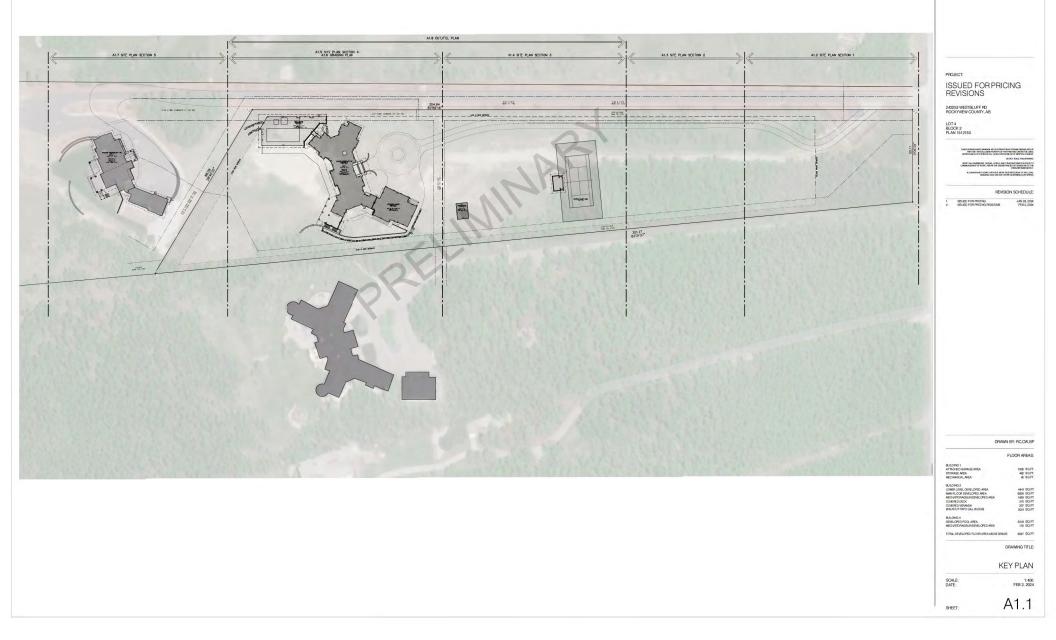
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DESIGN GROUP 403 | 719 | 6641

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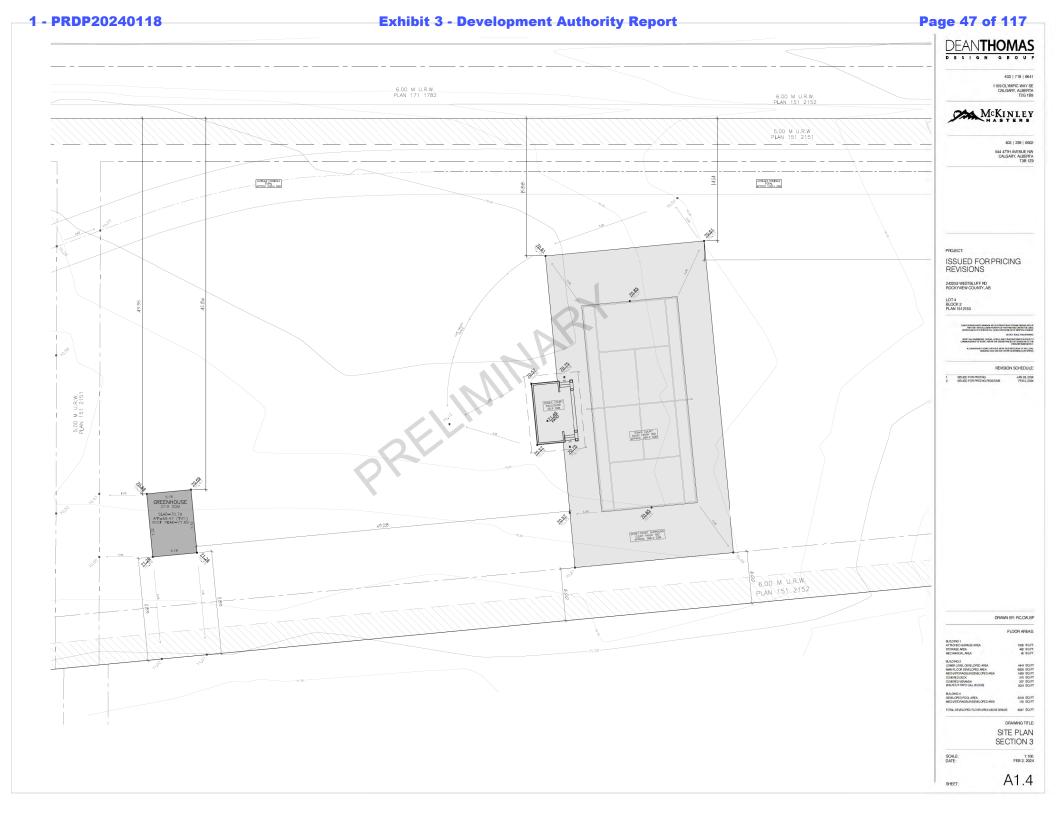
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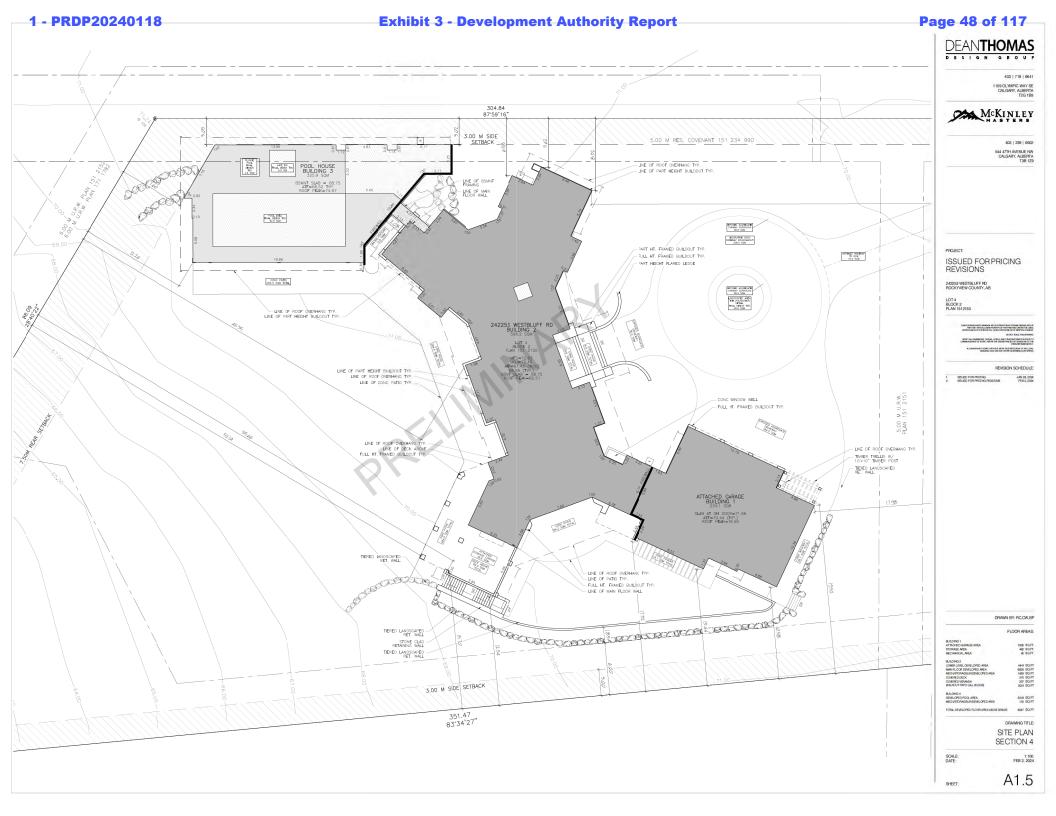
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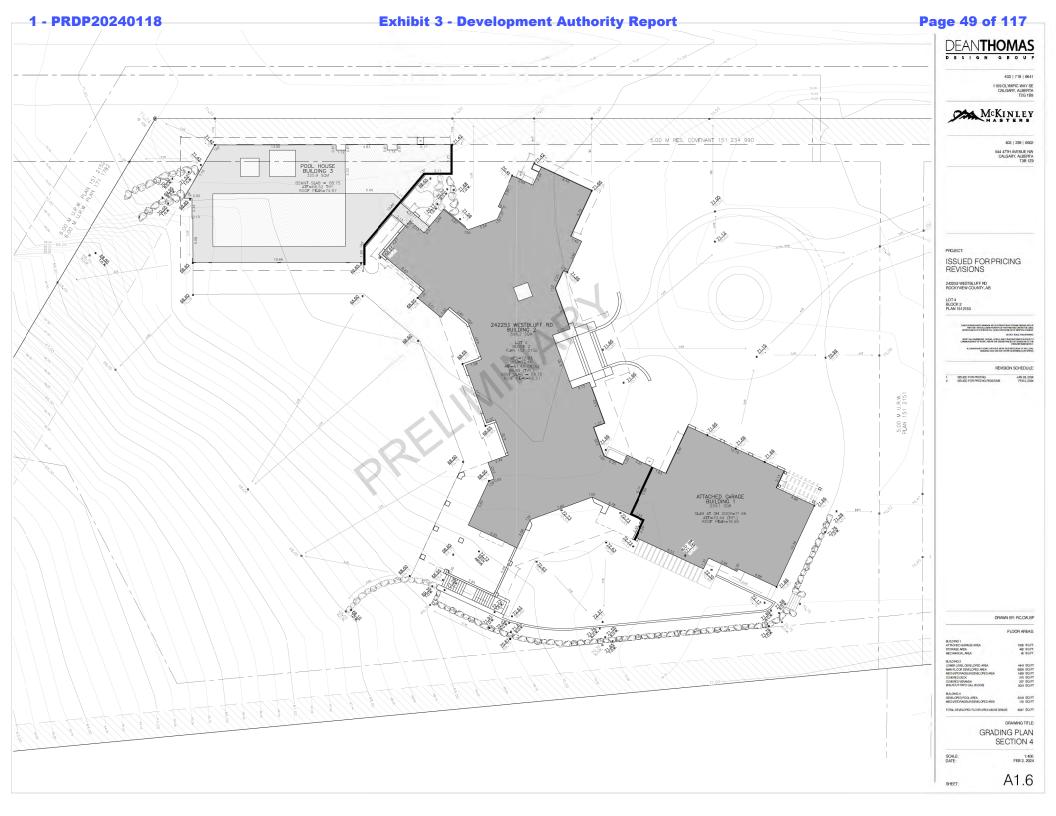


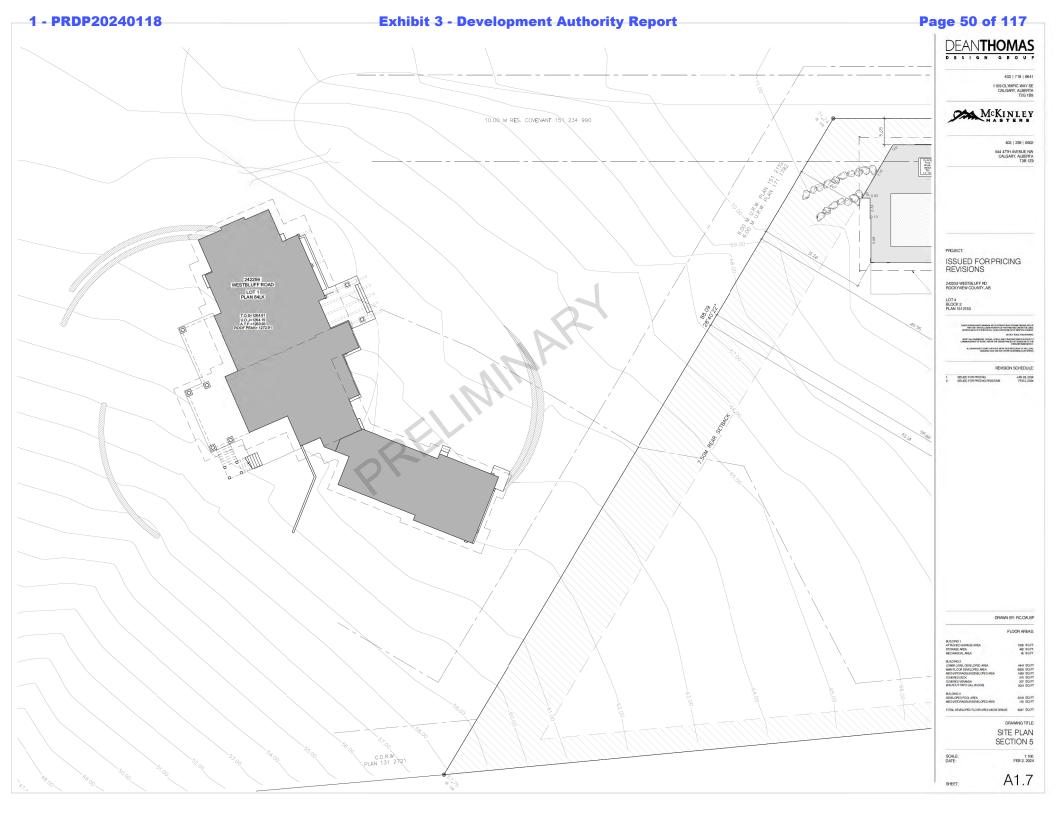
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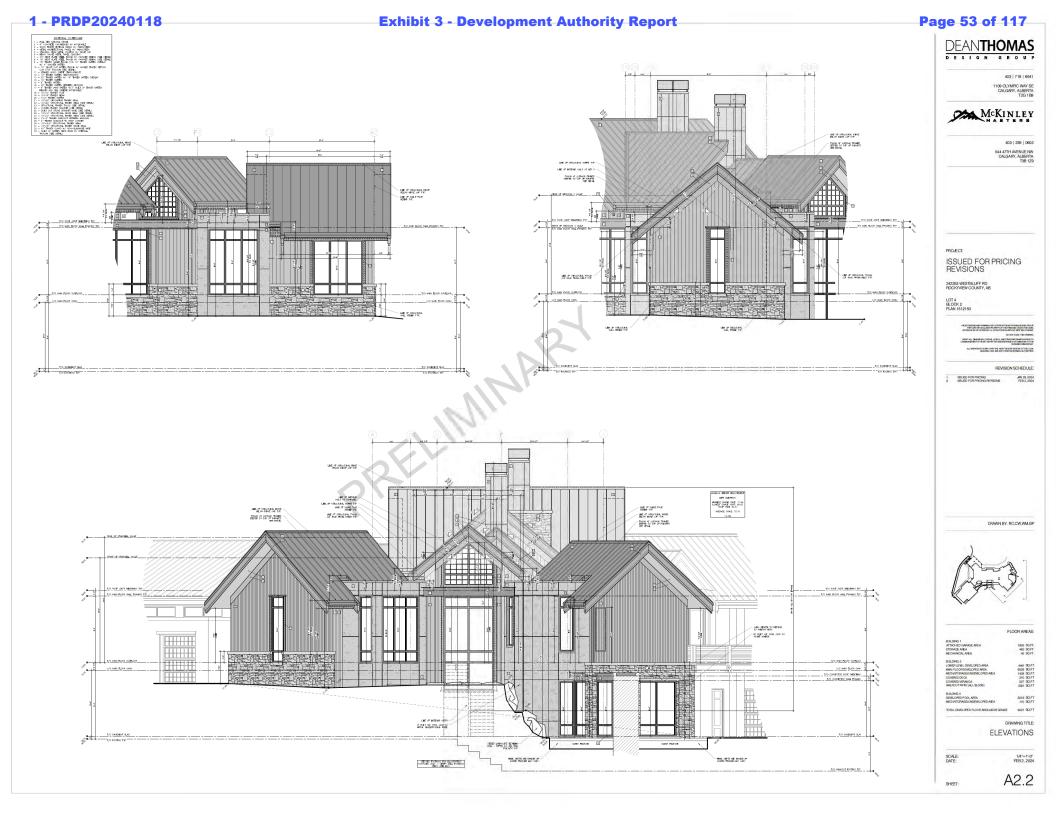


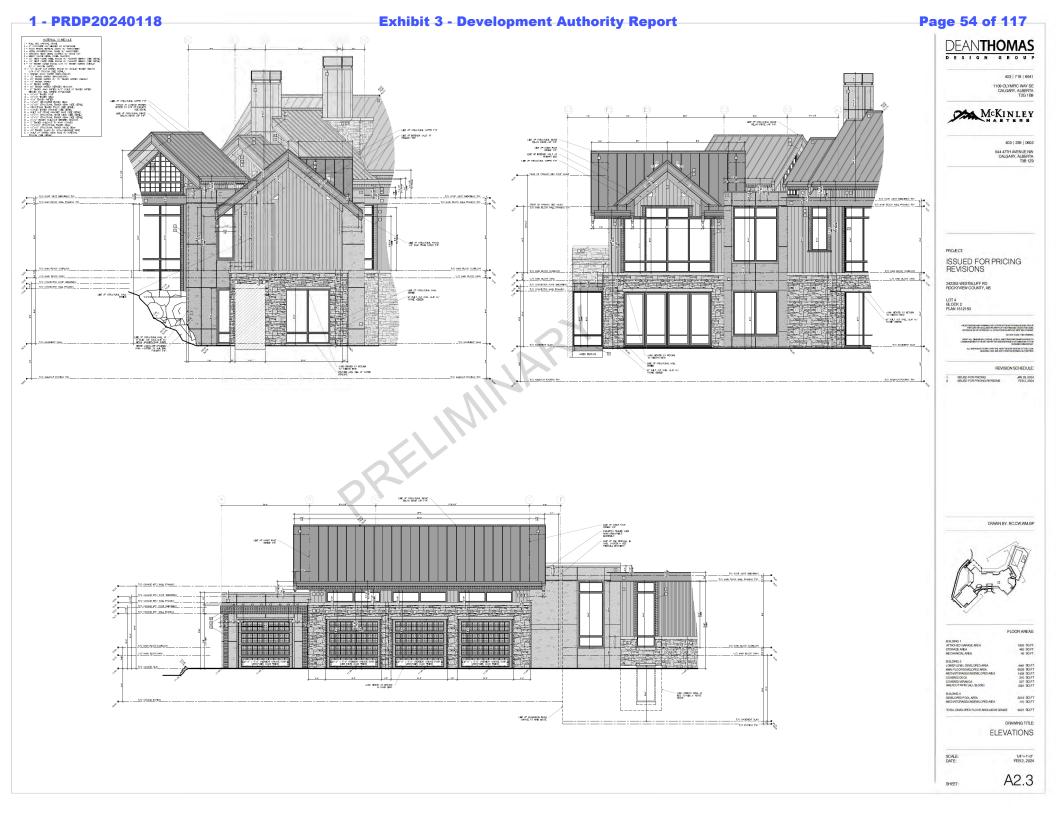




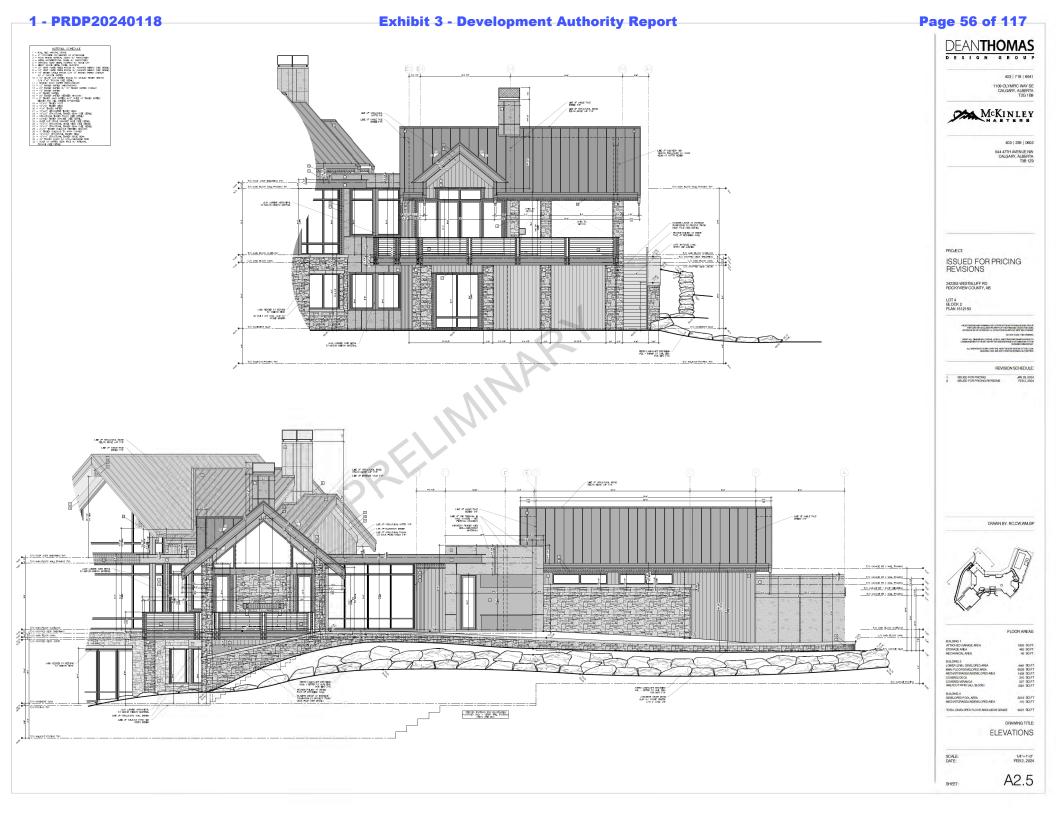




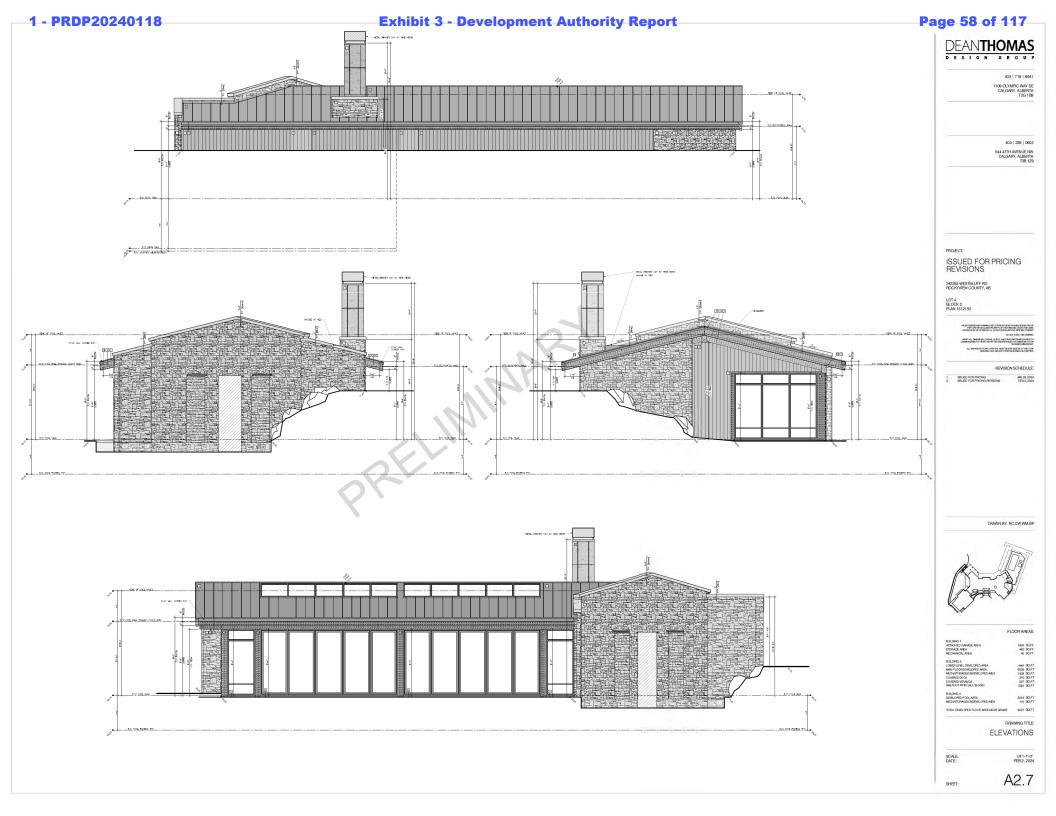






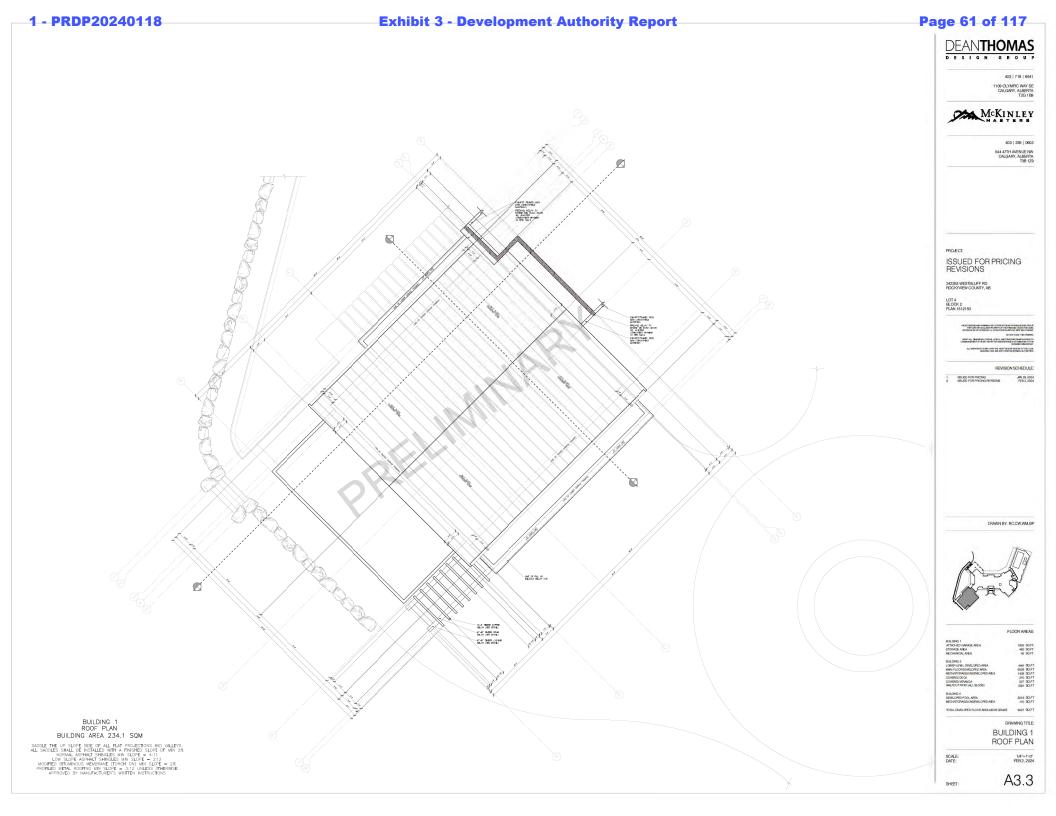


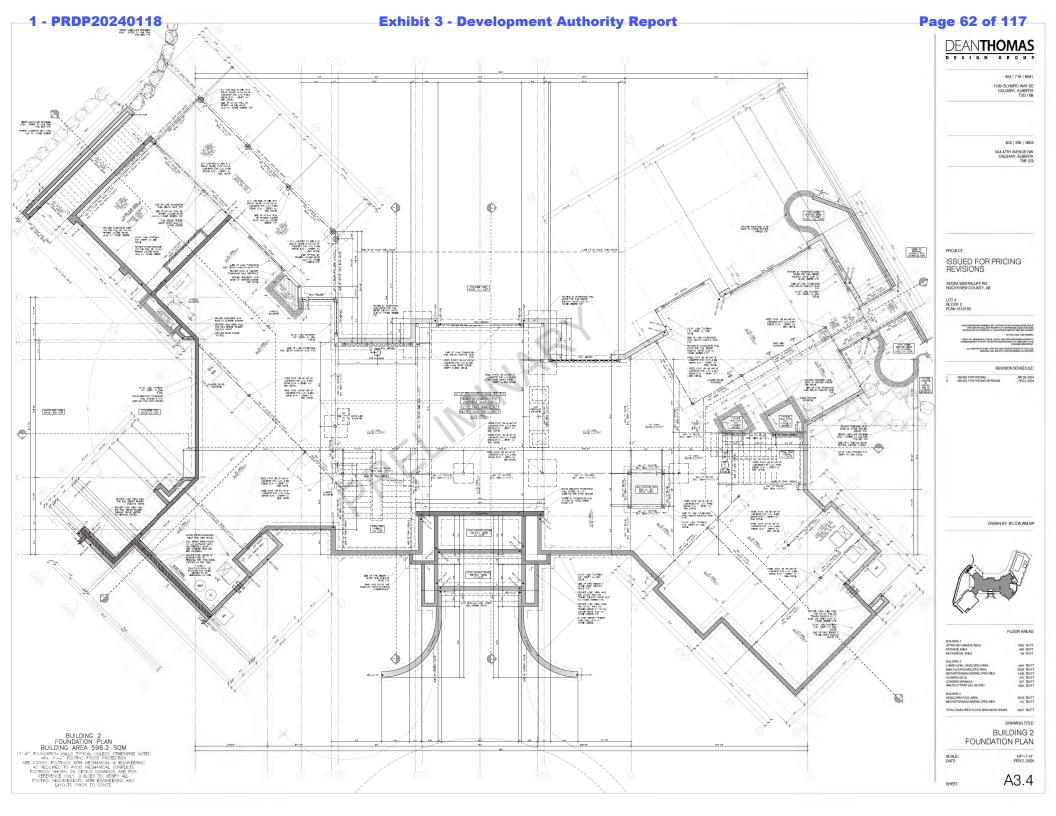


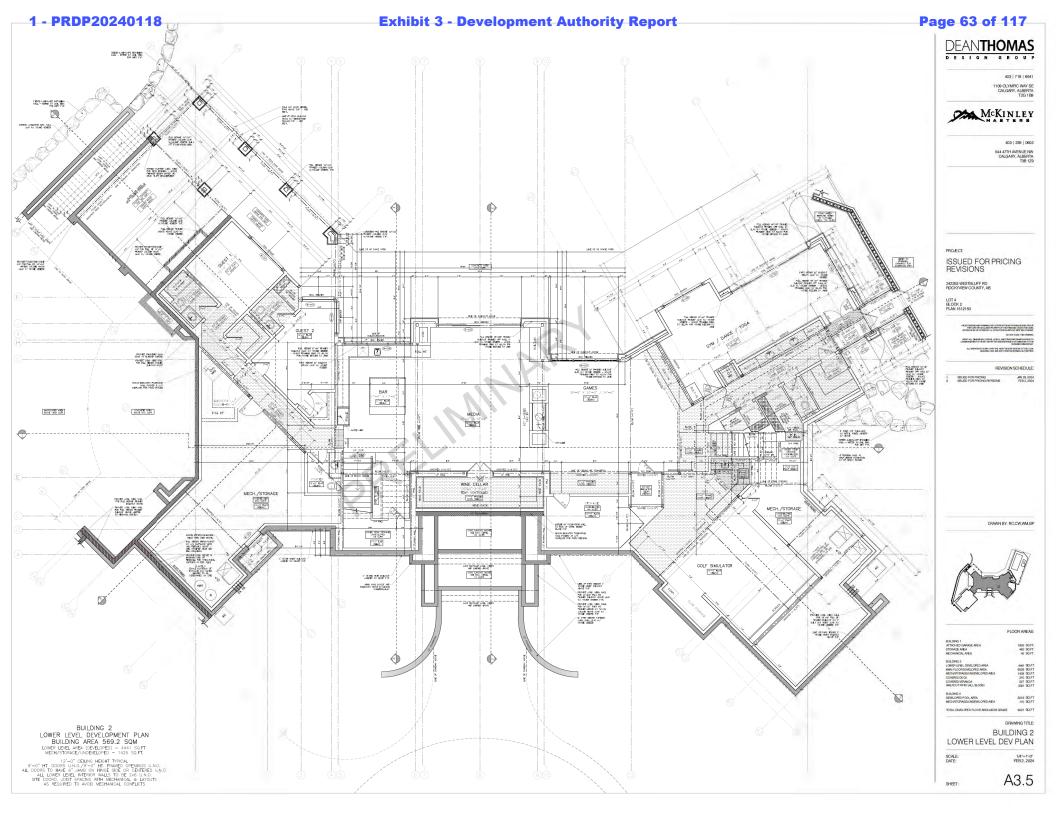


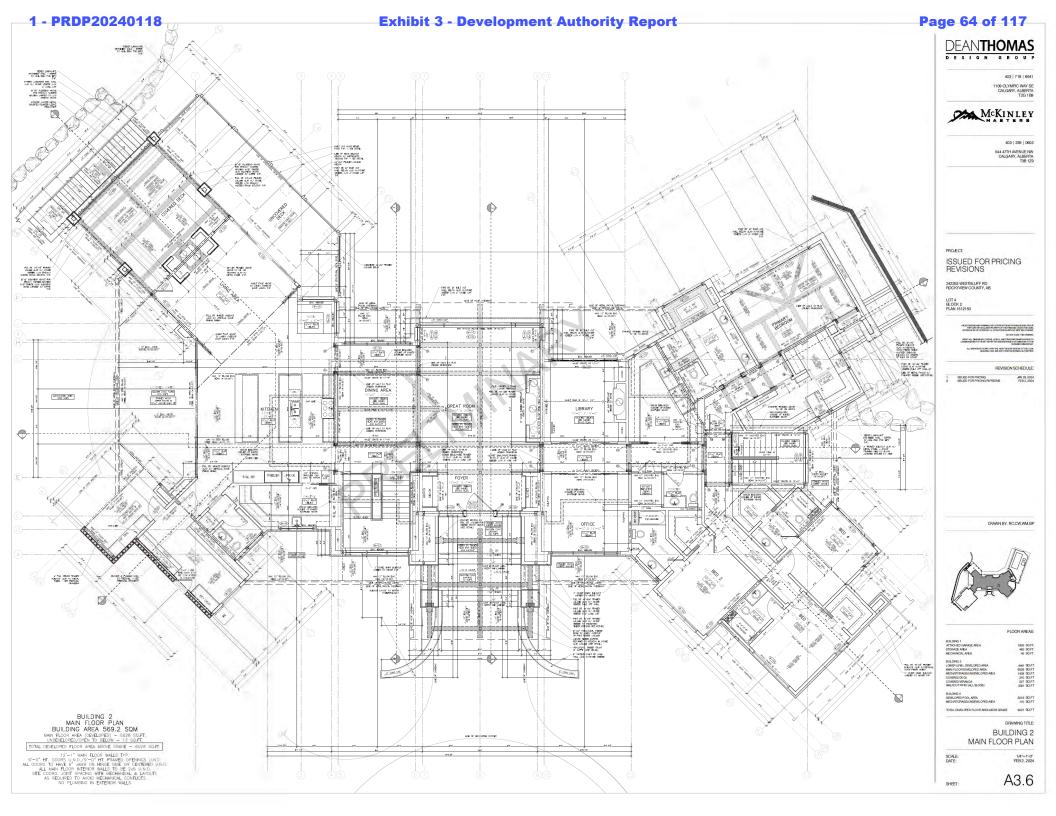


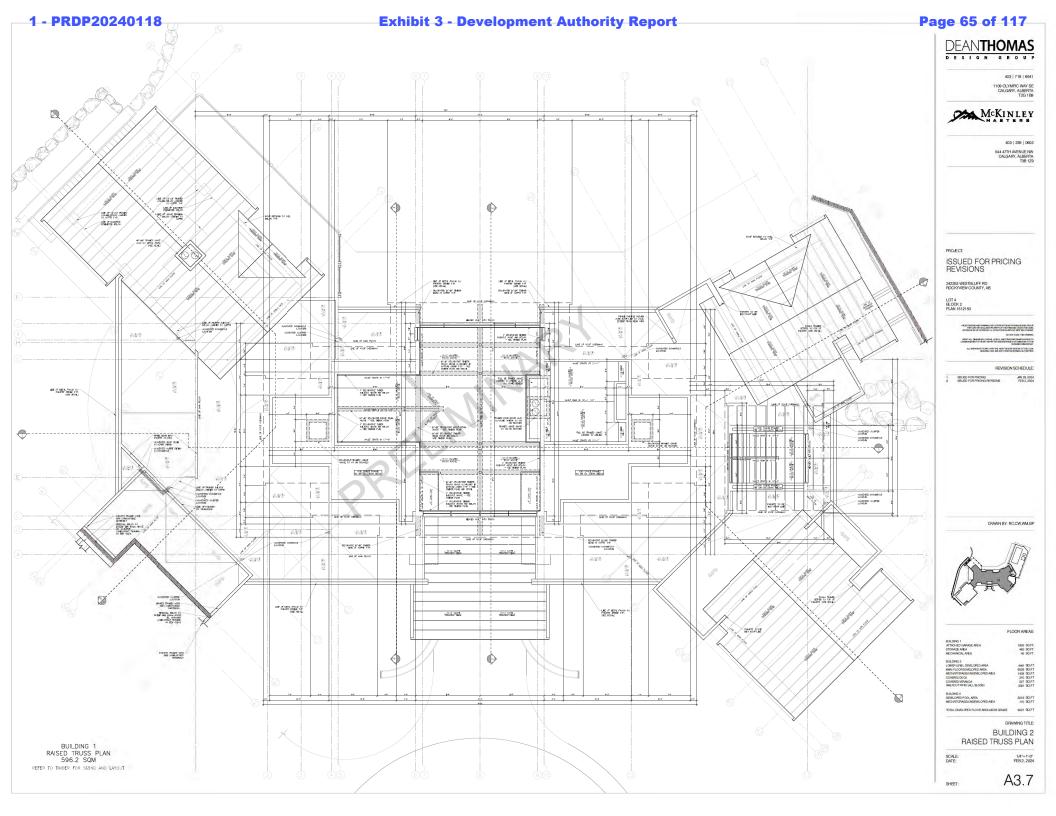


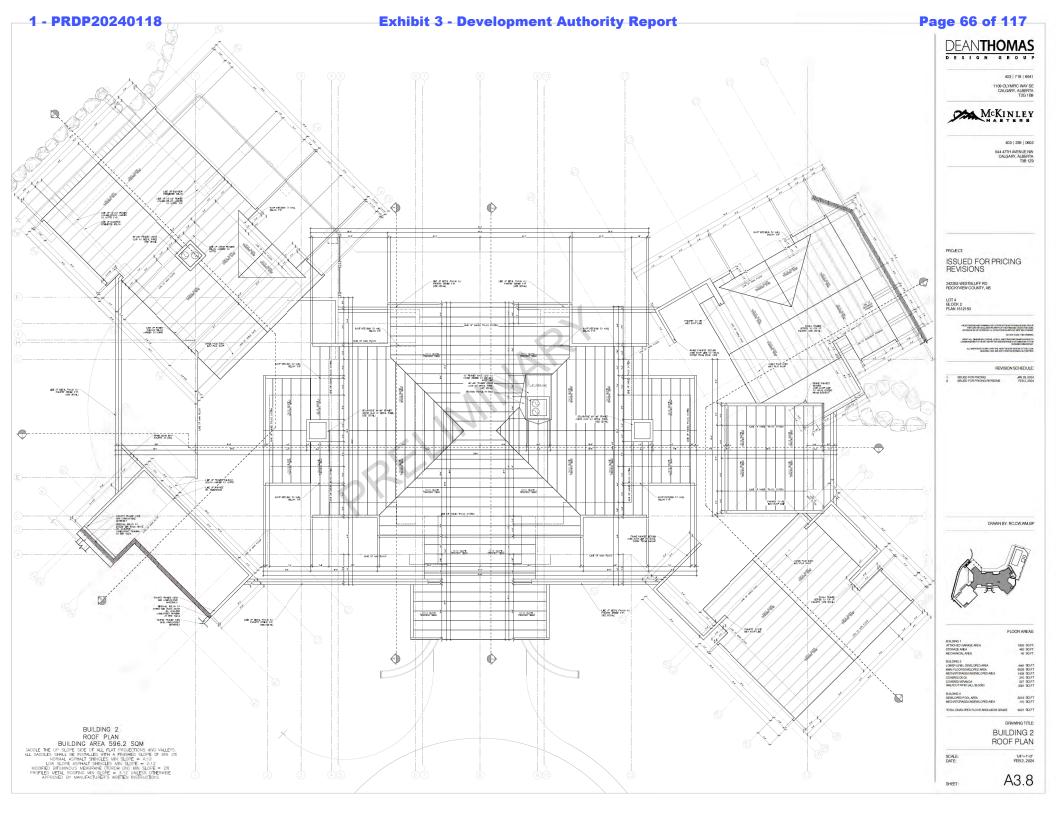


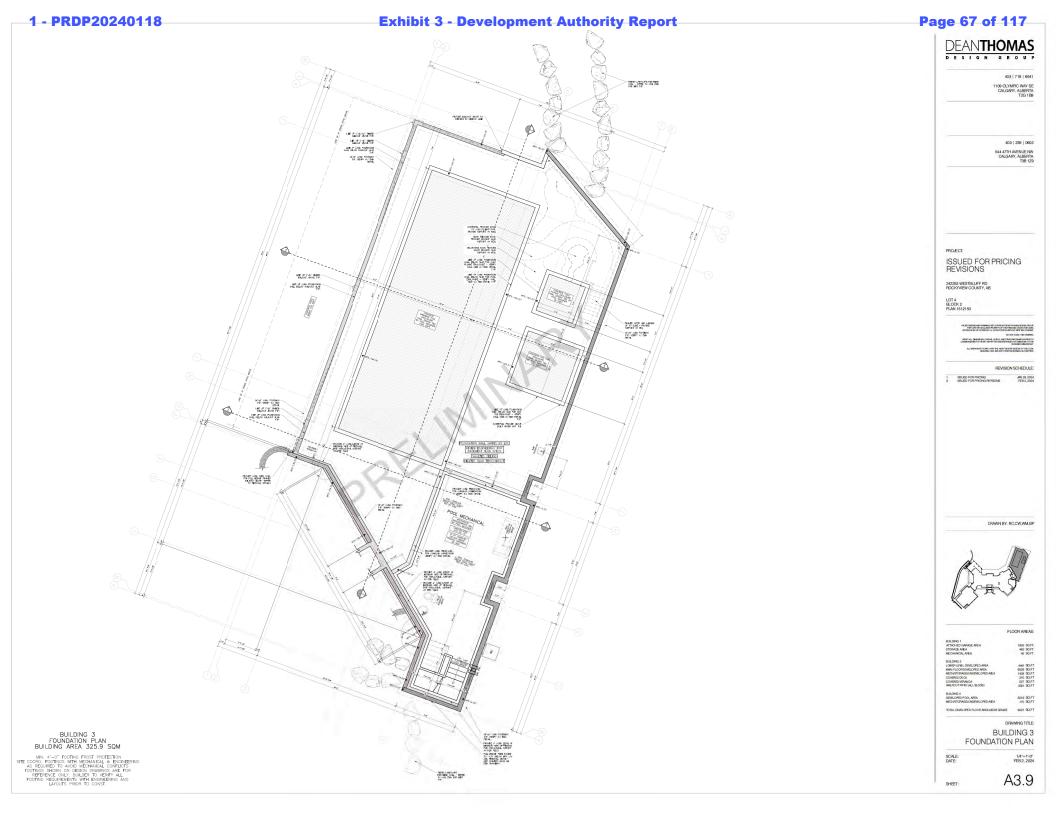






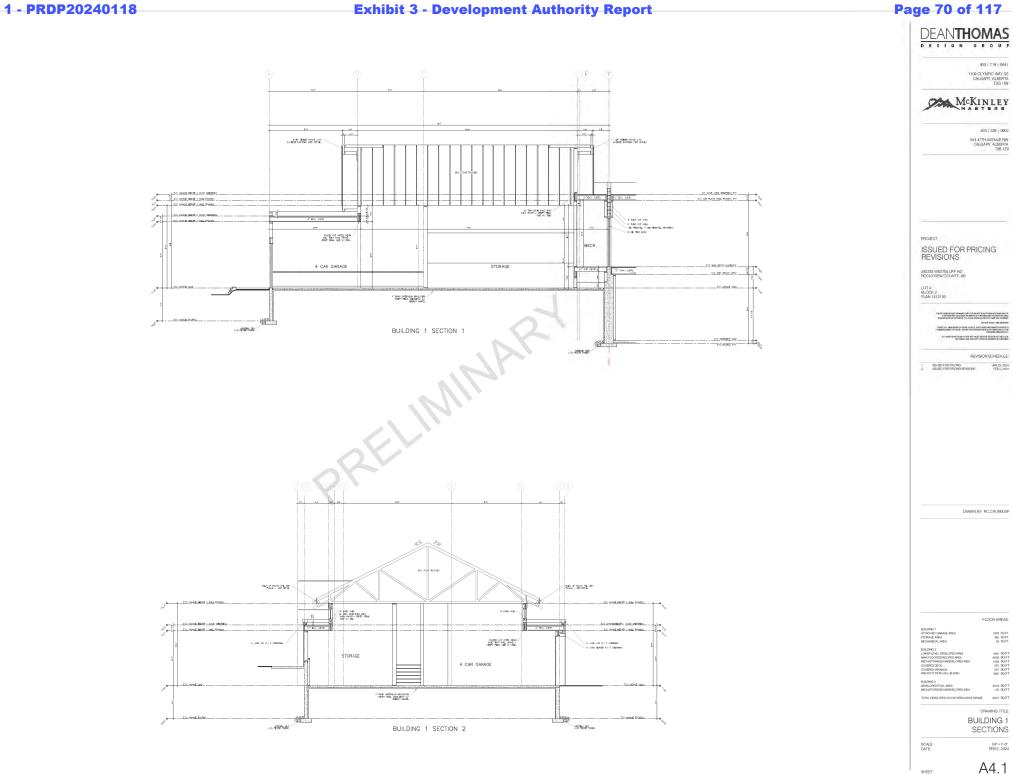


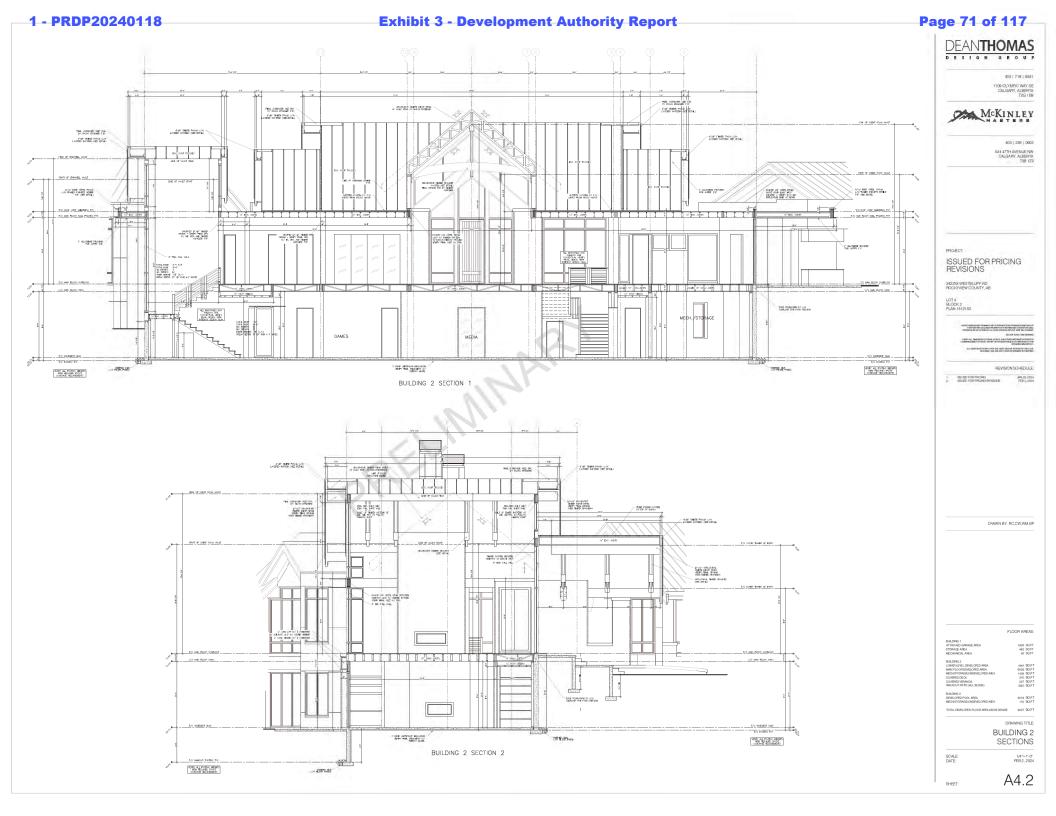


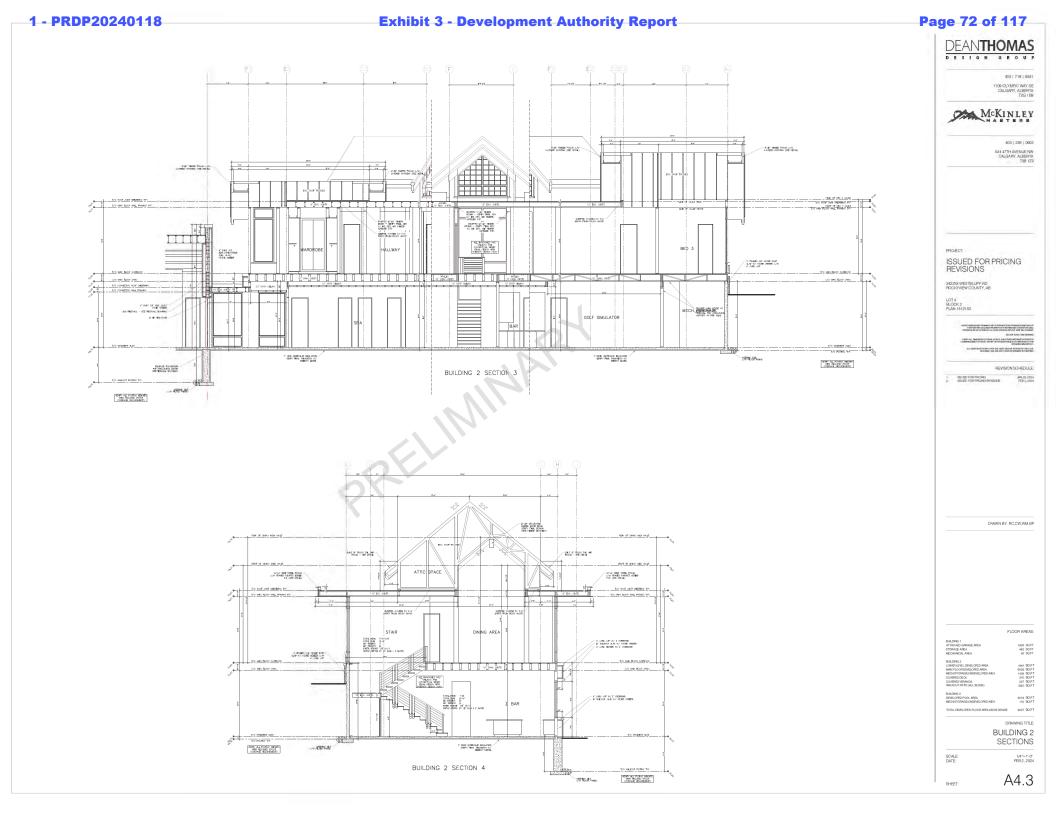


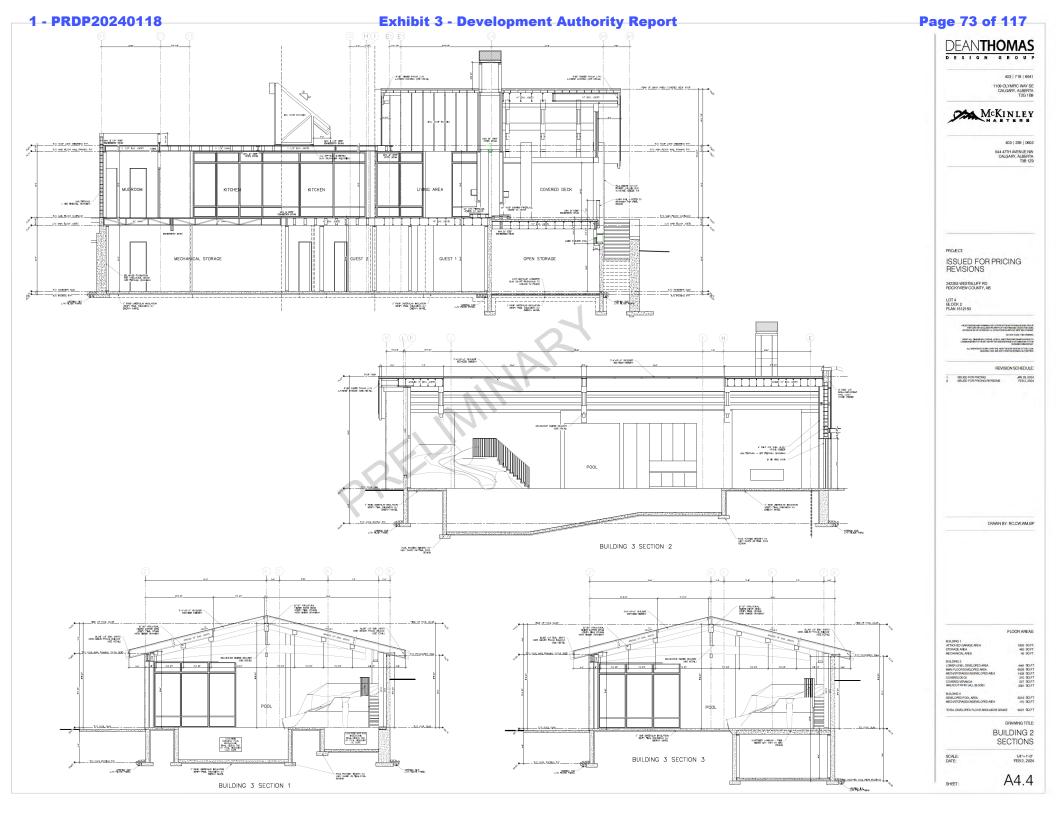




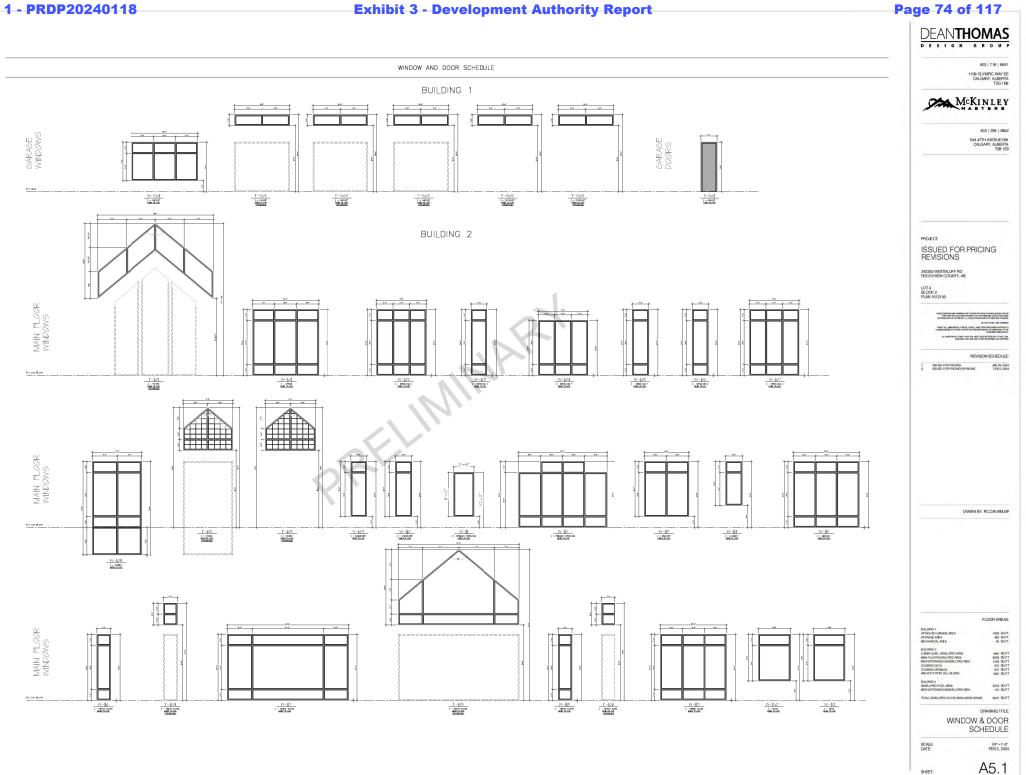


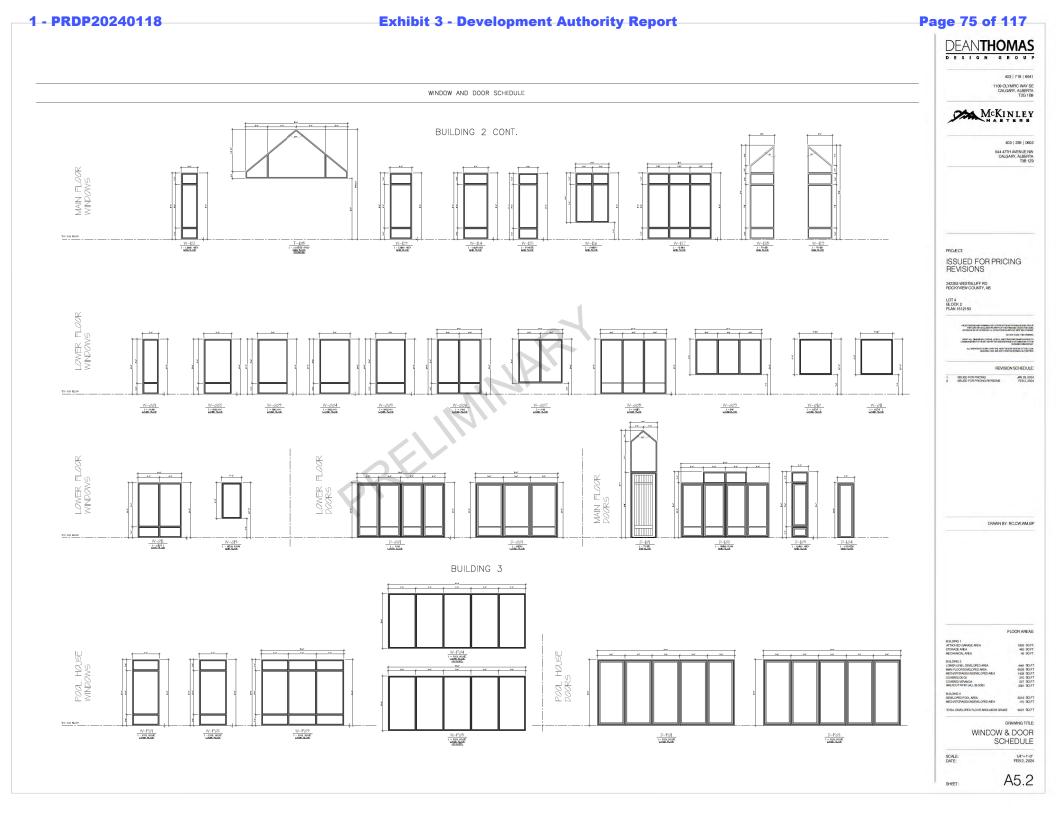






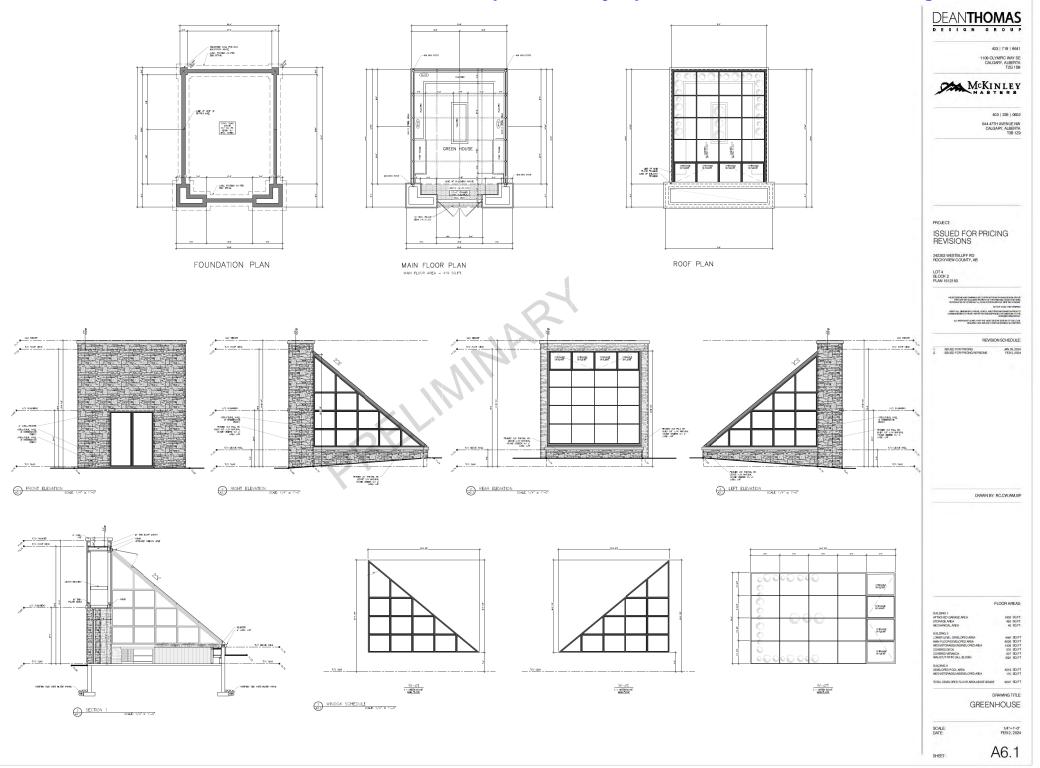
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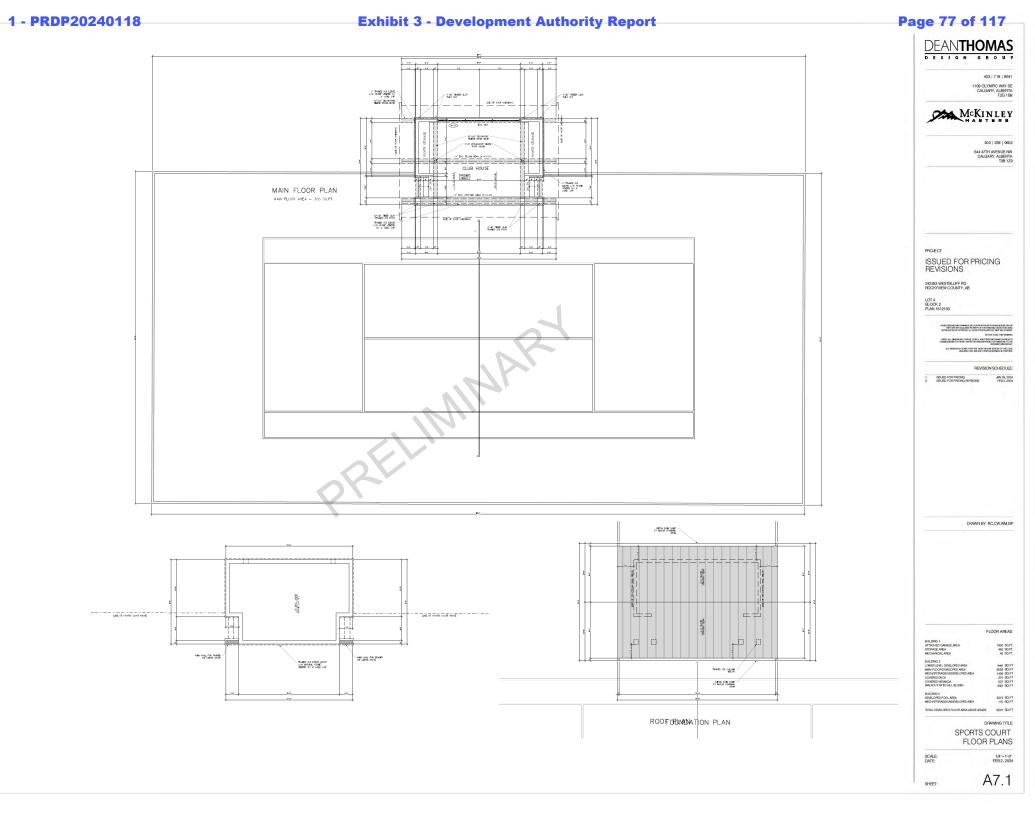


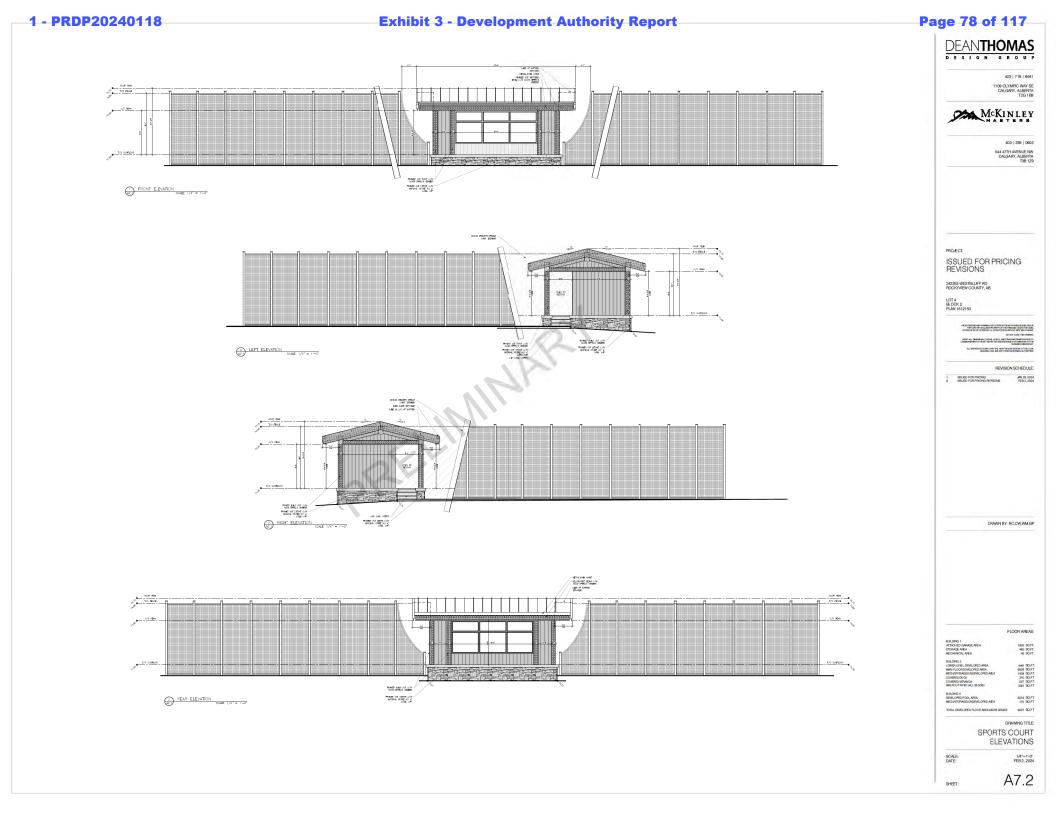




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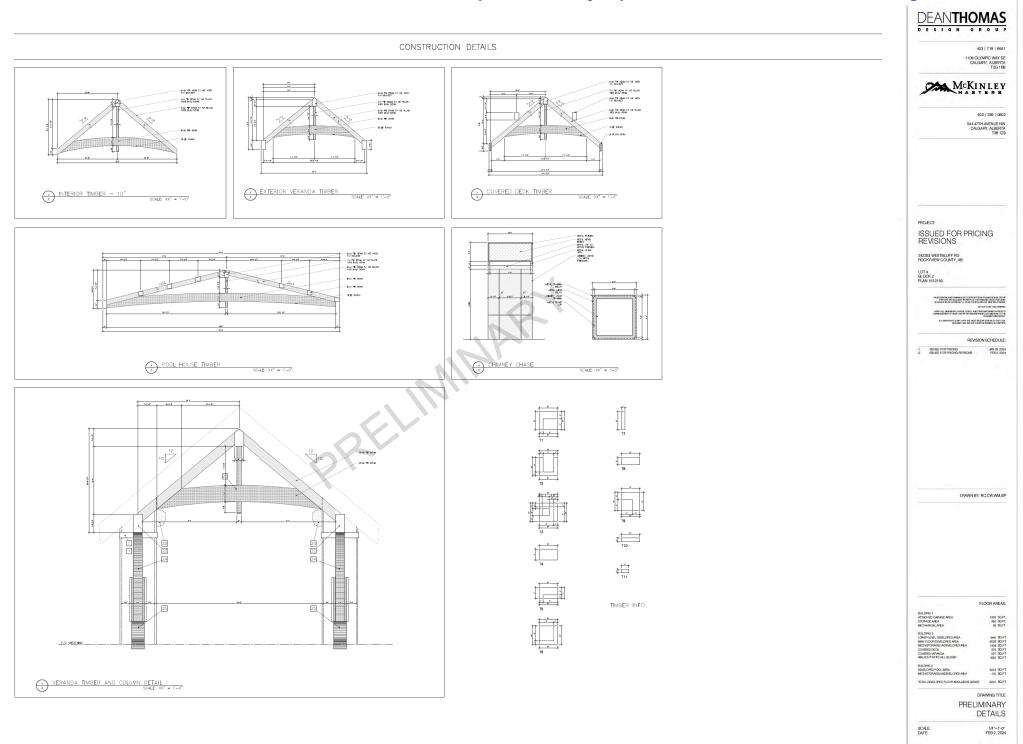


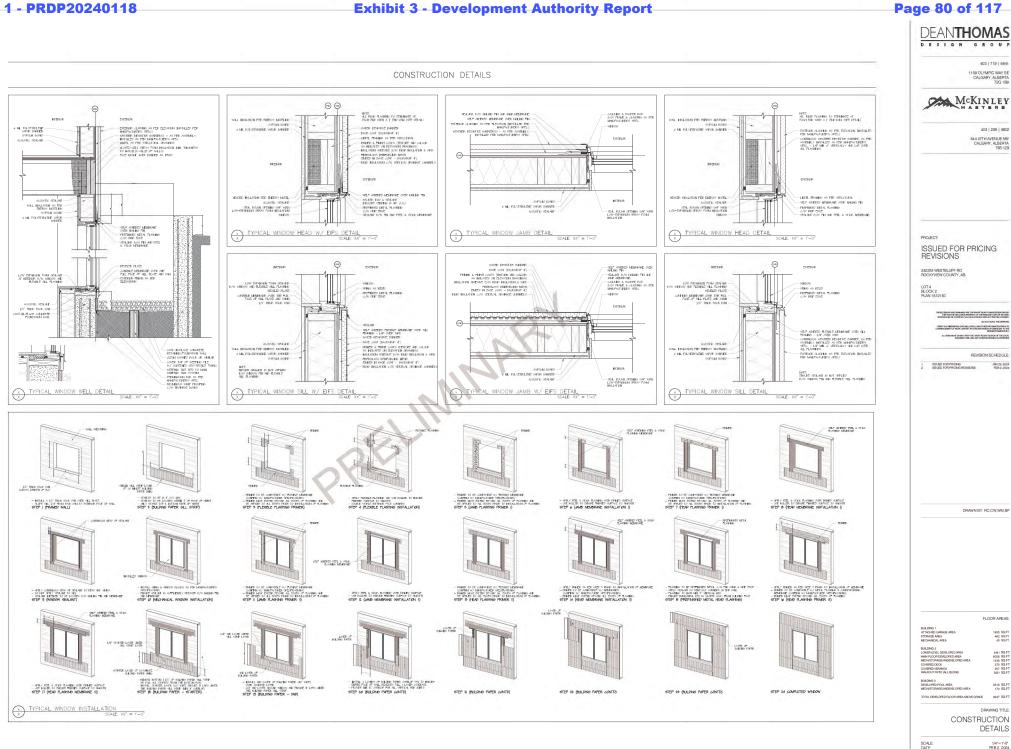


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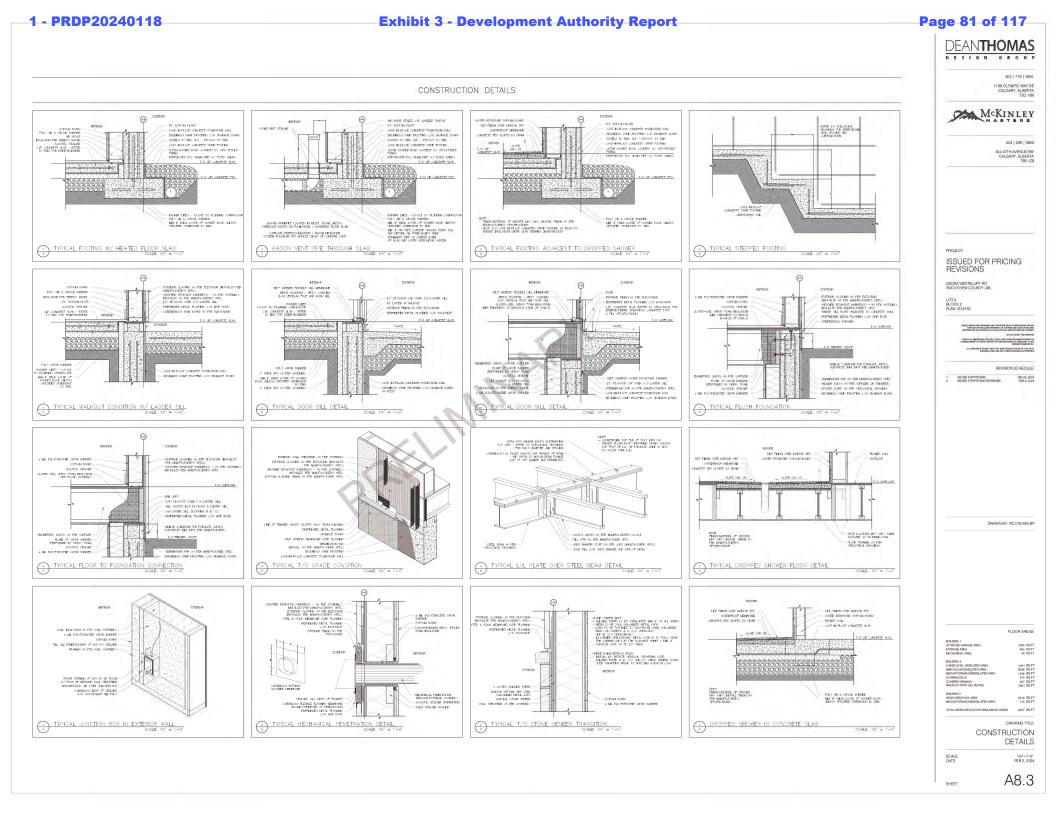
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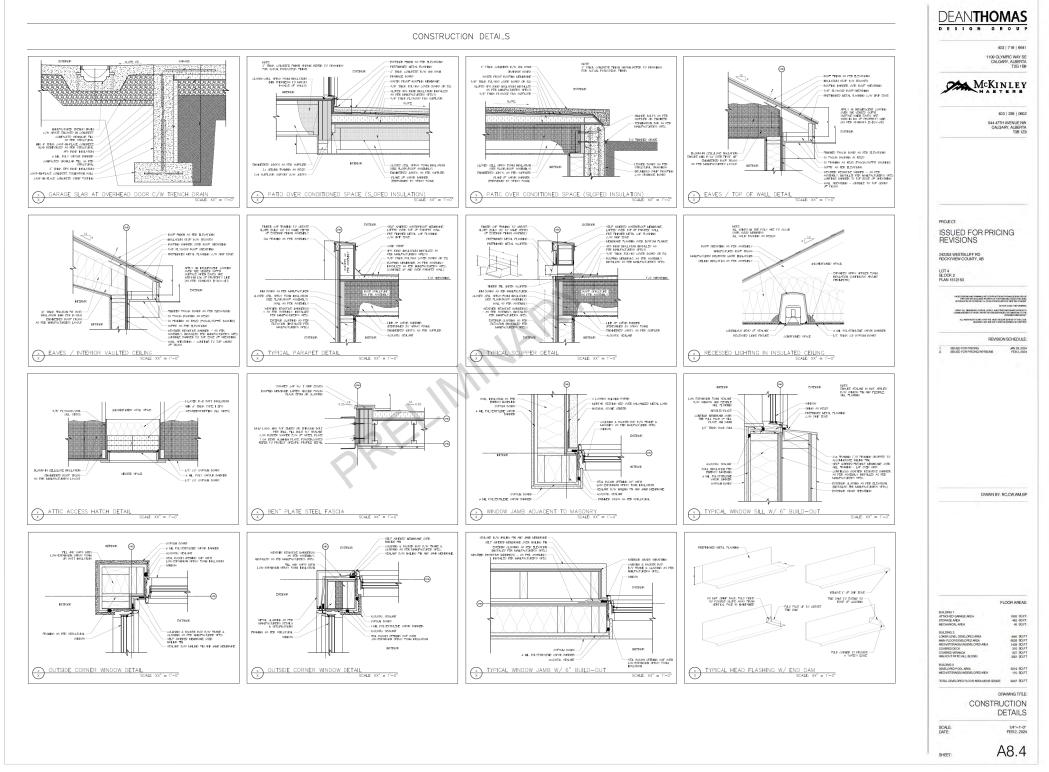


A8.2

SHEET



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Rocky View County 262075 Rocky View Point Rocky View County, AB T4A 0X2

Attn: Planning Department / Development Authority

Reference: Development Permit #PRDP20240118 Lot 4, Block 2, Plan 1512150, NE-18-24-02-05 (242253 Westbluff Road)

To whom it may concern:

Please be advised we are in receipt of your letter dated March 5, 2024 (Roll: 04618044), and would like to point out something that we trust will be considered in the evaluation of this Development Permit.

Several paragraphs in you letter reference stormwater issues (Paragraphs 2.i.; 4., 4.i, 4.ii, 18., 18.1). We would like to request that in its assessment, the County fully consider the implications of the proposed development vis-à-vis stormwater matters affecting surrounding properties.

There is a stormwater culvert in existence that runs beneath the driveway at 242259 Westbluff Road, perpendicular to the property line demarking the 242259 Westbluff Road and 242253 Westbluff Road properties. This culvert allows stormwater runoff from homes on Westbluff Ridge to flow onto the 242253 Westbluff Road property. It is located between the property lines of 15 Westbluff Ridge and 19 Westbluff Ridge. We also believe there was a berm constructed along the 242253 Westbluff Road's North property line East of the culvert, the purpose of which was to divert stormwater onto the subject property for proper drainage.

Thank you for your attention to this matter. If you could please confirm receipt of this letter and send it to Chris Lange at we would appreciate it.

Sincerely, Chris Lange Wendy Partridge

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Parkland Geotechnical Consulting Ltd. A-14, 6120 - 2nd Street S.E. Calgary, AB, T2H 2L8 www.parklandgeo.com T: 403 252 5036 F: 403 343 7966

> August 10, 2016 Project No. CA0241-01

amartens@mcdowelldesign.com Original will remain on file

McDowell & Associates Inc. Suite 501, 933 - 17 Avenue SW Calgary, Alberta T2T 5R6

ATTN: Mr. Abe Martens

RE: Geotechnical Slope Assessment 242253 Westbluff Road Rocky View County, Alberta

Dear Mr. Martens,

Parkland Geotechnical Consulting Ltd. (ParklandGEO) has been commissioned to undertake a slope assessment for the proposed residence at 242253 Westbluff Road, in Rocky View County, Alberta. The residence has been proposed at the crest of a southwest-facing slope. Rocky View County development guidelines allow unrestricted development on sites where slopes are no steeper than about 6.5H:1V (15 percent). This limit is set as a flag to trigger site specific slope stability assessments for proposed developments. The slope below the proposed residence is about 50 to 70 m high and up to about 3.0H:1V (33 percent) steep.

The slope assessment in this report is intended to provide a reasonable expectation with respect to slope stability and the potential for slope movement, and to communicate the technical risks so that informed development decisions can be made relating to the slope. This report is based on the results of drilling and test pitting undertaken at the site on June 22 and July 7, 2016, soil testing, site reconnaissance, and a review of available information. Available information for this assessment included: a topographic plan of the site, geological data, aerial photographs, and plan drawings for the proposed residence. In addition, three borehole logs from a previous investigation undertaken by ParklandGEO at the site in 2013 were referenced in the assessment.

Project No. CA0241-01 August 10, 2016 Page 2 of 8

1.0 SITE AND PROJECT DESCRIPTION

The proposed residence is located at 242253 Westbluff Road, in Rocky View County, Alberta. The location of the site is shown on the Key Plan, Figure 1. The residence has been proposed at the crest of a southwest-facing slope. The slope face on the property was stripped of topsoil at the time of the investigation. The natural slope to the west of the property was vegetated with native grasses and brush, and partially treed. As shown on the 2016 Aerial Photograph, Figure 8, the property is accessed from Westbluff Road and surrounded by similar residential lots in the community of Springbank.

A topographic plan of the site was provided by McDowell & Associates Inc. and four cross sectional profiles were surveyed by ParklandGEO. From the topographic plan and the survey, the slope below the proposed residence is about 50 to 70 m high and up to about 3.0H:1V (33 percent) steep.

The local geology in the area consists of a thin layer of glacial till of the Spy Hill formation, draped over tertiary gravel and bedrock of the Porcupine Hills formation of the Paleogene period. Bedrock of the Porcupine Hills formation generally consists of sandstone and siltstone. This sedimentary bedrock was formed through the cementation of ancient sand, silt, and clay particles deposited millions of years ago. Geologically, the current day slope is considered to have been formed by ancient cementation and erosion, followed by glacial action, followed by more recent water and wind erosion.

A 732 m² (approx.) bungalow style residence with a walkout basement has been proposed. The residence will include four bedrooms, four bathrooms, two four-car garages, and a west facing deck overlooking the walkout area. The basement has been proposed to a depth of up to about 3.5 m below exterior grade. A conventional strip and spread footing foundation is being considered for the residence.

2.0 FIELD AND LABORATORY PROGRAMS

On June 22, 2016, three boreholes were drilled to depths of 5.7, 4.8, and 5.2 m below grade with a Becker Hammer rig. On July 7, 2016, two test pits were excavated to depths of 0.9 and 2.8 m below grade with a rubber tired excavator. In addition, three borehole logs from a previous investigation undertaken by ParklandGEO at the site in 2013 were referenced in the assessment. The locations of the boreholes and test pits are shown on Figures 2 and 3.

The soils encountered were visually examined and logged according to the Modified Unified Soil Classification System. Becker Penetration Tests (BPTs) were recorded at 0.3 m intervals in the three boreholes drilled with a Becker Hammer rig. Soil samples were taken at selected depths in the boreholes and test pits and returned to ParklandGEO's laboratory for testing to determine the soil properties. Testing included moisture contents, grain size distribution, plasticity, and water soluble sulphates.



Upon completion of drilling, 25 mm standpipes were installed in the boreholes. Groundwater level measurements were taken on May 16, 2013 and July 5, 2016. The ground surface elevations at the borehole and test pit locations were surveyed by ParklandGEO using a Trimble Geo7X GPS receiver and a Trimble Zephyr Model 2 GPS antenna. The estimated post data correction vertical accuracy of this equipment is +/- 10 cm. The elevations are referenced to a geodetic datum.

3.0 SUBSURFACE CONDITIONS

The soil profile encountered at the site was, in descending order: topsoil, clay till, gravel, sand and silt, and bedrock. The following is a brief description of the soil types encountered:

- 1. A 180 to 400 mm thick layer of surficial topsoil was encountered in five of the six boreholes and both of the test pits. The topsoil was organic, black, and damp to moist.
- 2. Glacial silty clay deposits (clay till) were encountered in all six boreholes and one of the test pits, and extended to depths ranging from 0.5 to 2.6 m below grade. These deposits were a mixture of clay and silt, with varying proportions of sand and gravel, and occasional rust stains, coal inclusions, cobbles, and boulders. The clay till was generally medium plastic and very stiff with moisture contents ranging from 7 to 19 percent. The clay till is expected to have an internal angle of friction of at least 29° and a bulk unit weight of about 20.0 kN/m³. The clay till will have a small amount of long term cohesive strength (estimated less than 5 kPa).
- 3. Tertiary gravel deposits were encountered in three of the six boreholes and extended to a depth of 2.5 m below grade in Borehole 1, and beyond the 2.1 m depths drilled in Boreholes 1A and 1C. The sandy gravel was generally fine grained and well graded, with frequent cobbles, occasional boulders, and moisture contents ranging from 1 to 5 percent. The BPT values ranged from 124 to 290 blows per 300 mm of penetration, indicating that these deposits were very dense. The gravel is expected to have an internal angle of friction of at least 38° and a bulk unit weight of about 21.5 kN/m³. These gravel deposits are considered typical in this upland area of Rocky View County.
- 4. Sand and silt deposits were encountered in four of the six boreholes and one of the test pits and extended to depths ranging from 0.5 to 3.4 m below grade. These deposits were generally fine grained and poorly graded, with sandstone inclusions, and moisture contents ranging from 5 to 14 percent. The BPT values ranged from 24 to 62 blows per 300 mm of penetration, indicating that these deposits were generally dense. The sand and silt is expected to have an internal angle of friction of at least 35^o and a bulk unit weight of about 21.0 kN/m³.



- 5. Weathered bedrock was encountered in three of the six boreholes and both of the test pits at depths ranging from 0.5 to 3.4 m below grade. The bedrock was sedimentary in origin and consisted of sandstone and siltstone. The upper zone of the formation is considered to be weak, poorly cemented, and weathered rock, which generally has the density/consistency of a very dense/hard soil. The weathered bedrock is expected to have an internal angle of friction of at least 35° and a bulk unit weight of about 22.0 kN/m³. Residual bonding in the upper zone of the formation will provide an small amount of long term cohesive strength (estimated less than 10 kPa). Intact bedrock deeper within the formation expected to be well cemented resulting in much greater strength properties.
- 6. No groundwater seepage was observed in the boreholes and test pits during drilling and excavation. On May 16, 2013 and July 5, 2016, all of the boreholes were dry. The groundwater conditions at the site are expected to vary seasonally with peaks and possible perched conditions during periods of snow-melt and heavy or prolonged precipitation. Groundwater pressure and springs may be present in the bedrock at certain times of the year.

The detailed subsurface conditions encountered at the borehole and test pit locations are described on the attached logs. The soil test results and definitions of the terminology and symbols used on the logs are provided on the attached explanation sheets.

4.0 SLOPE ASSESSMENT

Slope stability analysis was conducted to assess the slope below the proposed residence. Rocky View County development guidelines allow unrestricted development on sites where slopes are no steeper than about 6.5H:1V (15 percent). This limit is set as a flag to trigger a site specific slope stability assessments for proposed developments. The slope below the proposed residence is about 50 to 70 m high and up to about 3.0H:1V (33 percent) steep.

Slope stability is described in terms of a factor of safety (FS) against slope failure which is the ratio of total forces resisting failure divided by the sum of forces promoting failure. In general, a FS of less than 1 indicates that failure is expected and a FS of more than 1 indicates that the slope is stable. A steepened slope will slump back over time to establish a stable profile for the existing soil and groundwater conditions. The FS of a slope will increase slightly as vegetation is established on the face to protect the subgrade soil from weathering. Given the possibility of soil variation, groundwater fluctuation, erosion, and other factors, slopes with a FS ranging between 1.0 and 1.5 are considered to be marginally stable and a "long term" stable slope is considered to have a FS of over 1.5.

A FS of at least 1.5 is desired for the critical failure surface intersecting any proposed top-of-slope or slope face development. The critical failure surface is the estimated failure surface with the lowest calculated FS intersecting the development.



4.1 SLOPE OBSERVATIONS AND AERIAL PHOTO REVIEW

On July 27, 2016, Bartek Ryczywolski, P.Eng., of ParklandGEO visited the site to visually inspect the slope below the proposed residence. The slope face on the property was stripped of topsoil and a recently constructed dry detention pond was present in the southwest corner of the property. The natural slope to the west of the property was vegetated with native grasses and brush, and partially treed. Bedrock outcrops and an erosional channel were also present on the natural slope face. Photographs of the slope are shown on Figures 9 and 10.

Based on visual observations, there was no evidence of deep-seated movement or slumping of the slope face. There were also no signs of springs or other natural groundwater features on the slope face which might impact the slope. However, the possibility of seasonal seeps or springs cannot be totally discounted under all conditions. The slope crest was very rounded indicating that the slope is relatively mature.

Based on a review of historical aerial photographs from 1949, 1966, 1979, 2000, 2008 and 2016, there was no evidence of former distress or landslide activity and the slope appeared to be stable. Natural drainage features and other indications of erosion were visible in the aerial photographs.

4.2 CROSS SECTIONAL PROFILES

Four cross sectional profiles were surveyed by ParklandGEO at the locations shown on the Contour Plan, Figure 3. The locations of the profiles were chosen based on site reconnaissance and a review of the topographic plan of the site provided by McDowell & Associates Inc.

4.3 SLOPE SOIL PROFILES

An idealized soil profile was developed for the stability analysis of each cross sectional profile. The soil profiles were inferred from the soils encountered at the borehole and test pit locations. A partially saturated slope face, representative of perched groundwater in the upper soils was considered in the analysis. The soil profiles are shown on Figures 4 to 7.

4.4 STABILITY ANALYSIS

The stability analysis was carried out using the *Geostudio 2012 Slope/W* computer program to evaluate the factors of safety for the representative slope profiles. The FS was calculated using the Morgenstern-Price Method and a variety of assumed parameters. The following soil parameters were estimated:



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SOIL PARAMETERS FOR STABILITY ANALYSIS										
Soil	Unit Weight (kN/m³)	Cohesion, c' (kPa)	Phi' (Degrees)							
Clay Till	20.0	2 - 5	29							
Gravel	21.5	0	38							
Sand and Silt	21.0	0	35							
Weathered Bedrock	22.0	5 - 10	35							

TABLE 1 SOIL PARAMETERS FOR STABILITY ANALYSIS

For long term stability, effective soil parameters and a predicted long term pore pressure/ groundwater condition were used in the analysis. Pore pressure/groundwater conditions were modelled by using the pore pressure ratio (Ru), where Ru = 0 represents a fully drained slope and Ru = 0.5 represents a fully saturated slope.

The first stage of the analysis was to model the slope stability under unsaturated conditions represented by an Ru < 0.2. For long term stability, it was assumed that the stability of the slope would be adversely affected by a saturated slope face simulated by an Ru of 0.4 to 0.5. This saturated condition is typical to possible weather and development impacts such as; heavy snow melt/precipitation, landscape watering, and possible service leaks or pipe breaks. A number of failure surfaces from the analysis are shown on Figures 4 to 7.

4.5 STABILITY ASSESSMENT

Based on the present slope configuration, vegetation cover, and soil moisture condition, the slope below the proposed residence appears to be stable with a long term FS of more than 1.5. The FS against a small shallow slump-type failure might fall to about 1.0 if the slope face were allowed to become saturated or over-steepened. The FS of the failure surface intersecting the proposed residence is more than 3.0, which is considered to be very stable and acceptable for a permanent structure.

Based on site observations and a review of historical aerial photographs, there was no evidence of deep-seated movement or slumping of the slope face, suggesting that the slope is mature and has not been subject to sliding in recent history. Saturation or over-steepening of the upper soils, leading to shallow slumping is considered to be the most likely mode of slope failure at this site.

5.0 SLOPE RECOMMENDATIONS

The residence has been proposed at the crest of a southwest-facing slope. The construction of the residence is not expected to have a significant impact on the stability of the slope. The potential for a major slope movement is very low under present normal conditions with reasonable variation.



Any site grading or stock piling on or near the slope should not be undertaken without a detailed review by a qualified geotechnical engineer. Any proposal to move the proposed location of the residence should also be reviewed.

6.0 GENERAL SLOPE CARE

As discussed above, the slope face may be subject to minor surficial slumping. Slope face stability is influenced by precipitation, surface erosion, groundwater, and soil moisture conditions. In order to reduce the possibility of surficial slumping, the slope should be kept well vegetated. It is very important that site development does not initiate any detrimental changes to the subsurface conditions and slope geometry. The following general recommendations are provided:

- 1. Permanent removal of vegetation from the slope is not recommended and the growth of new vegetation is encouraged. New vegetation for this site should be selected from native species with deep root systems that can grow with a minimum of watering.
- 2. It is recommended that exposed soils be vegetated soon after site grading is complete. Leaving graded areas of the site unvegetated for extended periods of time will cause increased infiltration into the slope, resulting in the saturation of the upper soils.
- 3. Erosion control measures should be implemented as necessary. If required, features to carry concentrated flows over the crest should be engineered.
- 4. If underground sprinklers, decorative water features, or swimming pools are proposed, they should be properly designed in consultation with qualified engineers and should be provided with leak detection and control systems.
- 5. Excessive watering of vegetated areas and trees on or near the slope should be avoided.
- 6. Under no circumstances should fill or construction debris be disposed of over the slope crest or on the slope face.
- 7. Discharge from roof leaders and weeping tile systems should be directed away from the slope.

The general recommendations in this section are common sense actions to undertake or avoid in order to minimize potential disturbance to the slope. These recommendations are not considered to be essential to the safety of the proposed development, but it is considered prudent to follow these recommendations to maintain a low risk to the development. These general recommendations may be subject to site specific modifications based on a review by a qualified geotechnical engineer.



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7.0 CLOSURE

This report is based on the findings in six boreholes and two test pits, soil testing, site reconnaissance, and a review of available information. If new information or different subsoil and groundwater conditions are encountered, this office must be notified and recommendations submitted herein will be reviewed and revised as required. This report has been prepared for the exclusive use of **McDowell & Associates Inc.**, and their approved agents, for the specified application to the proposed residence at 242253 Westbluff Road, in Rocky View County, Alberta. It has been prepared in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made. Use of this report is subject to acceptance of the attached General Terms, Conditions and Limitations.

We trust that this information meets with your present requirements. If you have any questions please contact our office.

Respectfully submitted, **PARKLAND GEOTECHNICAL CONSULTING LTD.** APEGA Permit #07312



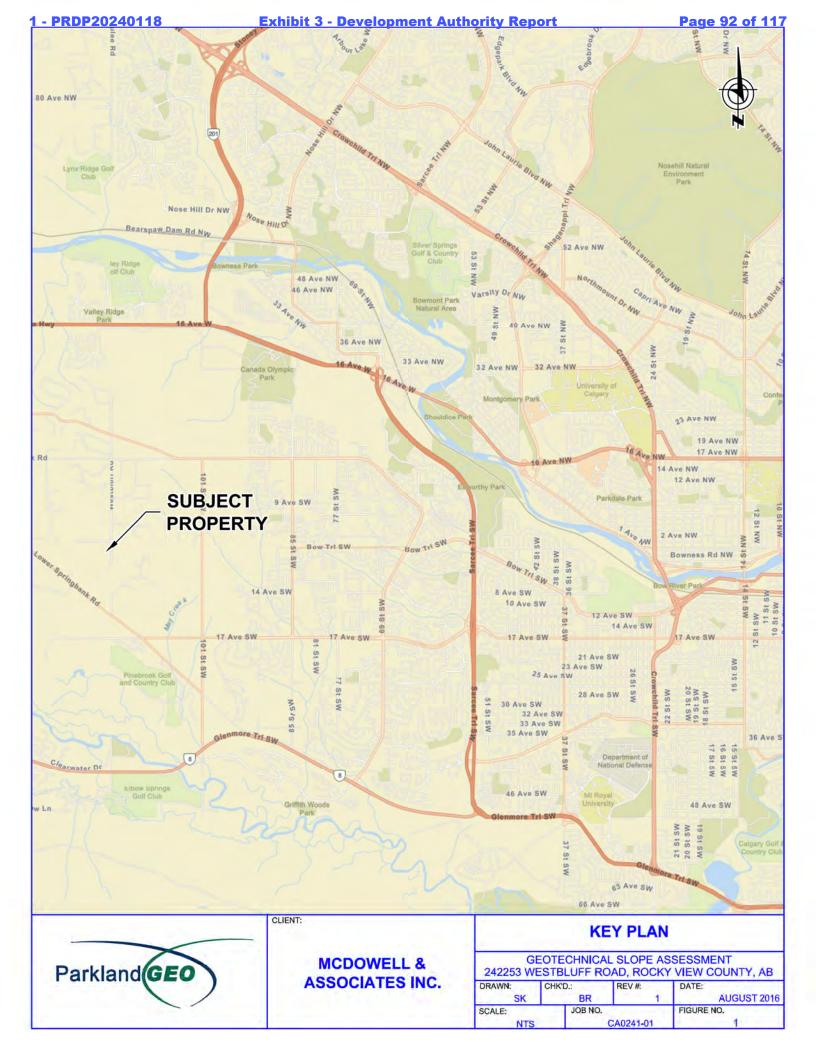
Bartek Ryczywolski, P.Eng. Geotechnical Engineer

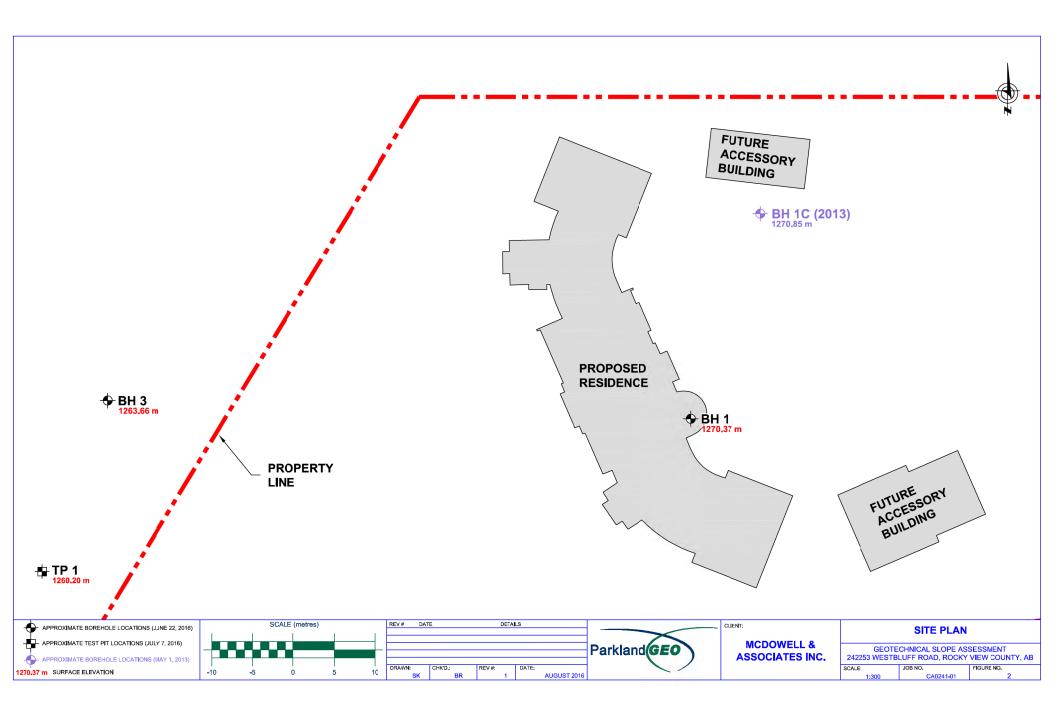
Reviewed By: Ramon Facundo, P.Eng.

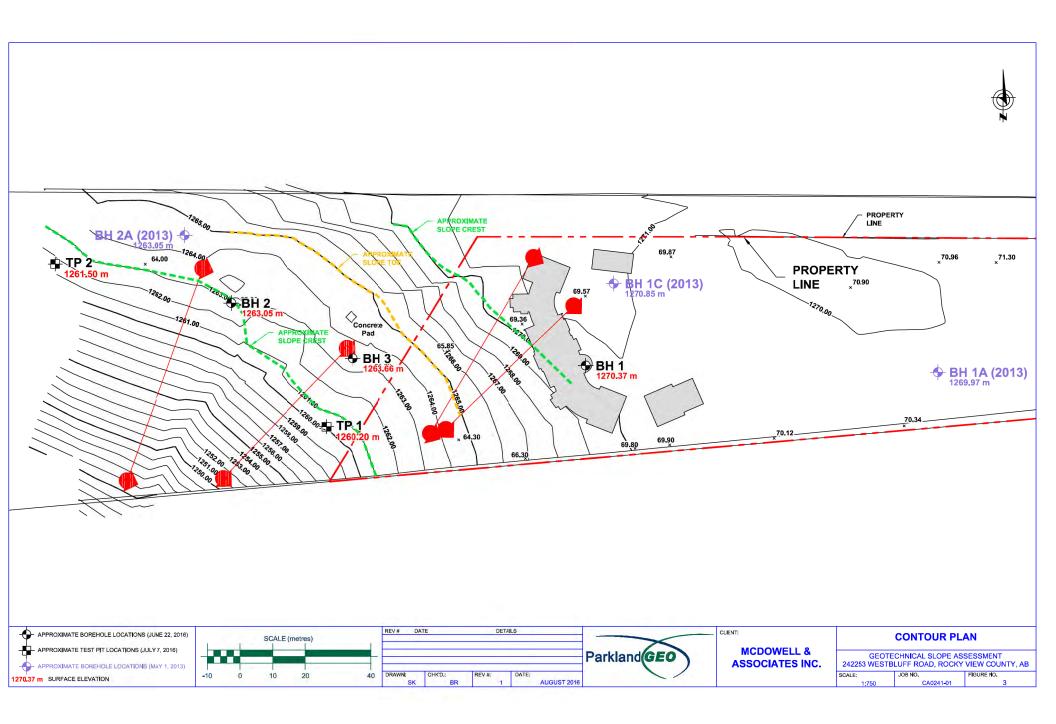
attach / Figure 1 - Key Plan Figure 2 - Site Plan Figure 3 - Contour Plan Figures 4 to 7 - Slope Profiles A to D Figure 8 - 2016 Aerial Photograph Figures 9 and 10 - Site Photographs Borehole and Test Pit Logs Soil Test Results Explanation of Terms and Symbols General Terms, Conditions and Limitations

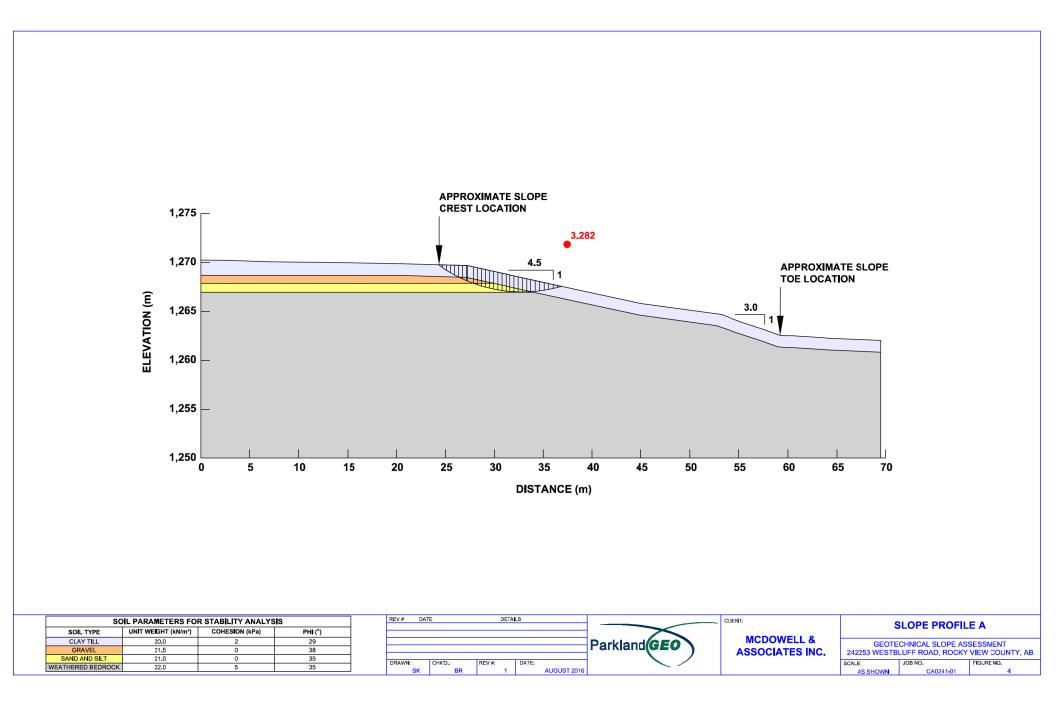
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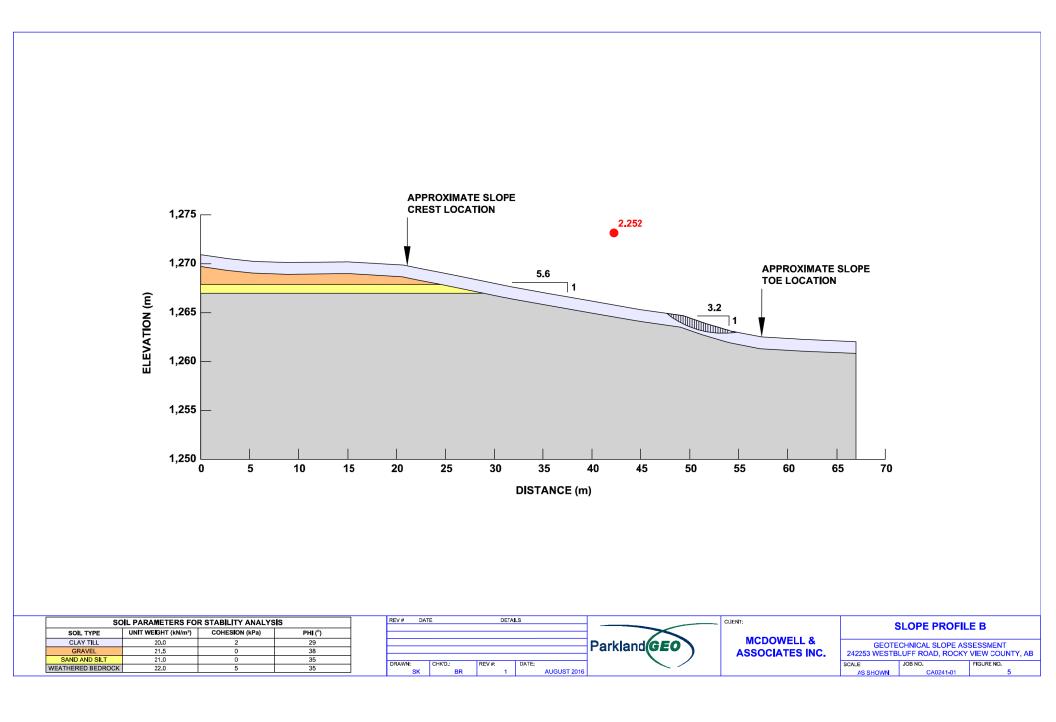


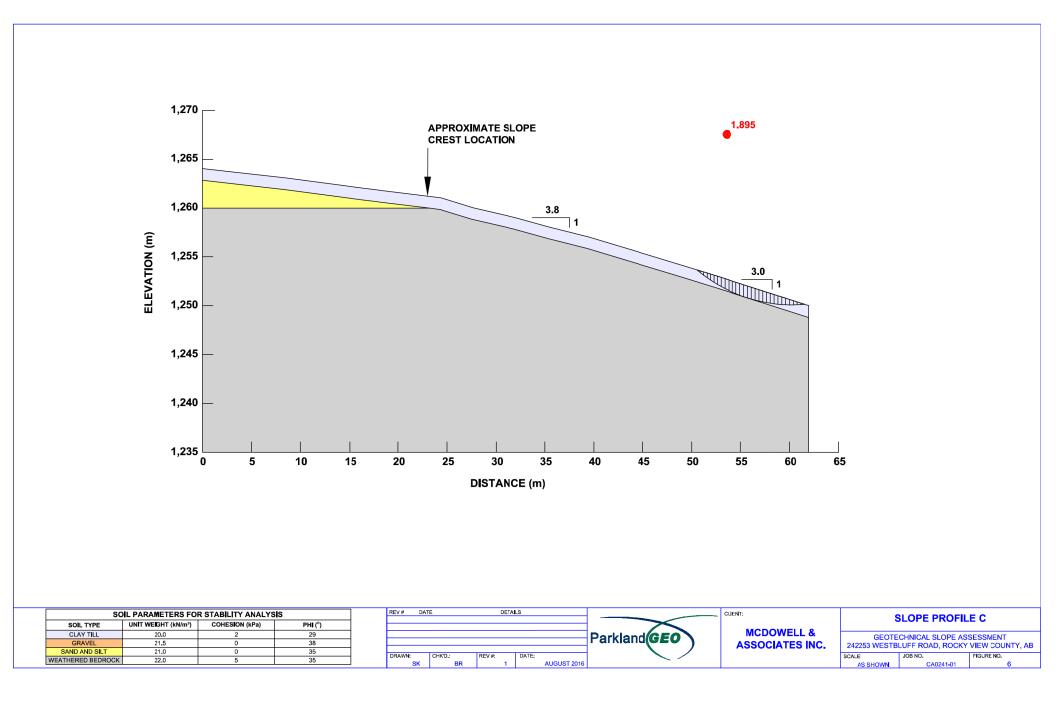


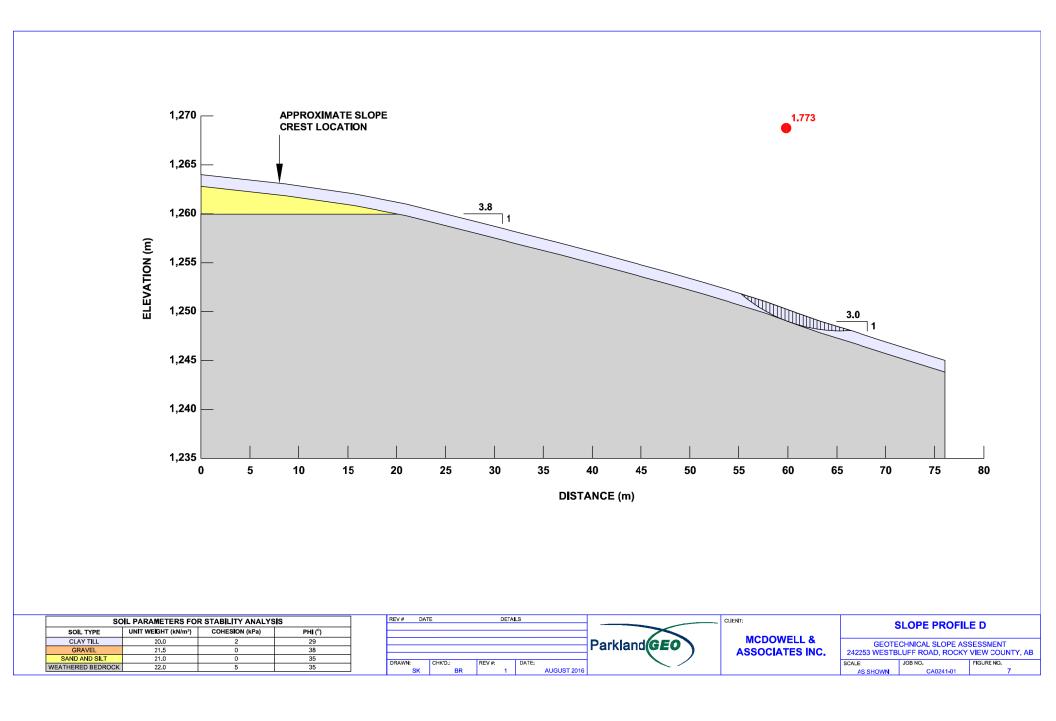




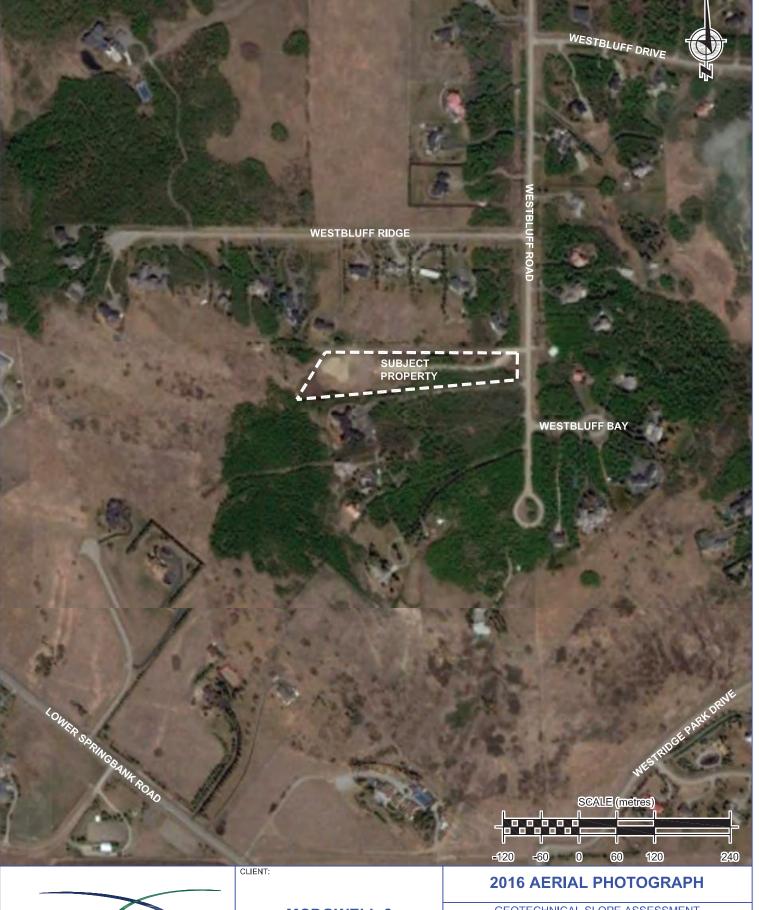








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MCDOWELL & ASSOCIATES INC.

242253 WESTBLUFF ROAD, ROCKY VIEW COUNTY, AB										
DRAWN:	CHK'	D.:	REV #:	DATE:						
SK		BR	1		AUGUST 2016					
SCALE:		JOB NO.		FIGURE NC).					
1:6000		С	A0241-01		8					



June 22, 2016 - Location of proposed residence. Facing west.



June 22, 2016 - Geotechnical drilling of Borehole 1. Facing southeast.

	CLIENT:	SITE PHOTOGRAPHS					
Parkland	MCDOWELL & ASSOCIATES INC.	GEOTECHNICAL SLOPE ASSESSMENT 242253 WESTBLUFF ROAD, ROCKY VIEW COUNTY, AE					
		DRAWN: CHK'D.:		REV #:	DATE:		
		SK	BR	1	A	AUGUST 2016	
		SCALE:	JOB NO.		FIGURE NO).	
			C	A0241-01		9	



July 27, 2016 - Slope below the proposed residence. Facing southeast.



July 27, 2016 - Treed slope face. Facing southeast.

	CLIENT:	SITE PHOTOGRAPHS					
Parkland GEO	MCDOWELL &	GEOTECHNICAL SLOPE ASSESSMENT 242253 WESTBLUFF ROAD, ROCKY VIEW COUNTY,					
	ASSOCIATES INC.	DRAWN:	CHK'D.:	REV #:	DATE:		
		SK	BR	1	AUGUST 2016		
		SCALE:	JOB NO.		FIGURE NO.		
				CA0241-01	10		

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Exhibit 3 - Development Authority Report

Page 102 of 117 BOREHOLE NO.: 1



CLIENT: McDowell & Associates Inc. SITE: 242253 Westbluff Road NOTES: Geotechnical Slope Assessment

PROJECT NO.: CA0241 BH LOCATION:

											_
Depth (m)	SUBSURFACE PROFILE	Symbol	Moisture Content (Wp X WI) 15 45	Type	Sample No			Count	Comments	Well Completion Details	Elevation (m)
0	GROUND SURFACE Till Clay, silty, little gravel, trace sand, stiff, medium plastic, brown, occasional cobbles, occasional rust stains, occasional coal inclusions,		2	G	1G1	10		268		+-SOLID PVC PIPE +	1270.37 1269.87
1	\moist. Gravel Sandy, trace silt, very dense, fine grained, well graded, grey, frequent cobbles, occasional boulders, damp.		1	G	1G2		124	197 • 216	Grain Size Analysis:		
2-			,1	G	1G3		14	225 225	Gravel = 67 % Sand = 29 % Silt & Clay = 4 %		
3	Sand and Silt Little to some clay, trace gravel, dense, fine grained, poorly graded, brown, occasional rust stains, damp.		6	G	1G4	38			Grain Size Analysis: Gravel = 11 % Sand = 20 % Silt = 43 % Clay = 26 %	SLOTTED PVC PIPE	1267.87
-			9	G	1G5	33	97	53	SO ₄ < 0.10 %	SLOTTED PVC PIPE	1266.97
4	rust stains, damp.	(1) (1) (2) (2) (2) (2) (2) (2) (2) (2) (2) (2	8	G	1G6		77	▲ 65			
5-		<pre>x = x = x = x = x = x = x = x = x = x =</pre>	9	G	1G7		14 111 126				
	Refusal at 5.7 m. 25 mm standpipe installed. Backfilled with soil. Dry upon completion. Dry on July 5, 2016.	4 C X X C X X X X X X X X X X X X X X X	8	G	1G8			>3	00	¥ C	1264.67
	LOGGED BY: BR CONTRACTOR: Great West Dri RIG/METHOD: Truck Mount / Be		-						UND ELEVATION: THING: TING:	1270.37 m	
	DATE: June 22, 2016 CALIBRATION:									PAGE	1 of 1

Page 103 of 117 BOREHOLE NO.: 2



CLIENT: McDowell & Associates Inc. SITE: 242259 Westbluff Road NOTES: Geotechnical Slope Assessment

PROJECT NO.: CA0241 BH LOCATION:

	SUBSURFACE PROFILE										$\widehat{}$
Depth (m)	Description	Symbol	Moisture Content (Wp X WI) 15 45	Type	Sample No		Blow Count BPT 75 225		Comments	Well Completion Details	Elevation (m)
0	GROUND SURFACE Topsoil (200 mm) Organic, black, damp. Till Clay, silty, some gravel, trace sand,					11 18 19					1263.05 1262.85
 1 	very stiff, medium plastic, brown, occasional cobbles, occasional rust stains, occasional coal inclusions, moist. Sand Silty, little to some clay, trace gravel,		8	G	2G1	40 42 42 35				-SOLID PVC PIPE	1262.05
2	dense, fine grained, poorly graded, brown, sandstone inclusions, occasional cobbles, damp.		5	G	2G2	44			Grain Size Analysis:	Such a process such a solution such a sol	
3-	Weathered Bedrock		5	G	2G3	36 39 •	85		Gravel = 12 % Sand = 34 % Silt = 29 % Clay = 25 % SO ₄ < 0.10 %		1259.95
- - - 4-	Sandstone, very dense, brown, damp.		5	G	2G4		84 100 137			-SLOTTED PVC PIPE	
-			6	G	2G5 2G6	_	133	>3	00		1258.25
5	Refusal at 4.8 m. 25 mm standpipe installed. Backfilled with soil. Dry upon completion. Dry on July 5, 2016.										
6- - -											
- - 7											
- - 8-											
	LOGGED BY: BR CONTRACTOR: Great West Dri RIG/METHOD: Truck Mount / Be DATE: June 22, 2016		-						und Elevation: Thing: 'Ing:	 1263.05 m	
	CALIBRATION: PAGE 1 of 1										1 of 1

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CLIENT: McDowell & Associates Inc. SITE: 242259 Westbluff Road NOTES: Geotechnical Slope Assessment

PROJECT NO.: CA0241 BH LOCATION:

BOREHOLE NO.: 3

			i			1				1	
	SUBSURFACE PROFILE		Moisture		_				-		Э Э
Depth (m)	Description	Symbol	(Wp X WI) 15 45	Type	Sample No		BF	Count PT 225	Comments	Well Completion Details	Elevation (m)
0-	GROUND SURFACE					10					1263.66
	Topsoil (180 mm) Organic, black, damp. Till Clay, silty, some gravel, trace sand, very stiff, medium plastic, brown,					10 28 20					1263.48
1-	occasional cobbles, occasional rust stains, occasional coal inclusions, moist. Sand					21 26				SOLID PVC PIPE	1262.46
2-	Salid Silty, little to some clay, trace gravel, compact, fine grained, poorly graded, brown, sandstone inclusions, occasional cobbles, damp.		6	G	3G1	24 34 39			Grain Size Analysis: Gravel = 1 % Sand = 45 % Silt = 33 % Clay = 21 %		
3-	Weathered Bedrock Sandstone, very dense, brown, damp.		3	G	3G2		89 133	3 240		C PIPE	1261.16
-			2	G	3G3	7	70	>3	00	SLOTTED PVC PIPE-	
4			3	G	3G4		91 102 93			HLOTS	
5-			3	G	3G5		•	>3	00		1258.46
- - - 6-	Refusal at 5.2 m. 25 mm standpipe installed. Backfilled with soil. Dry upon completion. Dry on July 5, 2016.										
- 7- -											
- - - 8-											
	LOGGED BY: BR CONTRACTOR: Great West Dr RIG/METHOD: Truck Mount / Bo		-	<u> </u>	<u>.</u>	<u>.</u>			UND ELEVATION: THING: TNG:	1263.66 m	
	DATE: June 22, 2016 CALIBRATION:									PAGE	1 of 1

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CLIENT: McDowell & Associates Inc. SITE: 242259 Westbluff Road NOTES: Geotechnical Slope Assessment

PROJECT NO.: CA0241 TP LOCATION:

TESTPIT NO.: 1

	SUBSURFACE PROFILE					<i>(</i> 0)	<u> </u>
Depth (m)	Description	Symbol	Moisture (Wp X WI) 25 50 75	Type	Sample No	Comments	Elevation (m)
0-	GROUND SURFACE						1260.20
-0	Topsoil (320 mm) Organic, black, moist.						1259.88
_	Sand	ĪŪ.	14	G	1G1		
-	Silty, little clay, dense, fine grained, poorly graded, brown, sandstone \inclusions, moist.						1259.70
-	Weathered Bedrock Sandstone, very dense, brown,		10	G	1G2		1259.30
1	Refusal at 0.9 m. Backfilled with soil. Dry upon completion.						
-	-						
-	_						
2-	-						
-	-						
-							
-							
3-							
-	_						
-							
4-							
-							
-							
-							
5-							
	LOGGED BY: BR					GROUND ELEVATION: 1260.20 m	
	CONTRACTOR: B&M Trenching		d.			NORTHING:	
	METHOD: Rubber Tire Excavate	or				EASTING:	
	DATE: July 7, 2016						
	CALIBRATION:					PAGE	1 of 1

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CLIENT: McDowell & Associates Inc. SITE: 242259 Westbluff Road NOTES: Geotechnical Slope Assessment

PROJECT NO.: CA0241 TP LOCATION:

TESTPIT NO.: 2

	SUBSURFACE PROFILE						Ê
Depth (m)	Description	Symbol	Moisture (Wp X WI) 25 50 75 I I I	Type	Sample No	Comments	Elevation (m)
0-	GROUND SURFACE	24:34					1261.50
-	Topsoil (260 mm) Organic, black, moist.	22 22 21 21 21 21 22 22 23 22 24 2					1261.24
-	Till Clay, silty, some sand, little gravel, very stiff, low to medium plastic, brown, occasional cobbles, occasional boulders, occasional rust stains, occasional coal inclusions,		12	G	2G1		
1-	moist.		11	G	2G2		
_			11	G	2G3		
2-			11	G	2G4		
-			12				
			•	G	2G5		1258.90
	Weathered Bedrock Sandstone, very dense, brown,		9	G	2G6		1258.70
	occasional rust stains, damp.						
3-	End of test pit at 2.8 m. Backfilled with soil.						
-	Dry upon completion.						
-							
4-							
-							
-							
5-							
	LOGGED BY: BR			1		GROUND ELEVATION: 1261.50 m	
	CONTRACTOR: B&M Trenchir	ng L	td.			NORTHING:	
	METHOD: Rubber Tire Excava					EASTING:	
	DATE: July 7, 2016						
	CALIBRATION:					PAG	E 1 of 1

RD	RDP20240118 Exhibit 3 - Development Auth					thority Report Page 107 of 117				
	Parkland GEO		CLIENT: Ms. Wend		rtridge	ł		HOLE NO.: 1A	-) (
			SITE: Partridge Pa	rcei			PROJECT NO.: CA0004-REV BH LOCATION:			
	SUBSURFACE PROFILE				0		S		(L)	
Depth (m)	Description	Symbol	Moisture (Wp X WI) 25 50 75	Type	Sample No	SPT (N)	Comments	Well Completion Details	Elevation (m)	
0-	GROUND SURFACE	204: 34						*	1269.97	
-	Topsoil (270 mm) Organic, black, moist.			•	1AU1				1269.70	
-	Till Clay, silty, little gravel, trace sand,						Grain Size Analysis: Gravel = 14 %	PVC		
_	very stiff, medium plastic, brown, occasional cobbles, occasional rust stains, occasional coal inclusions, moist.			•	1AU2		Sand = 5 % Silt = 39 % Clay = 42 %	H → Solid Pvc Pipe	1269.21	
1—	Gravel Sandy, trace silt, dense, fine grained,		3	G	1AG3		-			
-	well graded, grey, occasional cobbles, occasional boulders, damp.						-		1268.77	
-	- frequent cobbles, very dense at 1.2 m.							−SL		
-										
-				G	1401		-	SLOUGH		
2–			5		1AG4		-	SL 	1267.87	
	25 mm standpipe installed. Backfilled with auger cuttings. Slough to 1.5 m. Dry upon completion. Dry on May 16, 2013. Dry on July 5, 2016.									
	LOGGED BY: BR		<u> </u>	1	<u> </u>		ROUND ELEVATION	l: 1269.97 m	1	
	CONTRACTOR: Earth Drilling RIG/METHOD: Truck Mount /						IORTHING: ASTING:			
	DATE: May 1, 2013 CALIBRATION:							PAGE	1 of 1	

			OTES:	1				OCATION:	
Depth (m)	SUBSURFACE PROFILE Description	Symbol	Moisture (Wp X WI) 25 50 75	Type	Sample No	SPT (N)	Comments	Well Completion Details	Elevation (m)
0-	GROUND SURFACE	24:34		_			-		1270.8
_	Topsoil (280mm) Organic, black, moist.	2 25 2 21 2 2 25 2 2 25 2 11 11		•	1CU1			573 575 T	1270.5
_	Till Clay, silty, some gravel, trace sand, very stiff, medium plastic, brown, occasional cobbles, occasional boulders, occasional rust stains, occasional coal inclusions, moist.			Ī	1CU2		Grain Size Analysis: Gravel = 21 % Sand = 7 % Silt = 34 %	 SLOTTED	
1—			10	•	1CU3		Clay = 38 % SO ₄ < 0.10 %		1269.6
_	<i>Gravel</i> Sandy, trace to little silt, very dense, fine grained, well graded, grey, occasional cobbles, occasional boulders, damp.			G	1CG4		- Grain Size Analysis: Gravel = 48 % Sand = 35 %	LOTTED	
- 2-	- frequent cobbles at 1.8 m.		3				Silt & Clay = 17 % SO ₄ < 0.10 %	-SLOTTED	1269.0
-	Auger refusal at 2.1 m. 25 mmstandpipe installed. Backfilled with auger cuttings. Dry upon completion. Dry on May 16, 2013. Standpipe destroyed on July 5, 2016.							¥ <u>K3</u>	1200.1
3-									
_									
_									
4-									
_									
_									
_									
_									
5-									

ige DATE: May 1, 2013 CALIBRATION:

PAGE 1 of 1

1 - PRDP20240118Exhibit 3 - Development Authority ReportPage 108 of 117

BOREHOLE NO.: 1C

1 - PRDP20240118

Exhibit 3 - Development Authority Report

Page 109 of 117 BOREHOLE NO.: 2A



CLIENT: Ms. Wendy Partridge SITE: Partridge Parcel NOTES:

PROJECT NO.: CA0004-REV BH LOCATION:

	SUBSURFACE PROFILE								Ê
Depth (m)	Description	Symbol	Moisture (Wp X WI) 25 50 75	Type	Sample No	SPT (N)	Comments	Well Completion Details	Elevation (m)
0-	GROUND SURFACE	24.34		_					1264.35
-	Topsoil (400mm) Organic, black, moist.	2 25 2 <u>2</u> <u>2</u> 2 25 2 <u>2</u> <u>25</u> 2 <u>2</u> <u>25</u> 2 <u>2</u> 25 2 <u>2</u>		1	2AU1				1263.95
_	Till Clay, silty, little gravel, trace sand, very stiff, low to medium plastic, brown, occasional cobbles, occasional boulders, occasional rust			•	2AU2			SOLID PVC PIPE	
1-	stains, occasional coal inclusions, moist.		12		2AU3		Grain Size Analysis: Gravel = 1 % Sand = 2 % Silt = 41 % Clay = 56 %	Control PVC PIPE Solud PVC PIPE Solud PVC PIPE BackFilter Mith Auger Cuttings	
_				•	2AU4			* AUGER C	
_			7		2AD1	19		E B B MITH /	
2-	Sand		•					PVC PIP	1262.25
_	Silty, little clay, trace gravel, dense, fine grained, poorly graded, brown, sandstone inclusions, damp.			G	2AG3			SLOTTED PVC PIPE	
3	End of hole at 3.0 m. 25 mm standpipe installed. Backfilled with auger cuttings. Dry upon completion. Dry on May 16, 2013. Dry on July 5, 2016.		7						1261.35
4-									
_									
-									
5-									
	LOGGED BY: BR CONTRACTOR: Earth Drilling C RIG/METHOD: Truck Mount / So DATE: May 1, 2013					Ν	ROUND ELEVATION ORTHING: ASTING:	J: 1264.35 m	
	CALIBRATION:							PAGE	1 of 1



SIEVE PARTICLE-SIZE ANALYSIS

ASTM C136

PROJECT: Geotechnical Slope Assessment

PROJECT#: CA0241

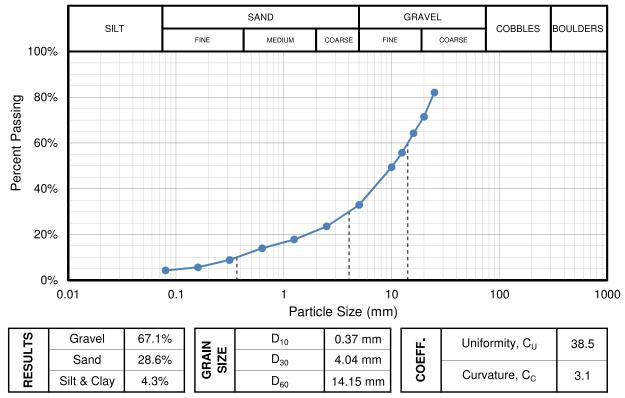
CLIENT: McDowell & Associates Inc.

SOIL DESCRIPTION: Sandy Gravel

SAMPLED: June 22, 2016 TESTED: June 28, 2016 SAMPLE ID: 1G3

DEPTH: 1.5 m

NG	Sieve Size (mm)	Mass Retained on Sieve (g)	Cumulated Mass Retained (g)	Total Mass Finer (g)	Percent Passing
PASSING	80.0				
ΡA	63.0				
L L	50.0				
PERCENT	40.0				
Ë	25.0	260.0	260.0	1188.8	82.1%
	20.0	153.2	413.2	1035.6	71.5%
AND	16.0	104.8	518.0	930.8	64.2%
	12.5	123.9	641.9	806.9	55.7%
	10.0	91.0	732.9	715.9	49.4%
W	5.0	239.4	972.3	476.5	32.9%
JRI	2.5	135.7	1108.0	340.8	23.5%
MEASUREMENTS	1.25	83.1	1191.1	257.7	17.8%
ME/	0.630	55.3	1246.4	202.4	14.0%
I S	0.315	74.0	1320.4	128.4	8.9%
MASS	0.160	46.8	1367.2	81.6	5.6%
2	0.080	19.7	1386.9	61.9	4.3%
	Pan	1.3	1388.2	60.6	0.0%





PROJECT: Geotechnical Slope Assessment

PROJECT#: CA0241

CLIENT: McDowell & Associates Inc.

SOIL DESCRIPTION: Sand and Silt

PARTICLE-SIZE ANALYSIS

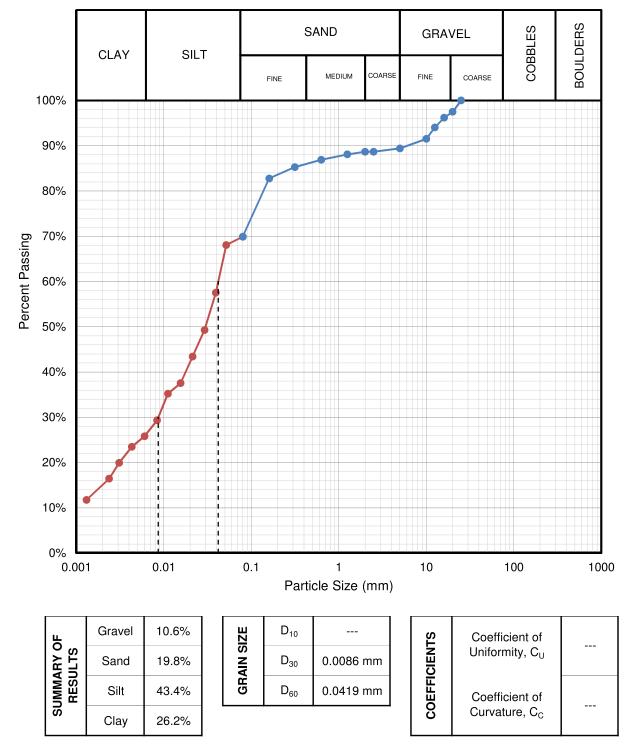
ASTM D422

SAMPLE DATE: June 22, 2016

TEST DATE: June 27, 2016

SAMPLE ID: 1G4

DEPTH: 2.3 m





PROJECT: Geotechnical Slope Assessment

PROJECT#: CA0241

CLIENT: McDowell & Associates Inc.

SOIL DESCRIPTION: Silty Sand

PARTICLE-SIZE ANALYSIS

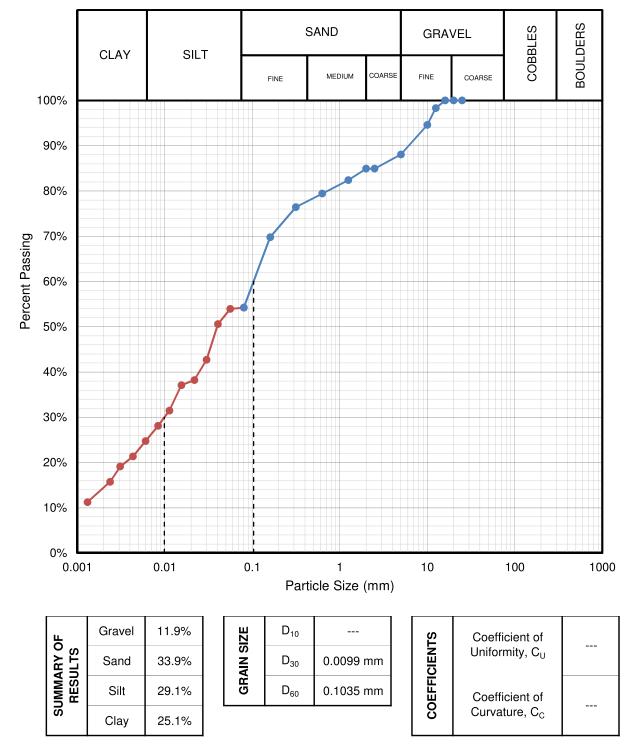
ASTM D422

SAMPLE DATE: June 22, 2016

TEST DATE: June 27, 2016

SAMPLE ID: 2G3

DEPTH: 2.3 m





PROJECT: Geotechnical Slope Assessment

PROJECT#: CA0241

CLIENT: McDowell & Associates Inc.

SOIL DESCRIPTION: Silty Sand

PARTICLE-SIZE ANALYSIS

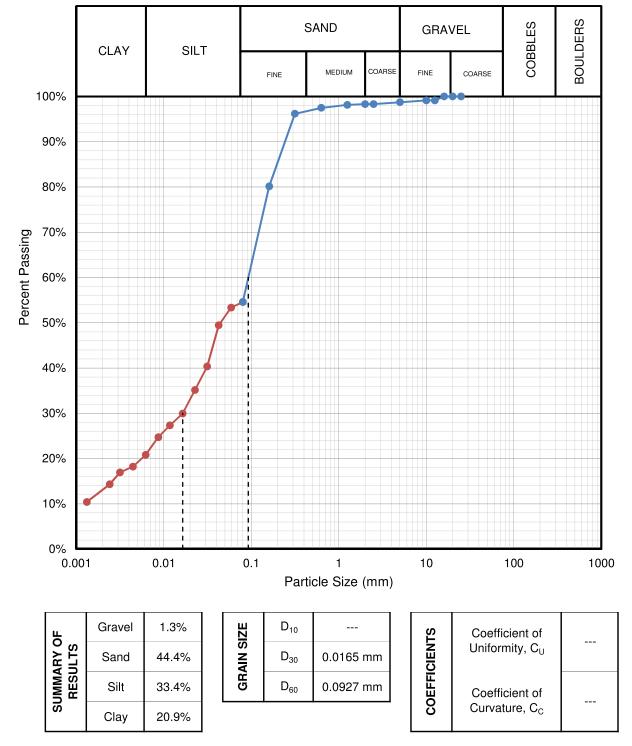
ASTM D422

SAMPLE DATE: June 22, 2016

TEST DATE: June 27, 2016

SAMPLE ID: 3G1

DEPTH: 1.5 m





Project:Geotechnical Slope AssessmentSubject:Geotechnical Testing - Soil Sulphate Test ResultsProject #:CA0241Date:

		Soil Su	Iphate Te	st Results	5				
Laboratory:	Parkland Geot	echnical							
Sample #: 1G4	4			ample #:					
Borehole: 1				orehole:					
Depth: 2.3				epth:					
Result (% Sulp	hate): 0.04		Re	Result (% Sulphate):					
Sample #: 2G3	3		Sa	ample #:					
Borehole: 2			Bo	orehole:					
Depth: 2.3	m		De	epth:					
Result (% Sulp	hate): 0.04		Re	esult (% Sulphat	e):				
Sample #:			Sa	ample #:					
Borehole:				brehole:					
Depth:			De	epth:					
Result (% Sulp	hate):			Result (% Sulphate):					
Sample #:				Sample #:					
Borehole:				orehole:					
Depth:				epth:					
Result (% Sulp	hate):		Re	Result (% Sulphate):					
Sample #:			Sa	ample #:					
Borehole:			Bo	Borehole:					
Depth:			De	Depth:					
Result (% Sulp	hate):		Re	Result (% Sulphate):					
Comments:									
REQUI	REMENTS FO	R CONCRETE S	UBJECTED TO	SULPHATE AT	FACK (CSA/CAN-A	\23.1-14)			
EXPOSURE CLASSIFICATION	DEGREE OF EXPOSURE	WATER-SOLUBLE SULPHATE(SO4) IN SOIL SAMPLE, %	SULPHATE(SO4) IN GROUND WATER SAMPLES, mg/L	MINIMUM SPECIFIED 56-DAY COMPRESSIVE STRENGTH, MPa	MAXIMUM WATER/CEMENTING MATERIALS RATIO	PORTLAND CEMENT TO BE USED			
S-1	Very Severe	over 2.0	over 10,000	35	0.4	HS			
S-2	Severe	0.20 to 2.0	1 500 to 10 000	32	0.45	HS			
S-3	Moderate	0.10 to 0.20	150 to 1 500	30	0.5	MS or HS			

Tech: JB Chkd: BR



Exhibit 3 - Development Authority Report Page 115 of 117 THE PARKLANDGEO CONSULTING GROUP EXPLANATION OF TERMS AND SYMBOLS

The terms and symbols used on the borehole logs to summarize the results of the field investigation and subsequent laboratory testing are described on the following two pages.

The borehole logs are a graphical representation summarizing the soil profile as determined during site specific field investigation. The materials, boundaries, and conditions have been established only at the borehole location at the time of drilling. The soil conditions shown on the borehole logs are not necessarily representative of the subsurface conditions elsewhere across the site. The transitions in soil profile usually have gradual rather than distinct unit boundaries as shown on the borehole logs.

1. **PRINCIPAL SOIL TYPE –** The major soil type by weight of material or by behaviour.

Material	Grain Size				
Boulders	Larger than 300 mm				
Cobbles	75 mm to 300 mm				
Coarse Gravel	19 mm to 75 mm				
Fine Gravel	5 mm to 19 mm				
Coarse Sand	2 mm to 5 mm				
Medium Sand	0.425 mm to 2 mm				
Fine Sand	0.075 mm to 0.425 mm				
Silt & Clay	Smaller than 0.075 mm				

2. **DESCRIPTION OF MINOR SOIL TYPE –** Minor soil types are identified by weight of minor component.

Percent	Descriptor
35 to 50	and
20 to 35	some
10 to 20	little
1 to 10	trace

3. RELATIVE STRENGTH OF COARSE GRAINED SOIL – The following terms are used relative to Standard Penetration Test (SPT), ASTM D1586, N value for blows per 300 mm.

Description	N Value
Very Loose	Less than 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	Over 50

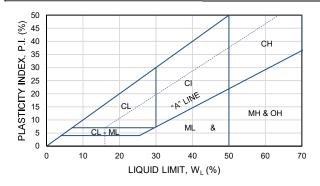
4. CONSISTENCY OF FINE GRAINED SOILS – The following terms are used relative to undrained shear strength and Standard Penetration Test (SPT), ASTM D1586, N value for blows per 300 mm. It is noted that this correlation needs to be used with caution as the correlation is only very approximate.

Description	Undrained Shear Strength, C _u (kPa)	N Value
Very Soft	Less than 12	Less than 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 150	15 to 30
Hard	Over 150	Over 30



Exhibit 3 - Development Authority Report Page 116 of 117 THE PARKLANDGEO CONSULTING GROUP EXPLANATION OF TERMS AND SYMBOLS

MODIFIED UNIFIED CLASSIFCATION SYSTEM FOR SOILS							
MAJOR DIVISION		GROUP SYMBOL	GRAPH SYMBOL	TYPICAL DESCRIPTION	LABORATORY CLASSIFICATION CRITERIA		
COARSE GRAINED SOILS (MORE THAN HALF BY WEIGHT LARGER THAN NO. 200 SIEVE)	GRAVELS MORE THAN HALF COARSE GRAINS LARGER THAN NO. 4 SIEVE	CLEAN GRAVELS	GW		WELL GRADED GRAVELS, GRAVEL- SAND MIXTURE, LITTLE OR NO FINES	$C_U = \frac{D_{60}}{D_{10}} > 4 \text{ AND } Cc = \frac{(D_{30})^2}{D_{10} X D_{60}} = 1 \text{ to } 3$	
		(LITTLE OR NO FINES)	GP	0.00	POORLY GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS	
		DIRTY GRAVELS (WITH SOME FINES)	GM		SILTY GRAVELS, GRAVEL-SAND- SILT MIXTURES	CONTENT OF FINES EXCEEDS 12%	ATTERBERG LIMITS BELOW "A" LINE OR P.I. LESS THAN 4
			GC		CLAYEY GRAVELS, GRAVEL-SAND- CLAY MIXTURES		ATTERBERG LIMITS ABOVE "A" LINE AND P.I. GREATER THAN 7
	SANDS MORE THAN HALF FINE GRAINS SMALLER THAN NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)	sw		WELL GRADED SANDS, GRAVELLY SANDS WITH LITTLE OR NO FINES	$C_U = \frac{D_{60}}{D_{10}} > 6 \text{ AND Cc} = \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$	
			SP		POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS	
		DIRTY SANDS (WITH SOME FINES)	SM		SILTY SANDS, SAND-SILT MIXTURES	CONTENT OF FINES EXCEEDS 12%	ATTERBERG LIMITS BELOW "A" LINE OR P.I. LESS THAN 4
			SC		CLAYEY SANDS, SAND-CLAY MIXTURES		ATTERBERG LIMITS ABOVE "A" LINE AND P.I. GREATER THAN 7
FINE-GRAINED SOILS (MORE THAN HALF BY WEIGHT PASSES NO. 200 SIEVE)	SILTS BELOW "A" LINE NEGLIGIBLE ORGANIC CONTENT	W _L < 50%	ML		INORGANIC SILTS & VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY		
		W _L > 50%	МН		INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS, FINE SANDY OR SILTY SOILS		
	CLAYS ABOVE "A" LINE NEGLIGIBLE ORGANIC CONTENT	W _L < 30%	CL	////	INORGANIC CLAYS OF LOW PLASTICITY, GRAVELLY, SANDY, OR SILTY SOILS		
		30% < W _L < 50%	CI	1/1	INORGANIC CLAYS OF MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS	CLASSIFICATION IS BASED UPON PLASTICITY CHART (SEE BELOW)	
		W _L > 50%	СН		INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS		
	ORGANIC SILTS & CLAYS BELOW'A" LINE	W _L < 50%	OL		ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW AND MEDIUM PLASTICITY		
		W∟ > 50%	он		ORGANIC CLAYS OF HIGH PLASTICITY, ORGANIC SILTS		
HIGHLY ORGANIC SOILS		Pt	90-90 9-99-95 90-90	PEAT AND OTHER HIGHLY ORGANIC SOILS	STRONG COLOR OR ODOR, AND OFTEN FIBROUS TEXTURE		



NOTES ON SOIL CLASSIFICATION AND DESCRIPTION:

- 1. Soil are classified and described according to their engineering properties and behaviour.
- Boundary classification for soil with characteristics of two groups are given combined group symbols (e.g. GW-GC is a well graded gravel sand mixture with clay binder between 5 and 12%).
- Soil classification is in accordance with the Unified Soil Classification System (ASTM D2487) with the exception that an inorganic clay of medium plasticity (CI) is recognized.
- 4. The use of modifying adjectives may be employed to define the estimated percentage range by eight of minor components.

The use of this attached report is subject to the following general terms and conditions.

- STANDARD OF CARE In the performance of professional services, ParklandGEO used the degree of care and skill ordinarily exercised under similar circumstances by reputable members of its profession practicing in the same or similar localities. No other warranty expressed or implied is made in any manner.
- 2. INTERPRETATION OF THE REPORT The CLIENT recognizes that subsurface conditions will vary from those encountered at the location where borings, surveys, or explorations are made and that the data, interpretations and recommendation of ParklandGEO are based solely on the information available to him. Classification and identification of soils, rocks, geological units, contaminated materials and contaminant quantities will be based on commonly accepted practices in geotechnical or environmental consulting practice in this area. ParklandGEO will not be responsible for the interpretation by others of the information developed.
- SITE INFORMATION The CLIENT has agreed to provide all information with respect to the past, present and proposed conditions and use of the Site, whether specifically requested or not. The CLIENT acknowledged that in order for ParklandGEO to properly advise and assist the CLIENT, ParklandGEO has relied on full disclosure by the CLIENT of all matters pertinent to the Site investigation.
- COMPLETE REPORT The Report is of a summary nature and 4. is not intended to stand alone without reference to the instructions given to ParklandGEO by the CLIENT, communications between ParklandGEO and the CLIENT, and to any other reports, writings or documents prepared by ParklandGEO for the CLIENT relative to the specific Site, all of which constitute the Report. The word "Report" shall refer to any and all of the documents referred to herein. In order to properly understand the suggestions, recommendations and opinions expressed by ParklandGEO, reference must be made to the whole of the Report. ParklandGEO cannot be responsible for use of any part or portions of the report without reference to the whole report. The CLIENT has agreed that "This report has been prepared for the exclusive use of the named CLIENT. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. ParklandGEO accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report."

The CLIENT has agreed that in the event that any such report is released to a third party, the above disclaimer shall not be obliterated or altered in any manner. The CLIENT further agrees that all such reports shall be used solely for the purposes of the CLIENT and shall not be released or used by others without the prior written permission of ParklandGEO.

5. LIMITATIONS ON SCOPE OF INVESTIGATION AND WARRANTY DISCLAIMER

There is no warranty, expressed or implied, by ParklandGEO that:

- a) the investigation uncovered all potential geo-hazards, contaminants or environmental liabilities on the Site; or
- b) the Site is entirely free of all geo-hazards or contaminants as a result of any investigation or cleanup work undertaken on the Site, since it is not possible, even with exhaustive sampling, testing and analysis, to document all potential geo-hazards or contaminants on the Site.

The CLIENT acknowledged that:

- a) the investigation findings are based solely on the information generated as a result of the specific scope of the investigation authorized by the CLIENT;
- b) unless specifically stated in the agreed Scope of Work, the investigation will not, nor is it intended to assess or detect potential contaminants or environmental liabilities on the Site;
- c) any assessment regarding geological conditions on the Site is based on the interpretation of conditions determined at specific sampling locations and depths and that conditions may vary between sampling locations, hence there can be no assurance that undetected geological conditions, including soils or groundwater are not located on the Site;
- any assessment is also dependent on and limited by the accuracy of the analytical data generated by the sample analyses;
- e) any assessment is also limited by the scientific possibility of determining the presence of unsuitable geological conditions for which scientific analyses have been conducted; and
- f) the laboratory testing program and analytical parameters selected are limited to those outlined in the CLIENT's authorized scope of investigation; and
- g) there are risks associated with the discovery of hazardous materials in and upon the lands and premises which may inadvertently discovered as part of the investigation. The CLIENT acknowledges that it may have a responsibility in law to inform the owner of any affected property of the existence or suspected existence of hazardous materials and in some cases the discovery of hazardous conditions and materials will require that certain regulatory bodies be informed. The CLIENT further acknowledges that any such discovery may result in the fair market value of the lands and premises and of any other lands and premises adjacent thereto to be adversely affected in a material respect.
- 6. COST ESTIMATES Estimates of remediation or construction costs can only be based on the specific information generated and the technical limitations of the investigation authorized by the CLIENT. Accordingly, estimated costs for construction or remediation are based on the known site conditions, which can vary as new information is discovered during construction. As some construction activities are an iterative exercise, ParklandGEO shall therefore not be liable for the accuracy of any estimates of remediation or construction costs provided.
- 7. LIMITATION OF LIABILITY The CLIENT has agreed that to the fullest extent permitted by the law ParklandGEO's total liability to CLIENT for any and all injuries, claims, losses, expenses or damages whatsoever arising out of or in anyway relating to the Project is contractually limited, as outlined in ParklandGEO's standard Consulting Services Agreement. Further, the CLIENT has agreed that to the fullest extent permitted by law ParklandGEO is not liable to the CLIENT for any special, indirect or consequential damages whatsoever, regardless of cause.
- 8. INDEMNIFICATION To the fullest extent permitted by law, the CLIENT has agreed to defend, indemnify and hold ParklandGEO, its directors, officers, employees, agents and subcontractors, harmless from and against any and all claims, defence costs, including legal fees on a full indemnity basis, damages, and other liabilities arising out of or in any way related to ParklandGEO's work, reports or recommendations.

Subdivision & Development Appeal Board B-1; March 28, 2024

File: PRDP20240118

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement

Applicant: Dean Thomas Design Group (Ryland Cook) Owner: Lang-Hodge, John & Claudine Appellants: Bird, Richard & Cathryn



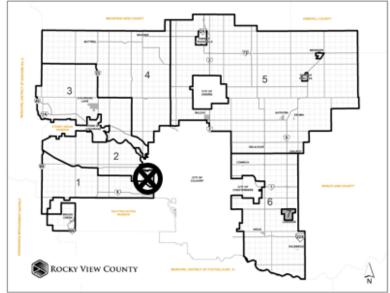
CKY VIEW COUNTY

1 - PRDP20240118

Exhibit 4 - Development Authority Presentation

Land Use and Location Page 2 of 11

- Located approximately 1 mile south of Springbank Road and on the west side of Westbluff Road
- 4.69 acres in area, zoned as Residential, Rural District (R-RUR)
 - Located within Central Springbank Area Structure Plan
 - Surrounded primarily by residential parcels of varying sizes

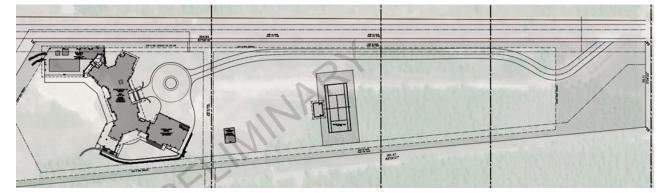


LAND USE MAP NE-18-24-02-05; Division: 2; Roll: 04618044



SITE PLAN

Exhibit 4 - Development Authority Presentation



NE-18-24-02-05;

Application Details

- Single-lot regrading, excavation, and placement of clean fill to accommodate the construction of a new Dwelling, Single Detached and site improvements
- Dwelling requires relaxation to the minimum top-bank setback requirement
 - Location of dwelling was chosen to effectively manage stormwater drainage given the size of the home
 - Meets all maximum building height and minimum setback requirements of the **R-RUR** district
- Subsequent technical reports have been included as prior to release conditions, to ensure the development is technically sound

ROCKY VIEW COUNTY

.

Division: 2; Roll: 04618044



 EAST & WEST
 NE-18-24-02-05;

 ELEVATIONS
 Division: 2; Roll: 04618044



Rocky View County





 NORTH & SOUTH
 NE-18-24-02-05;

 ELEVATIONS
 Division: 2; Roll: 04618044





SITE PHOTOS NE-18-24-02-05; Division: 2; Roll: 04618044



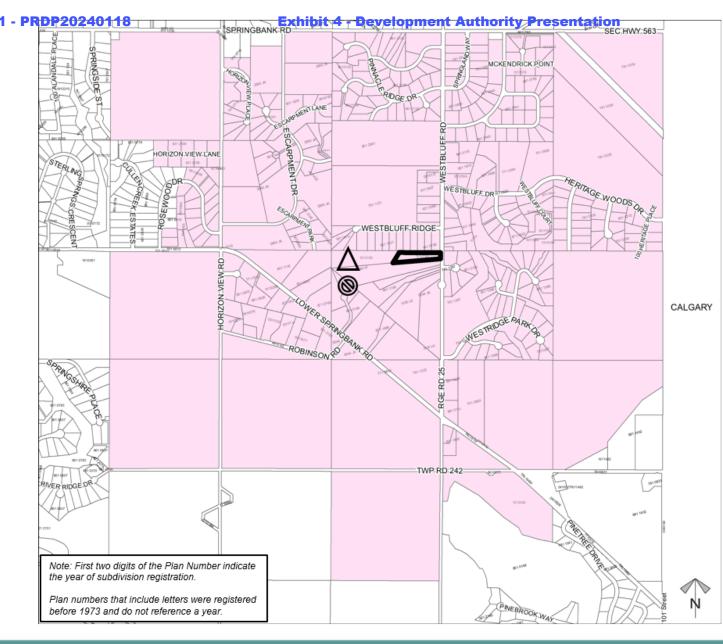
ROCKY VIEW COUNTY



SITE PHOTOS NE-18-24-02-05; CONT'D Division: 2; Roll: 04618044



ROCKY VIEW COUNTY



Page 8 of 11

ROCKY VIEW COUNTY

Landowner Circulation Area

Development Proposal

Single-lot regrading, excavation, and placement of clean fill, for the construction of a dwelling, single detached and site improvements, and relaxation to the minimum top-of-bank setback requirement.



Division: 2 Roll: 04618044 File: PRDP20240118 Printed: Mar 14, 2024 Legal: A portion of NE-18-24-2-W5M

LANDOWNER NE-18-24-02-05; CIRCULATION MAP Division: 2; Roll: 04618044



Reasons for Appeal

- 1. Proposed development will negatively impact the natural elevation/contours of the land, detracting from the rural setting of the surrounding area.
- 2. Proposed building height of the dwelling is incompatible with adjacent parcels.
- 3. Proposed development will negatively impact existing stormwater drainage patterns.
- Proposed development will negatively impact the existing skyline/viewshed of the appellant's home, resulting in decreased land value.
- 5. Requested relaxation to minimum top-of-bank setback requirement will result in the dwelling visually looming over dwellings at the bottom of the slope, including the appellant's dwelling.

REASONS FOR NE-18-24-02-05; APPEAL Division: 2; Roll: 04618044



CKY VIEW COUNTY

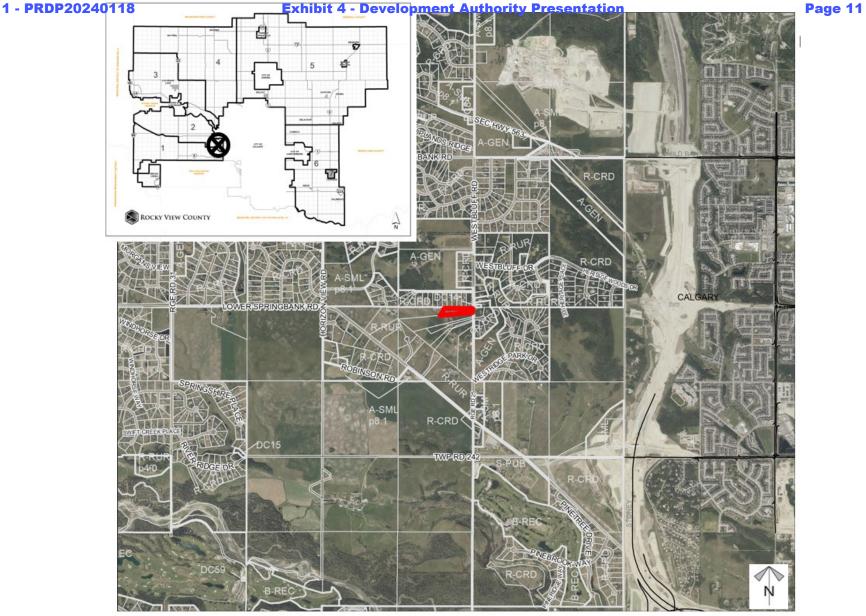
VIEW COUNTY

Development Authority Position

- 1. Proposed changes to existing grades/elevations are necessary to the construction of the dwelling, due to the footprint and location of the dwelling.
- 2. Proposed building height does not require a relaxation, and is compliant with the maximum building height requirement of the Residential, Rural District.
- 3. Proposed location of the home is to effectively manage stormwater drainage given the size of the home. A Site-Specific Stormwater Implementation Plan (SSIP) is included as a prior-to-release condition of approval.
- 4. Proposed dwelling is relatively parallel with other existing homes in the area at the top of the slope. Parcel is well screened via existing trees.
- 5. Minimum top-of-bank setback requirement is implemented to ensure safe placement of dwellings and can be relaxed at the discretion of the Development Authority. The purpose of such regulation is not in respect to form and massing of buildings. Dwelling location will not impact the existing form and massing of the community.

DEVELOPMENT NE-18-24-02-05; AUTHORITY POSITION Division: 2; Roll: 04618044





CONCLUSION

NE-18-24-02-05; Division: 2; Roll: 04618044



Page 11 of 11

From:	PAA SDAB
To:	Appeal Hearing File: 04618044, March 28, 2024 Re : Development Permit #PRDP20240118, Lot 4, Block 2, Plan
Subject:	1512150, NE-18-24-02-05 (242253 Westbluff Road)
Date:	Monday, March 25, 2024 1:16:29 PM

Att: Subdivision and Development Appeal Board for Rockyview County

To whom it may concern:

The south end of our property (Lot 3, Plan 9211421, SE-19-24-02-05; 15 Westbluff Ridge) is directly bordered by the access road to our neighbors' house 242259 Westbluff Road and the top-of-bank portion of property 242253 Westbluff Road (hereinafter referred to as The Development).

Historically stormwater drainage has not been an issue on our property.

The grade of the top-of-bank portion of The Development has been raised at least a couple of times over the years by previous owners.

Another raising of the grade of the top-of-bank portion of The Development is of considerable concern to us, especially since a north-south culvert had to be installed under the elevated access road south of our property in 2018 (following an inspection by Rockyview Community Peace Officer L II-Bylaw/Enforcement Services, Steve Usher) to ensure proper drainage of stormwater.

Please confirm that yet another substantial raising of the grade as proposed for The Development will not cause an adverse material impact on our property with regards to stormwater drainage and/or septic field drainage.

In addition we would like to thank Rockyview County for confirming that The Development will not obstruct existing view channels, will take into account the natural topography and will be consistent with the Residential Development Policies of the ASP (please see: 1 -PRDP20240118 Exhibit 3 - Development Authority Report, Page 4 of 117, 2.9 Residential Development; submitted by: Dominic Kazmierczak, Manager Planning; concurred by: Matthew Boscariol, Executive Director Community Development Services), ensuring that our property will not suffer negative material impacts, e.g. reduced monetary property value.

Sincerely,

Monika & Hagen Schultes 15 Westbluff Ridge Calgary, AB T3Z 3P2 Email:



1109 Olympic Way SE Calgary, Alberta T2G 1B9 **deanthomas.ca**

26 March 2024

Subdivision and Development Appeal Board for Rockyview County 262075 Rocky View Point Rockyview County, Alberta T4A 0X2

Re: PRDP20240118 File: 04618044 242253 Westbluff Road, Rockyview County

To the Subdivision and Development Appeal Board [SDAB],

Thank you for taking the time to preside over our appeal hearing on March 28th, 2024. As the Applicant of the property at 242253 Westbluff Road under appeal, we wish to submit materials to the board prior to the hearing for their consideration, as well as presentation during the hearing. Below is an itemized list of materials which corresponds to the package attached herein. Please review these materials and reach out with any additional question you may have surrounding these items.

List of Submission Materials for Appeal Hearing 04618044 [PRDP20240118, 242253 Westbluff Road]

Massing and placement

- 1. Large format overview of surrounding developments
- 2. Setback measurement of proposed development to appellant's
- 3. Setback measurement of existing developments to appellant's
- 4. 3D Representation of development from appellant's property
- 5. Google Earth Street View of similar view for comparison
- 6. 12.0m Height Restriction Illustration

Site Development

- 7. Approved geotechnical setback of 1270.00.
- 8. Comparison with 1270.00 with proposed development
- 9. Registered document and instrument outline including overland drainage ROW's
- 10. Stormwater management plan pending final review and approval from RVC
- 11. Deep fills report pending final review and approval from RVC

Thank you for taking the time to review these documents. We look forward to discussing further during the hearing.

Ryland Cook Director of Production Dean Thomas Design Group







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Image © 2024 Airbus Image Landsat / Copernicus

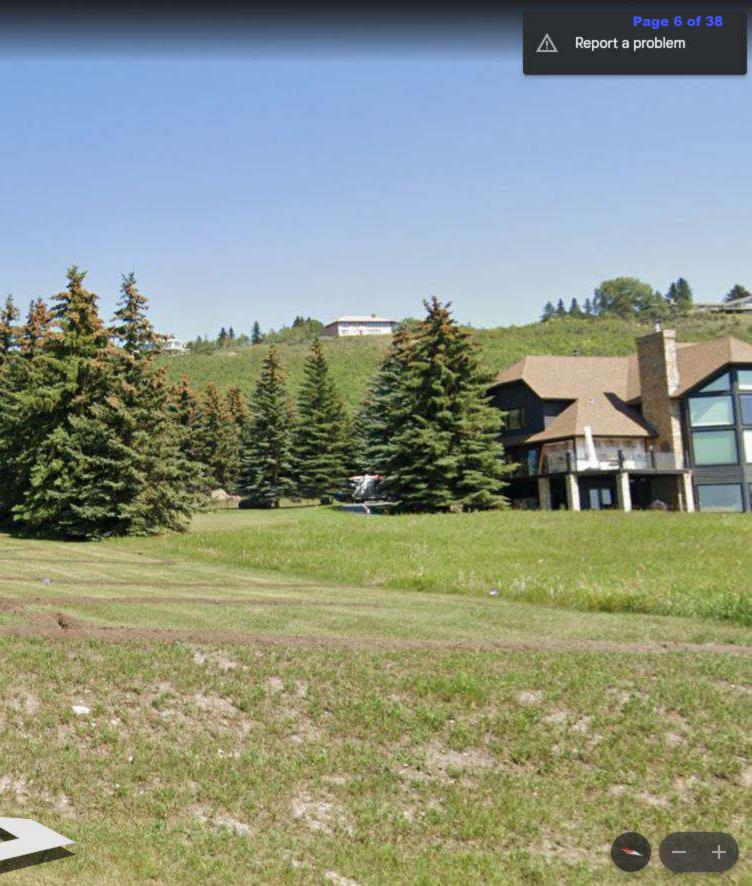


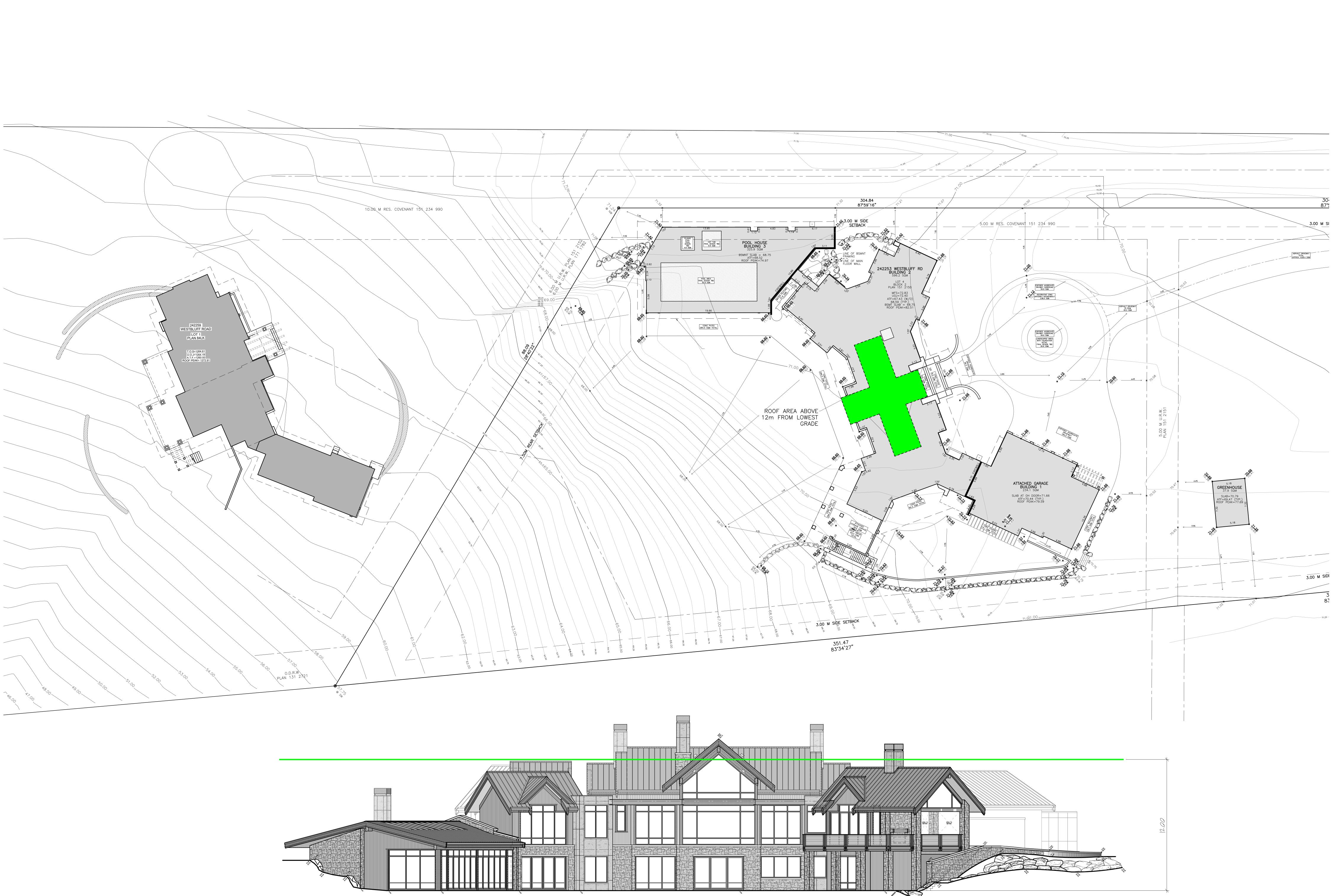


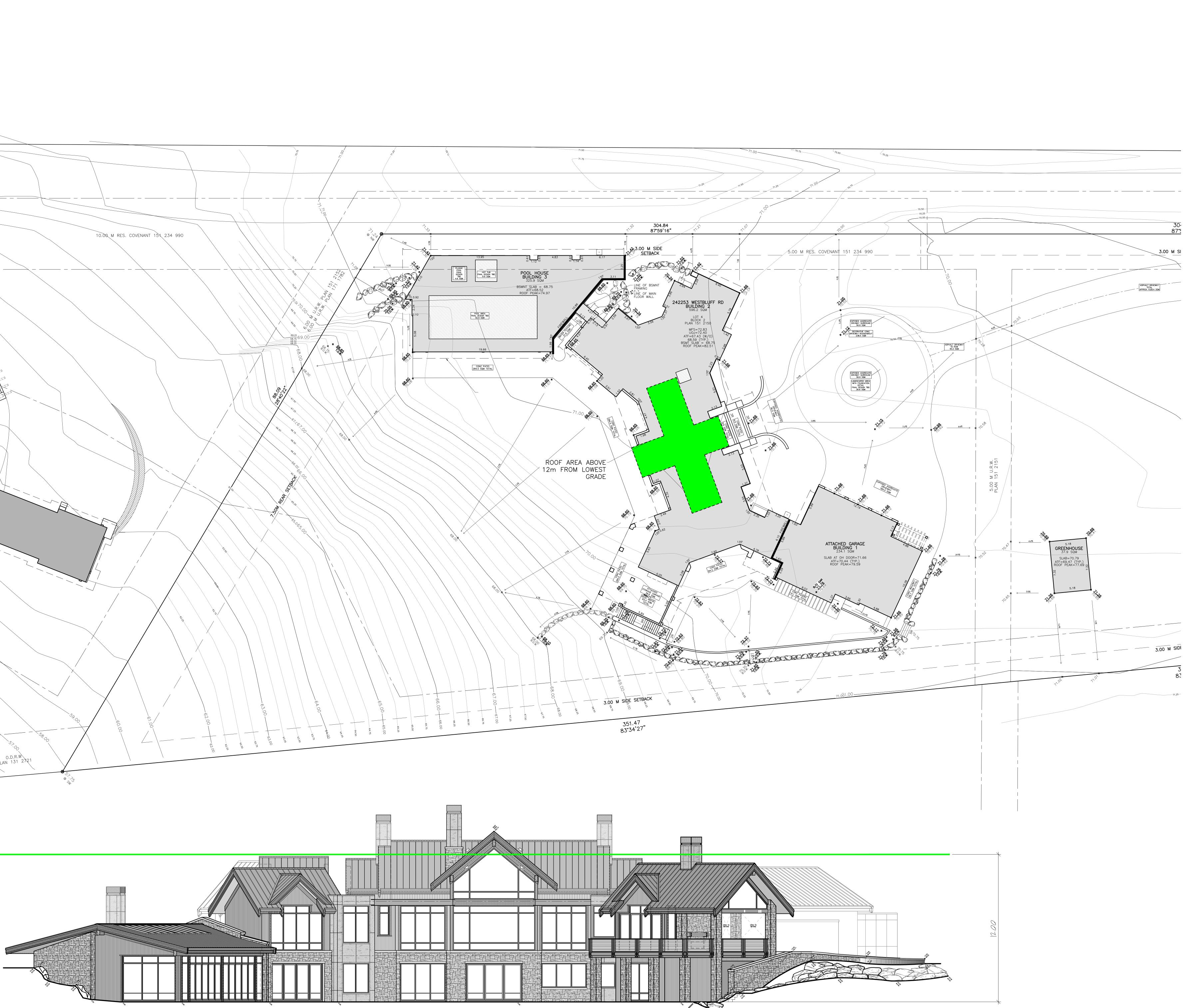
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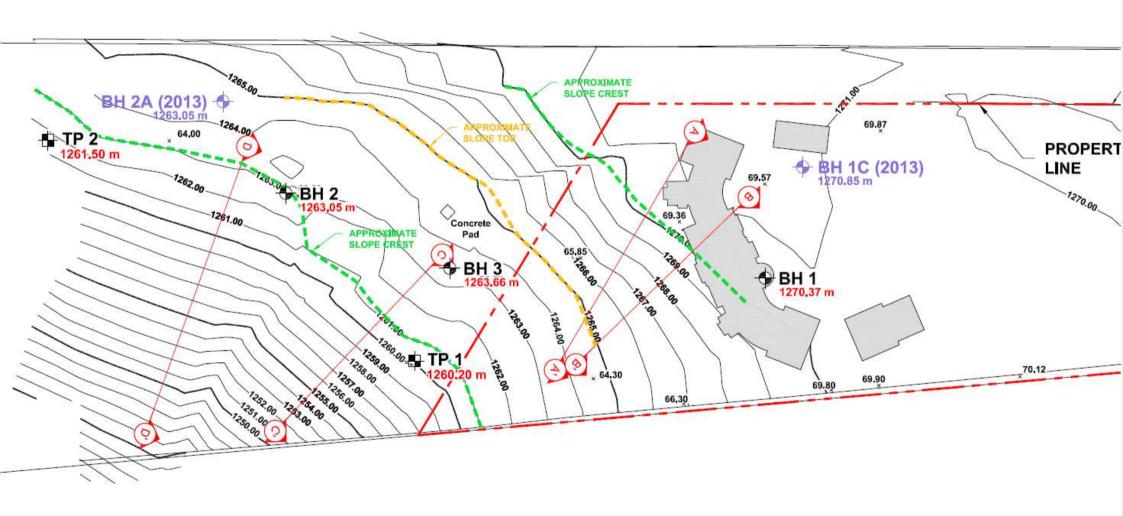
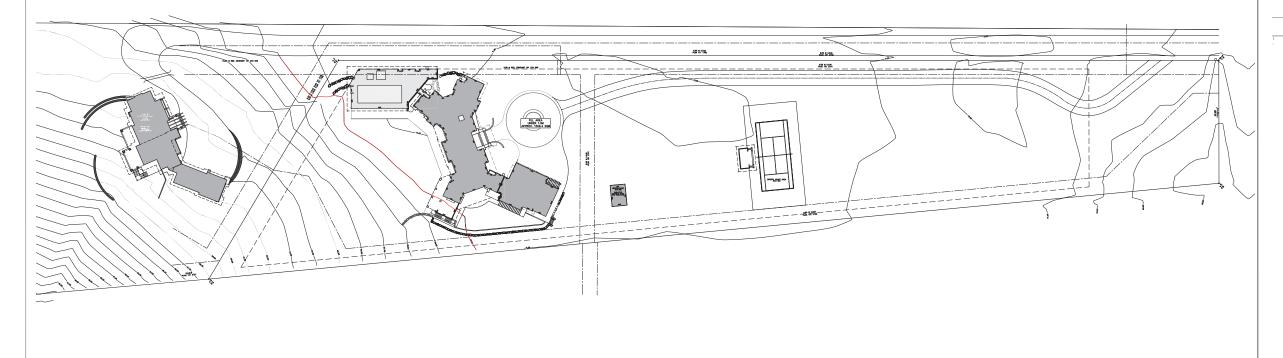


Exhibit 6 - Applicant Exhibit



Page 9 of 38



403 | 719 | 6641 1109 OLYMPIC WAY SE CALGARY, ALBERTA T2G 1B9



403 | 239 | 0602 544 47TH AVENUE NW CALGARY, ALBERTA T3B 1Z9

PROJECT:

ISSUED FOR APPEAL

242253 WESTBLUFF RD ROCKYVIEW COUNTY, AB

LOT 4 BLOCK 2 PLAN 1512150

сонструкт, высключения точного структование ракульное ракульное практа ракульное практа ракульное раку

REVISION SCHEDULE:

1. ISSUED FOR APPEAL MAR 26, 2024

DRAWING	TITLE

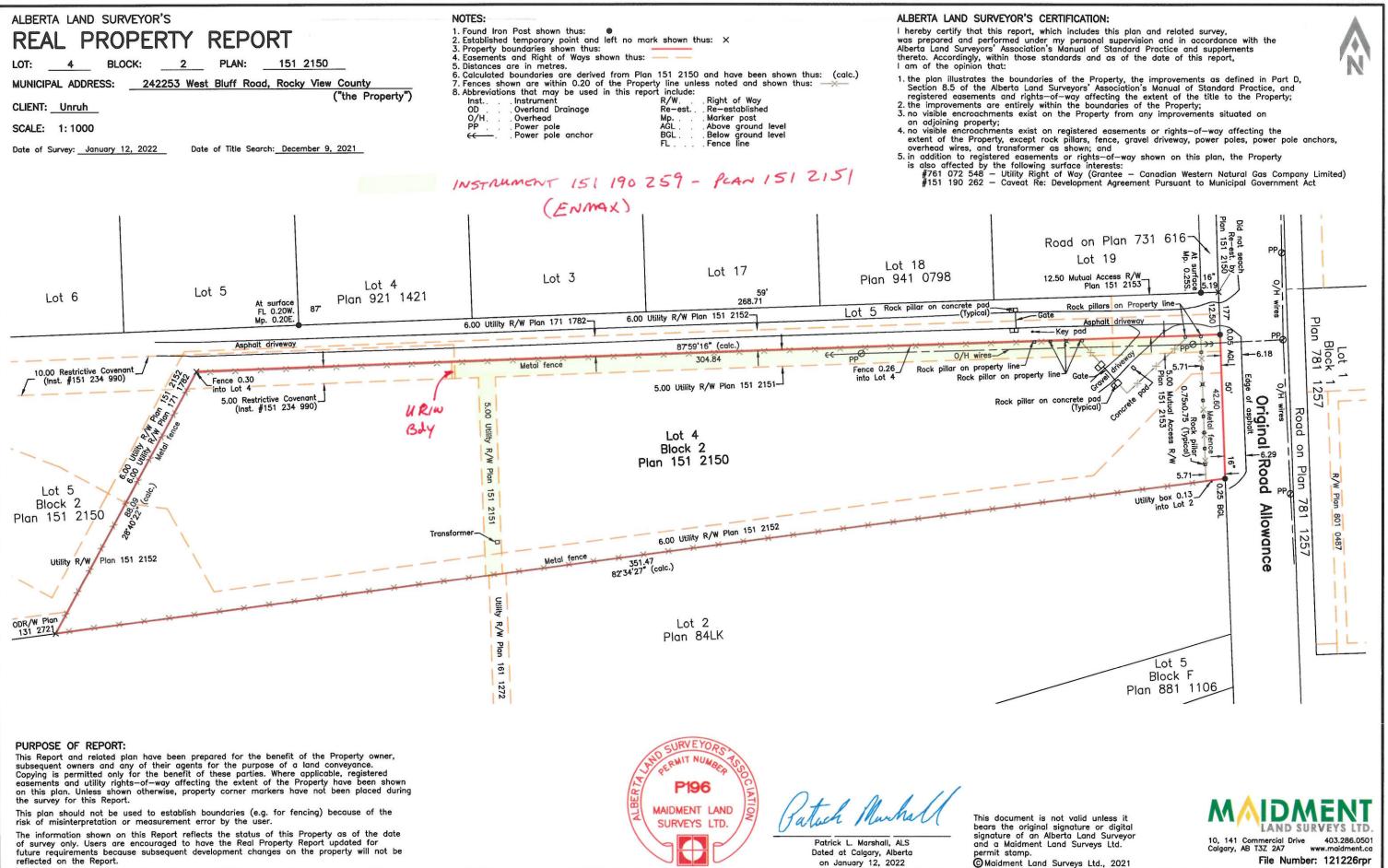
KEY PLAN

1:400 MAR 26, 2024

A1.1

SHEET:

SCALE: DATE:



on January 12, 2022

Exhibit 6 - Applicant Exhibit

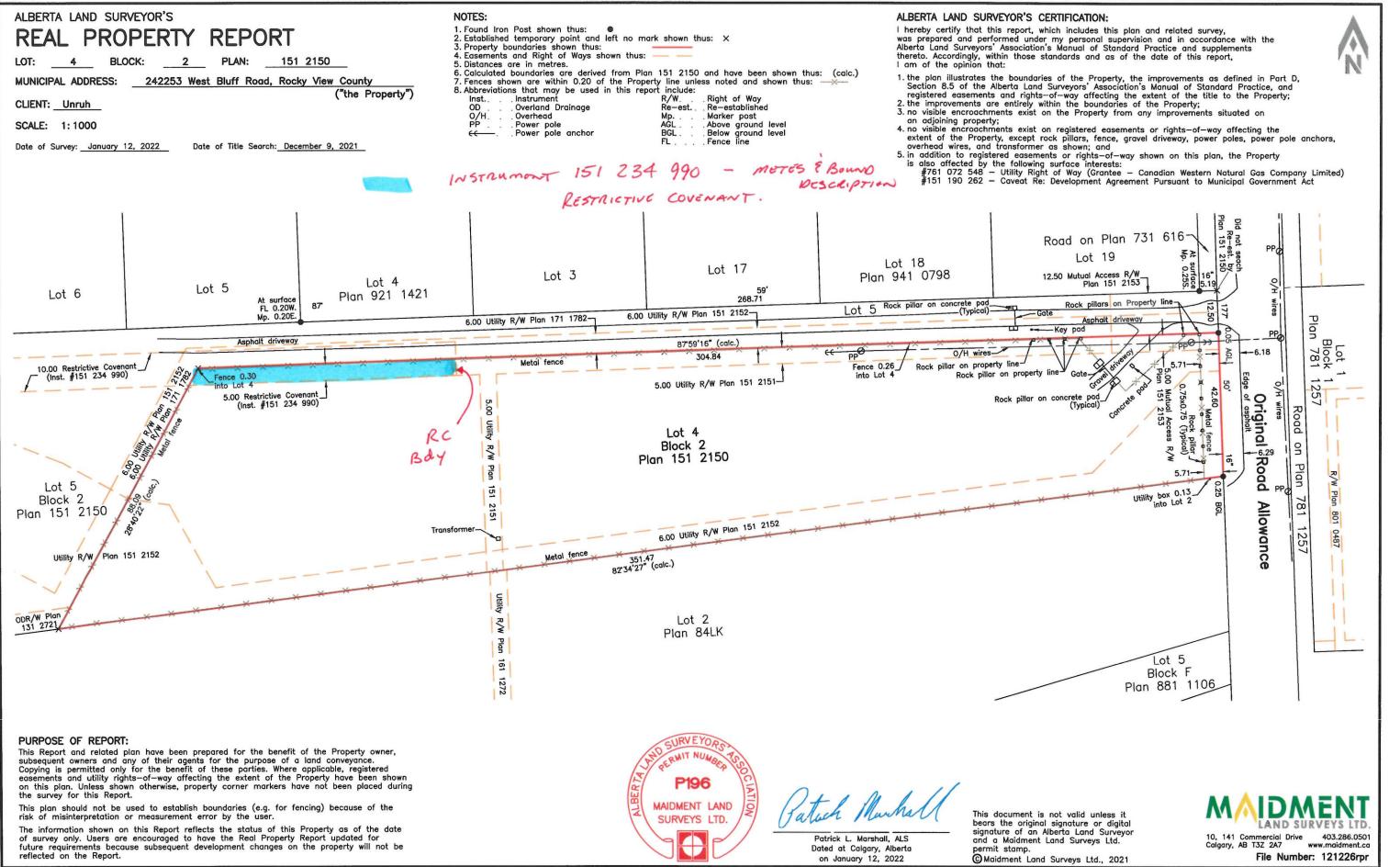
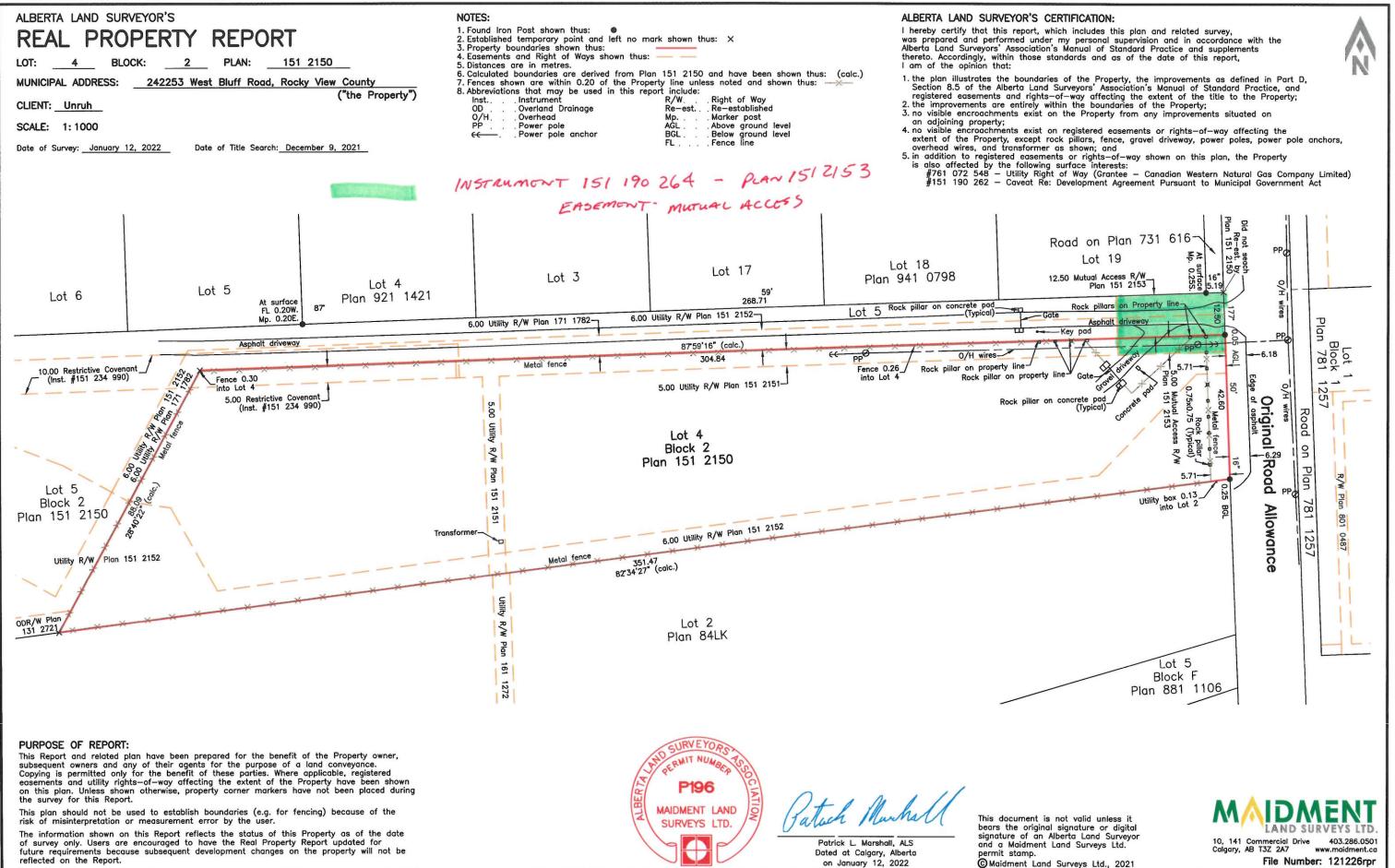


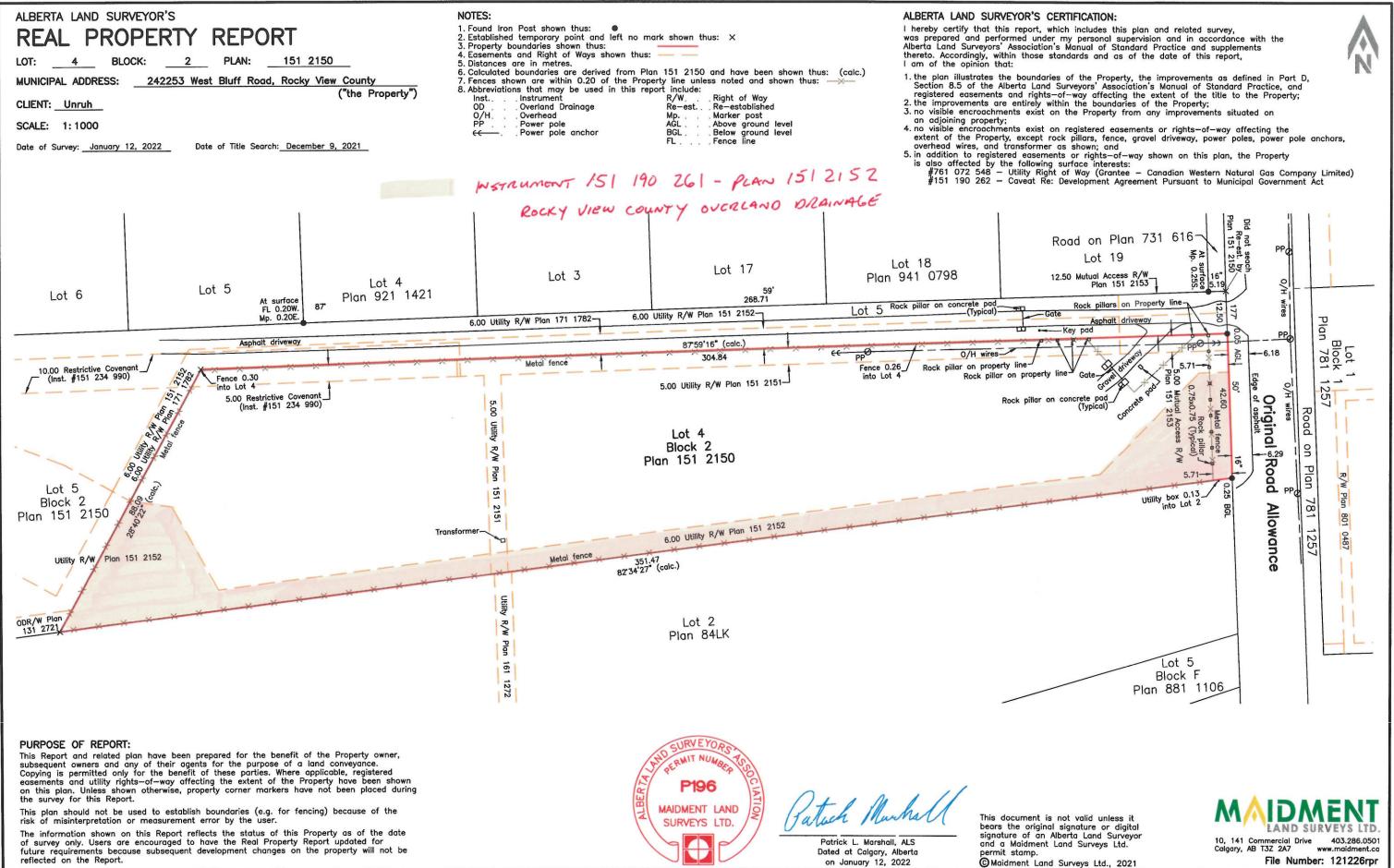


Exhibit 6 - Applicant Exhibit



on January 12, 2022

Exhibit 6 - Applicant Exhibit





OSPREY ENGINEERING INC. BOX 1367 · BLACK DIAMOND, ALBERTA · TOL OHO CANADA TEL: 403.933.2226 · EMAIL: ospreyeng@gmail.com

26 March 2024

Our file: 240927 Municipal file: PRDP20240118

Rocky View County 262075 Rocky View Point Rocky View County, AB T4A 0X2

Attention: Jeevan Wareh

RE: Lang-Hodge Residence 242253 Westbluff Road (Lot 4, Blk. 2, Plan 1512150, NE18-24-2-5) Site Specific Stormwater Implementation Plan (SSIP)

Dear Jeevan,

This letter is provided to address the following condition on the Notice of Decision dated 2023-08-22:

Prior to release, the Applicant/Owner shall submit a limited scope Site-Specific Stormwater Implementation Plan (SSIP) prepared by a qualified professional engineer, in accordance with Springbank Drainage Strategies [(Westhoff, 2004)] and County Servicing Standards [(Rocky View County, 2013)]. The SSIP must include a grading plan that illustrates the original ground profile; the depth of proposed fill; the total amount of soil to be imported/exported from the site; and analysis of the pre and post construction grades to determine whether there are any impacts to adjacent properties or the public road network. The engineer shall confirm pre and post construction conditions associated with site stormwater storage, unit area site releases, volume control target, and offsite drainage in accordance with recommendations of Springbank Drainage Strategies. The analysis shall also include recommendations for Erosion and Sediment control mitigation measures, as per County Servicing Standards.

I. BACKGROUND

242253 Westbluff Road is a country residential lot of 1.90 ha [4.70 acres] more or less located near the south end of Westbluff Road (see Figure 1 for general location). The parcel is presently vacant. The owner intends to construct a dwelling as shown on the architectural plans submitted to the county (excerpts included in this letter).

The parcel drains generally from east to west. The west portion of the parcel slopes steeply toward the southwest. Runoff from this slope flows overland to Clear Mountain Rise and Lower Springbank Road. Runoff in this area is tributary to the Elbow River near Highway 8. The general area is shown on Figure 2.

The Springbank Master Drainage Plan (Seeliger, 2016) (the MDP) is understood to be applicable. This plan is understood to generally describe how storm drainage *should* be managed in Springbank. However, an older document, *Drainage Strategies for Springbank* (Westhoff, 2004) served as the *de facto* master drainage plan at the time of subdivision.

A stormwater management plan was provided in 2013 in support of a subdivision which created the predecessor to this parcel and the lot immediately north (Bhaiji, 2013). A revision to this plan was provided in 2015 (Bhaiji, 2015). The current parcel boundaries were established in 2015.

The reports assumed the following regarding runoff from the subdivision:

LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORTWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY

- Unit area release rate: 1.714 L/s/ha
- No annual volume target

The stormwater management plan assumed the imperviousness of the lot would be approximately 440 $\rm m^2$ including

To meet this release rate, it is understood that 3 ponds were specified as follows:

- One pond (341 m³) in the southwest of Lot 5 at the edge of the steep slope,
- One pond (476 m³) in the west of the subject parcel (immediately south of the pond in Lot 5) and
- One pond (365 m³) in the southeast of the subject parcel adjacent to Westbluff Road.

The ponds were constructed in support of the subdivision and rights-of-way exist to contain them.

II. DISCUSSION

The site plan for the proposed dwelling (Dean Thomas Design Group, revision dated 2024-02-02) was provided by McKinley Masters. Other details are from publicly available data (AltaLIS Lidar 15 DEM, Google air photos) (see **Error! Reference source not found.**).

The following are notable:

- The grading plan provided by Dean Thomas appears reasonable.
- There is a clear path for runoff to the pond.
- Existing lot grading to the west directs runoff away from structures.
- Impervious surfaces
- Impervious surfaces (including roofs, asphalt and concrete driveways) total approximately 4078
 m² which is 21% of the total lot area:
 - The revised stormwater management plan (2015) for the parcel assumed a total impervious area of 440 m², including driveways.
 - Imperviousness in the east portion appears to be similar to what was previously predicted. No further concern is noted for the east portion of the parcel.
 - Imperviousness in the west 1.57 ha will be approximately 23.9%. This is approximately 10× larger than previous estimates.

Given the oversized dwelling proposed, stormwater management for the parcel must be revised. As no as-built survey was provided for the ponds, it is assumed they were constructed according to the stormwater management plan:

- Pond area at spill: 344 m²
 Pond depth at spill: 2.5 m above outlet pipe invert (3.2 m above flow control)
 Pond volume at spill: 476 m³
- Rate of discharge at spill: 2.4 L/s [0.0024 m³/s] (approx. 1.5 L/s/ha)

An EPA-SWMM model was constructed to determine the impact of the dwelling as proposed. This resulted in the pond flooding. As such, a larger pond is required to maintain the runoff from the parcel to the rate noted in 2015 and 2013 stormwater management plans. Based on analysis, the pond needs to



LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORTWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY

contain approximately 1100 m³ (630 m³ larger) below spill to maintain the rate of runoff prescribed in the previous stormwater management plan.

III. RECOMMENDATIONS AND CONCLUSION

Given the above, I assert the following:

- That the grading proposed is reasonable for the dwelling proposed.
- That adequate management of runoff can be accomplished provided the recommendations of this plan are followed.

The following recommendations are made specific to the development of proposed the proposed lot:

- That the pond should be expanded to ensure a volume of at least 1100 m³ is available below the spill elevation.
- All private sewage components shall be located above and at least 15 m from the spill contour of the pond.
- That all buildings shall be located outside areas of concentrated flow.
- That in conducting any construction the following general guidelines should be observed:
 - Land will be graded to ensure positive drainage.
 - Slopes will be kept as gentle as possible and within the range of 1% to 10% for side slopes and 1% to 3% for longitudinal slopes in ditches/swales.
 - Where necessary, limited areas [3 m or less] of steeper side slopes, up to 33% [3H:1V] can be accommodated provided they are adequately protected from erosion.
 - Slopes greater than those noted above will require specific measures (see below) to ensure erosion is controlled.
 - Where areas are disturbed, topsoil will be placed to a depth of not less than 200 mm [8"] and preferably 300 mm [12"] or more.
 - Placement of native topsoil from within the parcel is acceptable.
 - If imported topsoil is used it will have a clay content less than 40%, be more than 3% organic matter, have a sodium adsorption ratio (SAR) in the "good" range for plant growth, and have a neutral pH.
 - All disturbed areas will be seeded or sodded to ensure a good cover of vegetation as soon as practical, and
 - Specific species of vegetation will be at the proponent's discretion but regardless will be appropriate for the area and location planted.
- As the proponent is aware of the legal risks and penalties associated with unauthorized discharge of sediment into a water body, during any construction, the proponent will employ "good housekeeping practices" for erosion and sedimentation control on this site. This includes:
 - Locating any material stockpiles away from drainage courses, water bodies or areas of concentrated runoff flows.
 - Protecting stockpiles from the effects of wind.
 - Ensuring material will be temporary and will be removed or stabilized as noted below.
 - Ensuring that all bare earth is suitably stabilized with topsoil and an appropriate mulch and seed mix to allow establishment of vegetative cover as soon as possible. Alternately, bare slopes can be covered with a suitable, commercially-available erosion control matting (e.g. coco, hemp, geotextile).
- Any products proposed for use in erosion and sedimentation control shall be appropriate for their application. If any questions exist in this regard, the owner will contact a professional engineer or other professional skilled in erosion control (e.g. P.Ag. or CPESC) to provide recommendations, and



Exhibit 6 - Applicant Exhibit

LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORTWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY Page 4 26 March 2024

A further reference for erosion and sedimentation control best practices is *Erosion and Sediment Control Field Manual* (Calgary (City of), 2017), which is available at no cost from www.calgary.ca.

If you have any questions regarding this report, please contact the undersigned.

Yours truly,



^{ID 67456} 2024-03-26 Michael A. Kitchen, P.Eng. President

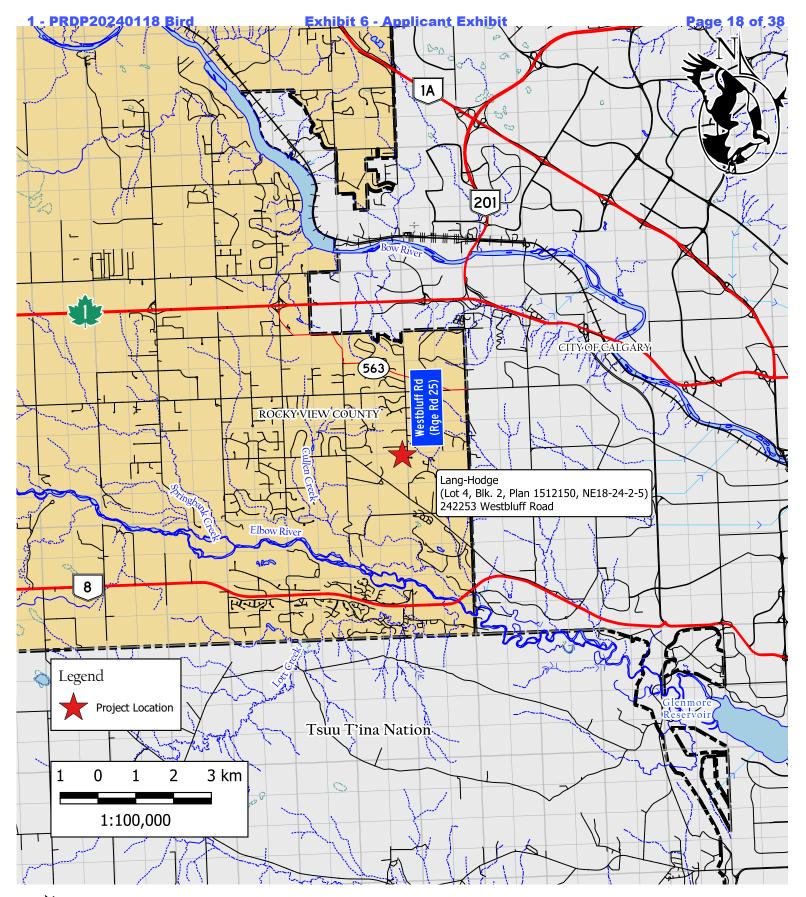
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cc: Cody Dunn – McKinley Masters File

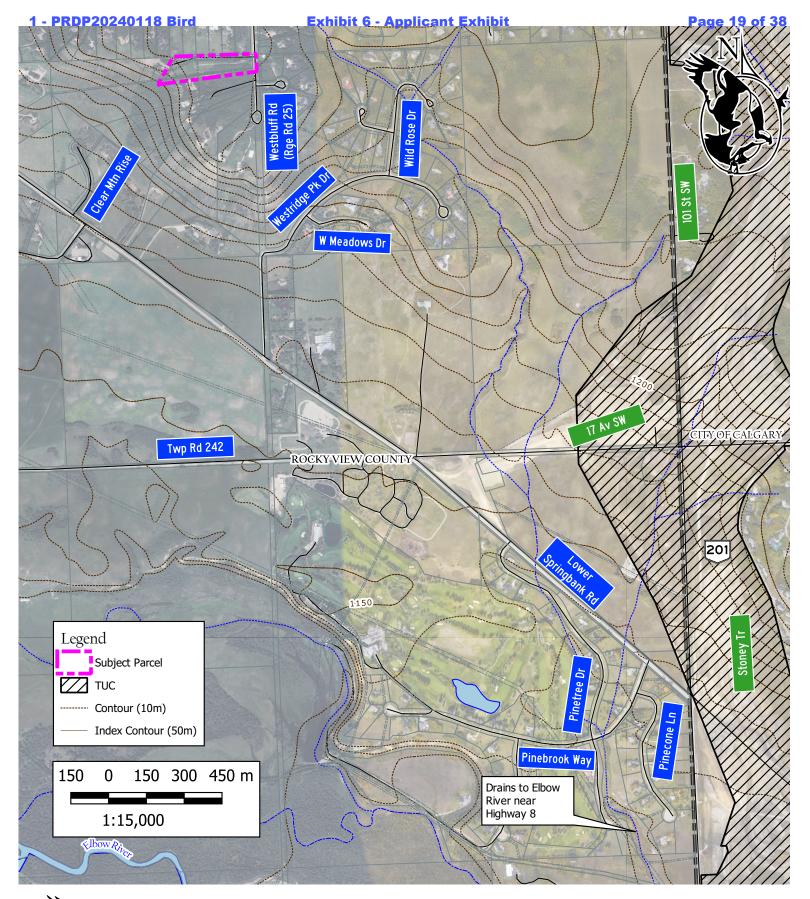
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RM APEGA ID #:67456					
DATE: 2024-03-26					
PERMIT NUMBER: P010743 The Association of Professional Engineers and Geoscientists of Alberta (APEGA)					





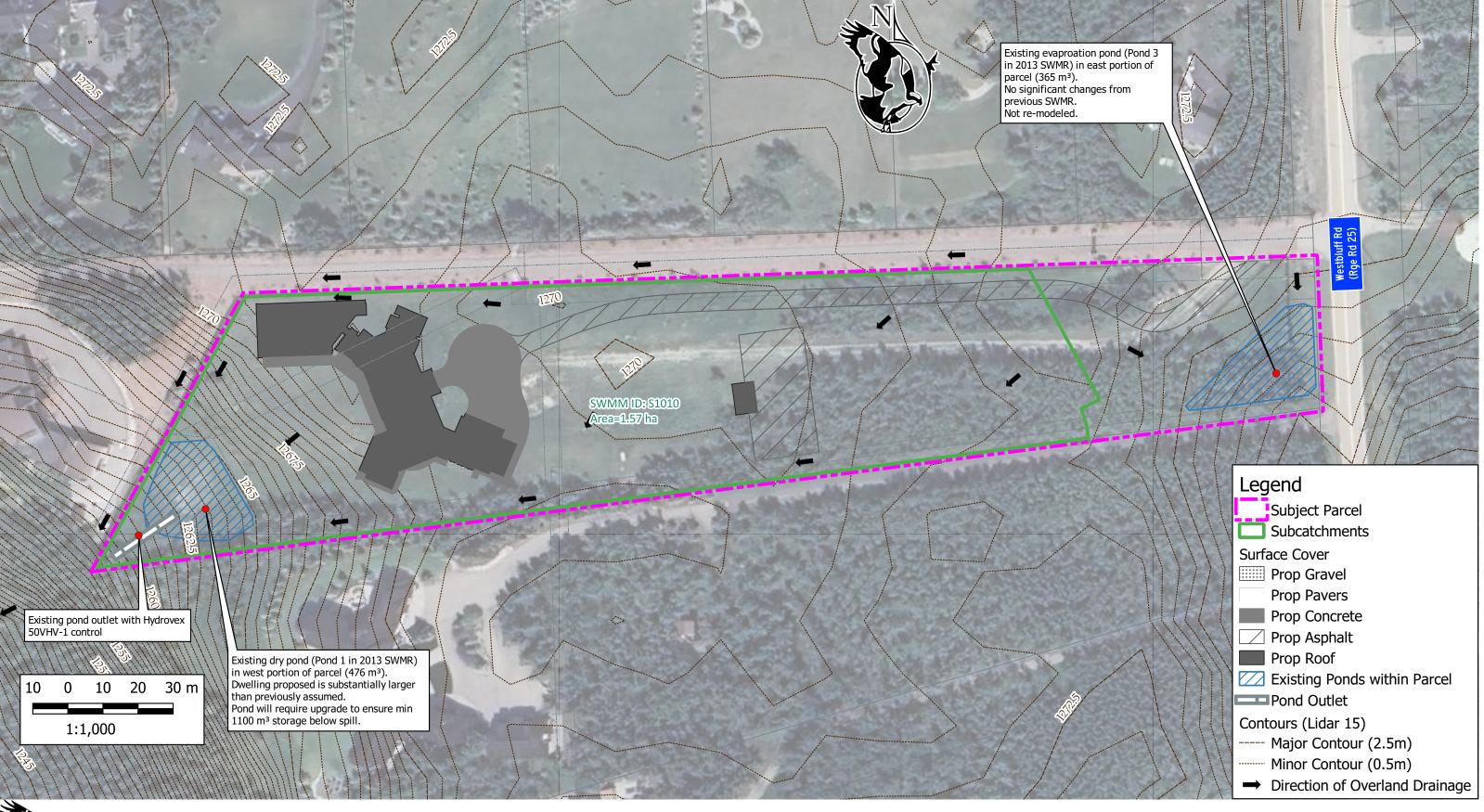


Lang- Hodge Residence Site Specific Stormwater Implementation Plan (SSIP) Figure 1 – Location





Lang- Hodge Residence Site Specific Stormwater Implementation Plan (SSIP) Figure 2 – Area Context





Lang- Hodge Residence Site Specific Stormwater Implementation Plan (SSIP)

Figure 3 - Surface Features

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LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORTWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY PAGE 8 26 March 2024

APPENDIX A – EPA SWMM MODEL

The following contains the results from the EPA-SWMM model constructed for the Lang-Hodge residence. Note that, similar to the previous stormwater management plans, the release rate from the site is so low that longer-term precipitation governs pond volumes. As such, only the continuous precipitation data set (1960-2010) was reviewed. The City of Calgary 1:100-year, 24-hour design storm (Calgary (City of), 2011) will not govern design and was not reviewed.

A. Hydrology

Storm drainage area (subcatchment) boundaries are shown on Error! Reference source not found.. Table 1 details the

specific hydrologic assumptions made for each subcatchment in EPA-SWMM. Assumptions common to all subcatchments are detailed in Table 2.

Table 1 – Subcatchment Parameters

Subcatchment ID	Runoff Drains to (ID)	Area (ha)	Width (m)	Flowpath Length (m)	Slope (%)	Imperviousness (%)
S1010	SU101	1.57	263	60	5.6	23.9

Table 2 – General Hydrologic Assumptions

Parameter	Value	Source
Surface roughness (Manning's n)	Impervious = 0.015 Pervious = 0.25	Pervious assumes lawn or pasture (American Society of Civil Engineers, 1992)
Depression storage	Imperv.: 1.6 mm Pervious: 3.2 mm (backslope, 150 mm topsoil) 7.5 mm (absorbent landscaping, 300 mm topsoil)	Impervious is as per developed areas, on-site pervious assumes absorbent landscaping: 0.3 m loamy topsoil, minimum.
Sub-area routing	Pervious	Routes both impervious surfaces as no storm sewers exist
Soil characteristics (Green-Ampt)	Clay loam K = 1.0 mm/hr ψ = 210 mm IMD = 0.27	(Rossman & Huber, 2016)

1. <u>Imperviousness</u>

Assumed imperviousness for different cover types are as prescribed by the City of Calgary (Calgary (City of), 2011). Overall imperviousness for each subcatchment was



PAGE 9

26 MARCH 2024

LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORMWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY

> derived using an area-weighted average based on the proposed sited development plan provided by the owner.

2. <u>Evaporation</u>

Evaporation in EPA-SWMM is calculated internally (Rossman & Huber, 2016) based on approved climate data (daily maximum and minimum temperatures) for Calgary. No evaporation is assumed in single-event modelling.

3. <u>Seasonal Variation of Parameters</u>

Seasonal variation of parameters (hydraulic conductivity) was assumed per the following:

- May to October: 1×value noted in Table 2.
- November to April: 0.05×value noted in Table 2.

This is not applicable to single-event models.

4. <u>Snowmelt</u>

Snowmelt is considered as noted in Table 3. This is not applicable to single-event models.

Table 3 - Snowmelt Parameters

Parameter	Value
Dividing temperature between rain and snow	2°C
Antecedent temperature index	0.5
Negative melt ratio	0.6
Elevation above MSL	1080 m
Latitude	51°N
Longitude correction	36 min (Mtn. Std. Time [105°W] to 114°W)
Minimum melt coefficient	0.05 mm/hr/°C
Maximum melt coefficient	0.3 mm/hr/°C
Base temperature for melt	0°C
Free water fraction to produce liquid	0.1

The monthly average windspeeds shown in Table 4 were used in the snowmelt model. Table 4 – Average Windspeeds (km/h) for Calgary Airport

January	February	March	April	May	June
14.8	14.6	15	16.5	16.6	15.6
July	August	September	October	November	December
14	13.2	14.1	14.6	13.7	14.9
В.	Pond Design				

The existing pond was assumed from the 2013 stormwater modeling (Bhaiji, 2013), and its stagestorage relationship is shown in Table 5.



LANG-HODGE RESIDENCE
242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5)
SITE SPECIFIC STORMWATER IMPLEMENTATION PLAN (SSIP)
ROCKY VIEW COUNTY

Table 5 – Stage Storage	Assumed for Existing Pond	(SWMM Node SU101)

Elevation (m AGD)	Depth (m)	Surface Area (m²)	Volume Detained (m ³)	
1261.10	0.00	1		Invert of control
1261.79	0.69	1	1	
1261.80	0.70	34	1	Invert of outlet
1264.30	3.20	344	473	Spill

1. <u>Offsite Flow Control</u>

Consistent with the previous stormwater management plans and the *de facto* MDP at subdivision, offsite runoff has an allowable unit release rate (AURR) of 1.714 L/s/ha. Offsite flow control for the minor system assumes inlet control devices (ICDs) on the pond control manhole. A Hydrovex 50VHV-1 control was specified. Outflow was assumed per Table 6 and was derived the manufacturer's published performance curves.

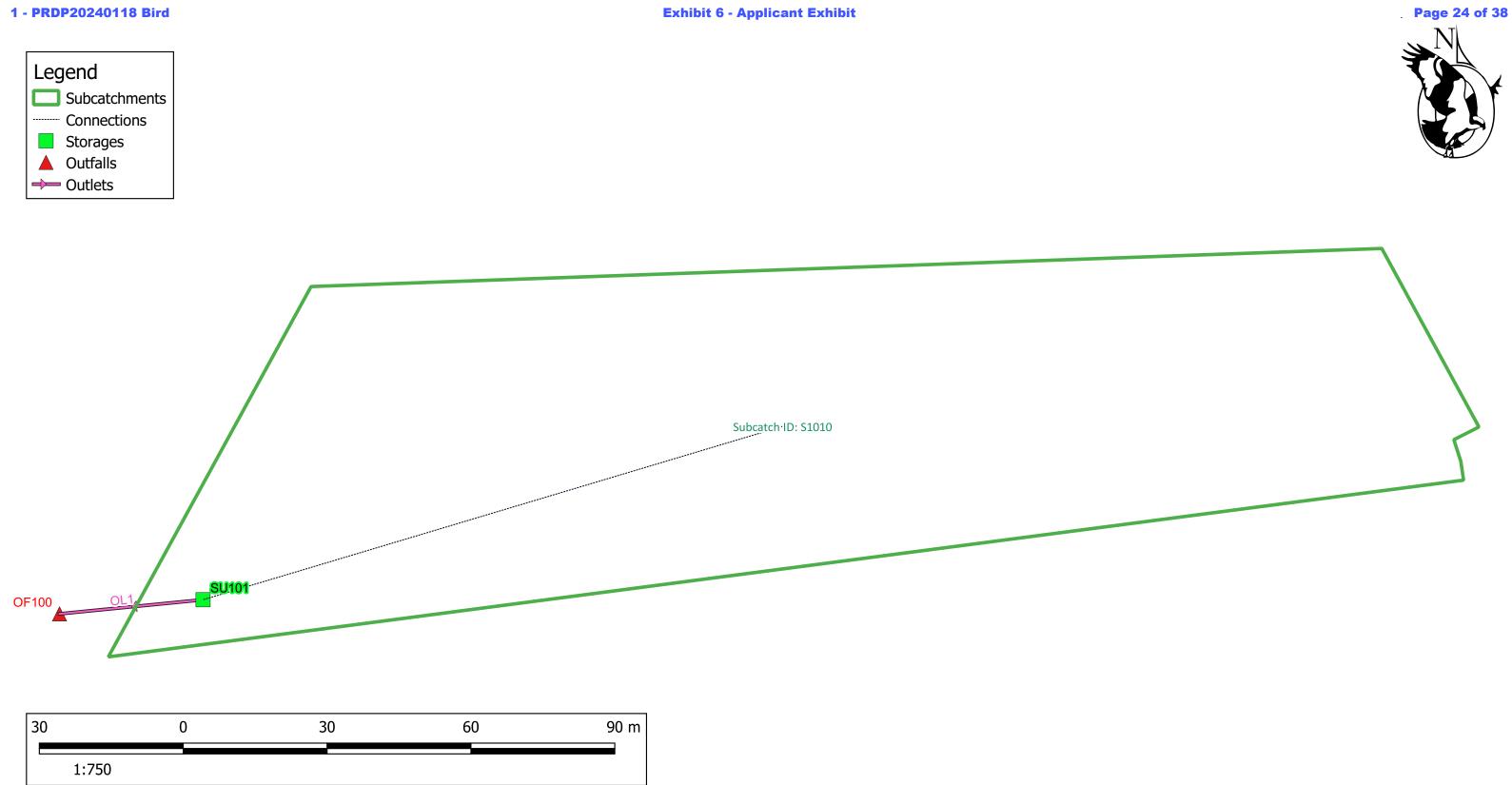
Depth/Head (m)	Flow (m³/s)
0	0
0.65	0.001
1.3	0.0015
2.5	0.002
5.7	0.003

Table 6 – Flow Control

2. <u>Determining Required Volume</u>

The required volume for the pond was determined by an extreme value analysis of annual pond volumes. This analysis was consistent with that prescribed by the City of Calgary (McMechan, et al., 2014). This required a second scenario with a pond of a large but arbitrary volume to ensure no flooding (overflow). This volume will guide detail design of the pond expansion. Results of the frequency analysis are appended.







Lang- Hodge Residence Site Specific Stormwater Implementation Plan (SSIP)

Figure 4 - EPA-SWMM Model Schematic

Exhibit 6 - Applicant Exhibit

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Lang-Hodge - Post-d								
	ev with ex	pond - Conti	nuous					

Element Count								
Number of rain gage Number of subcatchm	s 1							
Number of nodes	2							
Number of links								
Number of pollutant Number of land uses	Ö							

Name	Data Sou	rce		Data Type	Recordi Interva			
 Raingage							_Precip_Data HLY03.tx	+
Kanigage	1. \Data\.	Swim (CTTILace I	Data (cary	агу Аррг	oveu Data\F	IIIa1_HOUI_	_Precip_bata Heros.tx	L
*****	*							
Subcatchment Summar	ý							
Name	Ar	ea Width		%s1o			Outlet	
s1010		57 263.05		5.61			su101	

Node Summary								
Nama	Turne	I	nvert	Max.	Ponded Area	External		
Name	Туре							
OF100 SU101	OUTFALL STORAGE	120 120	60.77 61.10	0.00	0.0			
50101	STORAGE	12.	01.10	5.20	0.0			

Link Summary								
Name Fr	om Node	To Node		Туре	Len		lope Roughness	
	101	OF100		OUTLET				
cross Section Summa								
*****	**					-		
Conduit Sh	ape	Full Depth	Full Area	Hyd Rad	. Max. . Width	No. of Barrels	FUII Flow	
Flow Units	CM:	S						
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***** Highest Flow Instability Indexes All links are stable.

***** Routing Time Step Summary Winimum Time Step:Average Time Step:Maximum Time Step:% of Time in Steady State:Average Iterations per Step:% of Steps Not Converging: 30.00 sec 30.00 sec 30.00 sec 0.00 1.00 0.00

***** Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imper∨ Runoff mm	Perv Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
s1010	20455.70	0.00	2960.79	15344.48	3177.63	2171.10	2171.10	34.19	0.22	0.106

**** Node Depth Summary

Node	Туре	Average Depth Meters	Maximum Depth Meters	HGL	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF100	OUTFALL	0.00		1260.77	0 00:00	0.00
SU101	STORAGE	0.02		1264.30	3816 13:27	3.20

Node Inflow Summary

Node	туре	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
OF100	OUTFALL	0.000	0.002	3816 13:27	0	32.8	0.000
SU101	STORAGE	0.223	0.223	17322 19:00	34.2	34.2	0.006

Node Flooding Summary

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m³
su101	18.90	0.221	17322 19:00	1.323	0.000

Storage Volume Summary

Storage Unit	Average	Avg	Evap	Exfil	Maximum	Max	Time of Max	Maximum
	Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	Occurrence	Outflow
	1000 m³	Full	Loss	Loss	1000 m³	Full	days hr:min	CMS
SU101	0.001	0.2	0.3	0.0	0.473	100.0	3816 13:27	0.002

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	AVg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
OF100	1.49	0.001	0.002	32.768
System	1.49	0.001	0.002	32.768

Link Flow Summary

Link	Туре	Flow	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
0L1	DUMMY	0.002	3816 13:27			

***** Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Mon Mar 25 17:27:26 2024 Analysis ended on: Mon Mar 25 17:27:42 2024 Total elapsed time: 00:00:16

Exhibit 6 - Applicant Exhibit

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

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***** Highest Flow Instability Indexes All links are stable.

***** Routing Time Step Summary Winimum Time Step:Average Time Step:Maximum Time Step:% of Time in Steady State:Average Iterations per Step:% of Steps Not Converging: 30.00 sec 30.00 sec 30.00 sec 0.00 1.00 0.00

***** Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imper∨ Runoff mm	Per∨ Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
s1010	20455.70	0.00	2960.79	15344.48	3177.63	2171.10	2171.10	34.19	0.22	0.106

Node Depth Summary

-	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_	_

Node	Туре	Average Depth Meters	Depth	HGL	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF100 SU101	OUTFALL STORAGE	0.00 0.01		1260.77 1263.48	0 00:00 17323 11:14	0.00 2.38

Node Inflow Summary

Node	туре	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
OF100	OUTFALL	0.000	0.002	17323 11:14	0	31.6	0.000
SU101	STORAGE	0.223	0.223	17322 19:00	34.2	34.2	0.001

Node Flooding Summary

No nodes were flooded.

***** Storage Volume Summary

Storage Unit	Average	Avg	Evap	Exfil	Maximum	Max	Time of Max	Maximum
	Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	Occurrence	Outflow
	1000 m³	Full	Loss	Loss	1000 m³	Full	days hr:min	CMS
su101	0.005	0.4	7.4	0.0	0.952	74.4	17323 11:14	0.002

******************** Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
OF100	8.83	0.000	0.002	31.649
System	8.83	0.000	0.002	31.649

Link Flow Summary

Link	Туре		Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
0L1	DUMMY	0.002	17323 11:14			

***** Conduit Surcharge Summary

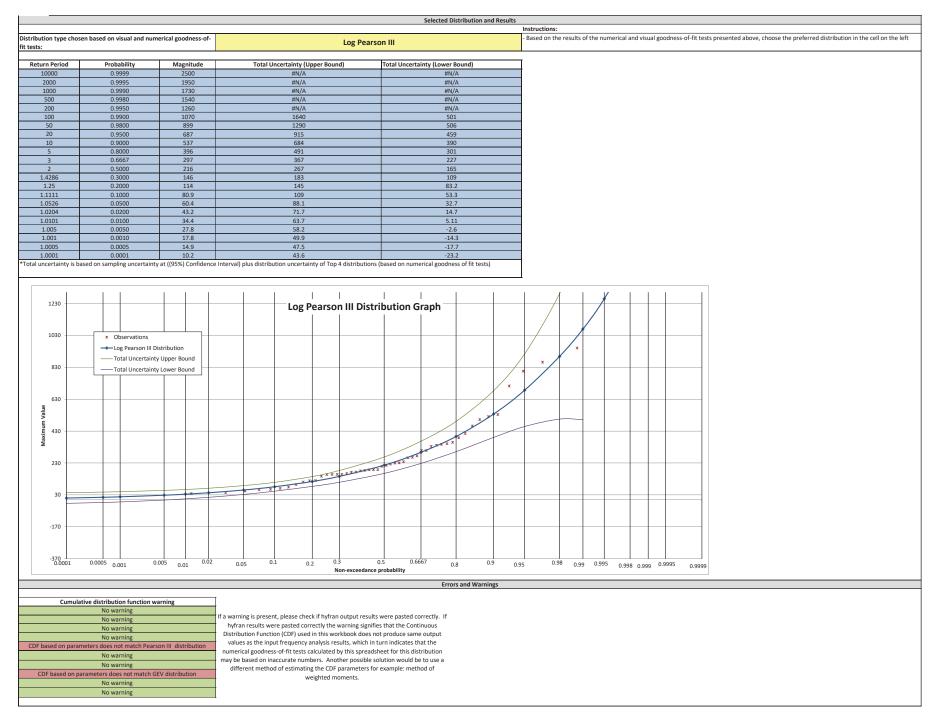
No conduits were surcharged.

Analysis begun on: Tue Mar 26 10:06:48 2024 Analysis ended on: Tue Mar 26 10:07:04 2024 Total elapsed time: 00:00:16

1 - PRDP202401188 Bird Data and Frequency Analysis Spreadsheet for the City of Calgary - Version 1.2 - February 2014

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Data and Frequency Analysis Spreadsheet for the City of Calgary - Version 1.2 - February 2014



LANG-HODGE RESIDENCE 242253 WESTBLUFF ROAD (LOT 4, BLK. 2, PLAN 1512150, NE18-24-2-5) SITE SPECIFIC STORMWATER IMPLEMENTATION PLAN (SSIP) ROCKY VIEW COUNTY PAGE 18 26 March 2024

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DEEP FILLS REPORT

To: Cody Dunn McKinley Masters

Re: 242253 Westbluff Road Rocky View County, AB Project Number: Project Type: Report Date: 304-001 Deep Fills Report March 24, 2024

Dear Sir,

It is understood that a single-family home is to be constructed at the municipal address of 242253 Westbluff Road in Rocky View County and that backfill thicknesses of up to approximately 2.9 m are planned. As per the Rocky View County County Servicing Standards (RVCCSS), when constructed depth of fill exceeds 1.2 m, a deep fills report is required to provide general recommendations for different types of building foundations and compaction testing of fill.

Upon provision of the Development Permit, the geotechnical engineer of record (Factor Geotechnical Ltd.) must provide compaction testing services to ensure that backfill placement is compliant with the RVCCSS, industry standards, and the recommendations within this report.

DESKTOP REVIEW AND FOUNDATION TYPE

A geotechnical investigation and slope assessment by ParklandGeo "Geotechnical Slope Assessment, 242253 Westbluff Road" dated August 10, 2016 was provided to Factor Geotechnical Ltd. (Factor) for review. This report includes one borehole located near the residence's proposed location that is expected to be representative of the local soil conditions.

As excavation has not yet begun, Factor has reviewed the above noted report, surficial geology maps, and nearby projects to determine the expected soil types at the project location. Based on said review, it is likely that glacial sediment overlying tertiary fluvial-channel sediment will be encountered. Sediments in this area are expected to consist of a layer of clay till overlying gravel.

FOUNDATION RECOMMENDATIONS

It is understood that the preferred foundation type for the subject structure are shallow footings. Based on the desktop review of the expected soil conditions, shallow footings are considered suitable for the proposed development. It is anticipated that fill up to approximately 2.4 m is required within the building footprint and that footings will be placed on native soils near the current grade elevation (or on structural fill).



SITE PREPARATION

The subgrade for all proposed buildings, roads, flatwork, and other structures must be stripped of all vegetation, organics, fill, topsoil, and construction debris prior to construction. Failure to provide a properly prepared subgrade may result in settlement, whether differential or excessive, that may negatively affect the building performance and serviceability of the proposed development. A properly prepared subgrade is defined below:

- Any construction debris is completely removed from the subgrade.
- Organic materials, vegetation, and any untested fill materials are not present in the subgrade.
- The subgrade has been inspected by Factor to verify that adequate subgrade support is available.
 - The entirety of the proposed footing areas should be cut neat to the bottom of footing elevation prior to inspection. Any areas not prepared to the satisfaction of Factor at the time of inspection will be subject to reinspection.
 - The subgrade in all proposed roadways or foundations shall be scarified to a minimum depth of 200 mm, moisture conditioned, and recompacted to 98% of the Standard Proctor Maximum Dry Density (SPMDD). If any additional fill is required, all placed fill (fill lifts not to exceed a thickness of 200 mm) shall be compacted to 98% of SPMDD.
 - The subgrade for paved areas is recommended to be proof-rolled under the supervision of a qualified geotechnical engineer prior to placement of the granular subbase materials. A proof roll is completed by slowly driving (4 to 6 km/hr) a fully loaded tandem axle dump truck/water truck with a rear axle load of no less than 8,200 kg over the prepared subgrade while the inspecting engineer observes deflections.
- The subgrade is not frozen at the time of foundation construction and will not freeze immediately before, during, or after foundation construction, for the lifetime of the structure. Methods to prevent freezing of the foundation subgrade include glycol lines with insulated tarps or heating and hoarding.
- Positive drainage is maintained away from the structure before, during, and after construction of the foundation.
- Areas of the subgrade that have been identified as soft, loose, excessively moist, or otherwise unsuitable for construction have been remediated under the direction of Factor (see Appendix A).
- Prior to fill placement, slopes in fill areas should be cut back to a maximum gradient of 5H:1V to minimize the
 potential for differential settlement.



SETTLEMENT COMMENTS

It is expected that the native soils in the project area will be normally consolidated and may be subject to further consolidation when loaded by the backfill and proposed structure. Settlements of this nature are typically minor and well tolerated. Should variations in soil type be noted in the native soils across the backfill footprint, additional analysis may be required to account for any differential settlements in the native soils.

Self weight settlements of backfill compacted to 98% of the material's SPMDD can be expected to be between 0.5% to 2.0% of the fill height, depending on the type of soil. Fine grained soils such as clays or silts will tend towards the higher end of the given range, whereas coarse grained soils such as gravels or sands will tend towards the lower. Further settlement due to loading of the compacted backfill will also occur, but is expected to be minor, should proper compaction procedures be followed.

Based on the anticipated 2.9 m of maximum fill thickness on the site, self-weight settlement of fine-grained soils compacted to 98% of SPMDD can be expected to be a maximum of 58 mm. Where coarse-grained soils or structural fill compacted to 98% SPMDD is used, settlement can be expected to be a maximum of 15 mm.

BACKFILL AND COMPACTION RECOMMENDATIONS

Backfill should be placed in lifts no larger than 300 mm before compaction, compacted to 98% of SPMDD with testing completed on each lift. Areas outside of roadways or foundations must be compacted to a minimum of 95% of SPMDD. Per RVCCSS, "Following development approval, all deep fill placement must have a record of compaction testing". Once compaction work has been completed, compaction reports and a summary of the work can be provided by Factor.

Should backfill and compaction activities take place during freezing temperatures, it is recommended that the backfill material used in deep fill areas (over 1.2 m thick) consists of coarse-grained soils such as 75 mm sandy gravel with less than 8% fines content. Coarse grained materials should still be protected from freezing but are less susceptible to frost heave than fine grained soils such as the clay materials expected to be present on site. If clay is used as backfill, careful monitoring of the soil temperature will be required in addition to compaction testing.

Backfill should be consistent throughout the fill areas and not contain cobbles over 150 mm in diameter. Backfill below any proposed structures must consist of structural fill, or a lag time will apply to construction of the structure's foundation elements. If structural fill is not proposed for fill below foundations, contact Factor for additional recommendations related to lag times.



CLOSURE

We trust that this document meets your present requirements. Should you have any questions or wish to discuss the contents of this letter, please contact the undersigned.

Yours truly,

PERMIT TO PRACTICE FACTOR GEOTECHNICAL LTD.	SHAL ENGINEER
RM SIGNATURE:	SSU DI 13/19/
DATE: Mar. 24 2024 PERMIT NUMBER: P015247	
The Association of Professional Engineers and Geoscientists of Alberta (APEGA)	Josh Clark, P.Eng. Geotechnical Engineer

Tyler Daigle, E.I.T. Geotechnical EIT



LIMITATIONS AND CONDITIONS

LIMITATIONS

This Report has been prepared in accordance with the applicable jurisdiction's generally accepted engineering practices. No other warranty, expressed or implied, is intended or made. The information provided within this report is for the sole benefit of the Client. No other party may use or rely on the Report without written consent from Factor.

The identification and classification of soil type and geological profiles are a professional opinion based on the information available at the time of the inspection or investigation. Soil is inherently variable, and the actual site conditions can vary significantly between the investigated locations. The parties relying upon this Report should be aware of this risk and the delivery of this Report is subject to the express condition that such risks are accepted by the parties relying upon this Report.

The information and recommendations within this Report are based on the information gathered from information provided to Factor. Factor is entitled to rely on the information and representations provided by the Client and is not required to verify the accuracy of such information or representations.

If a geotechnical letter of assurance, compliance, or sign-off is required for this project, the Client is required to notify Factor so that timely field reviews can be provided during construction. Field reviews will allow Factor to verify that recommended construction practices are followed, and site conditions are consistent with this report.



GENERAL DESIGN & CONSTRUCTION GUIDELINES

1. DEFINITIONS

"General engineered fill" is used in areas where moderate subgrade movement is tolerable by the gradesupported structures (asphalt or sidewalks). This material may consist of low to medium plastic inorganic clay or granular materials. Materials meeting the standards of "select engineered fill" or "structural engineered fill" would be considered acceptable for use as "general engineered fill"

"Select engineered fill" is used in areas where only minor subgrade movement is tolerable by the grade-supported structures (slab-on-grade or within the building footprint). This material may consist of clay or granular soils meeting the following specifications:

Clay:	Liquid Limit =	20 to 40%	
	Plastic Limit =	10 to 20%	
	Plasticity Index =	10 to 30%	
Gravel:	Free of clay, loam, or other deleterious materials Less than 10% of particles passing No. 200 sieve "Structural engineered fill" would be considered acceptable		

"**Structural engineered fill**" is used in areas where the subgrade is used to support structural loads, such as under footings. This material may consist of clean, well-graded crushed aggregate, free of organics, coal, clay lumps, or fine soil particles. This material should have less than 10% of particles passing the No. 200 sieve and meet all specifications for the project's jurisdiction.

"Landscape fill" is used in areas where settlement can be tolerated such as berms or grassed areas. This material may consist of any locally available soils.

Standard Proctor Density (SPD) refers to the Standard Proctor Maximum Dry Density as determined by ASTM D698. Optimum moisture content is also defined in ASTM D698.

2. BACKFILL AND COMPACTION

All backfill must be free of frost, construction debris, and lumps must be broken down before placement. Any oversized particles exceeding 50% of the lift thickness must be removed. Backfill must not be placed over a frozen subgrade.

Backfill material used adjacent to grade beams, pile caps, basement walls, abutments, above footings, and below pavement sections should consist of "general engineered fill" materials.

Backfill material used within 500 mm of the final grade near foundation walls, grade beams, pile caps, and footings should be relatively impervious to reduce seepage into the subsoil against the structure. This material can consist of cohesive "general engineered fill" materials.



Backfill placement against structures should be delayed until the structure can sufficiently withstand the earth pressures resulting from placement and compaction. If any deflection of the structure is noted during compaction, the compaction equipment, lift thickness, and other factors should be evaluated by a geotechnical engineer prior to further backfilling activities. Only handheld compaction equipment is recommended within 1 m of the backside of retaining walls or basement walls. Where fill placement is required on both the front and back of the structure, both sides should be backfilled and compacted in such a way that the difference in fill elevation is no greater than 500 mm.

Adequate bonding is required between backfill lifts. Any desiccated layers must be scarified, moisture conditioned, recompacted to the specified density, and bonded to the following lift. Granular materials should be scarified approximately 75 mm, moisture conditions, and recompacted to allow for bonding.

3. COMPACTION AND MOISTURE CONDITIONING

The following general compaction guidelines should be considered the minimum requirements. The stricter of these recommendations or the project specifications shall be used.

"General engineered fill" and "select engineered fill" shall be compacted to a minimum of 98% of SPD in maximum 200 mm thick lifts. Cohesive materials should be compacted at 0 to 2% above their optimum moisture content, while granular materials should be compacted 0 to 2% below their optimum moisture content.

"Structural engineered fill" shall be compacted to a minimum of 100% of SPD in maximum 150 mm lifts at 0 to 2% below their optimum moisture content.

"Landscape fill" shall be compacted to a minimum of 90% of SPD in maximum 300 mm lifts.

4. DRAINAGE AND BEDDING MATERIALS

Gravel utilized for drainage or weeping tile bedding should be clean, free-draining gravel or crushed rock generally containing no more than 5% soil particles passing the No. 200 sieve.

Coarse sand conforming to the following grading limits shall be considered suitable for drainage, use in pipe bedding, and use within the pipe embedment zone:

Sieve Size	Coarse Sand
10 mm	100
5 mm	95 – 100
2.5 mm	80 – 100
1.25 mm	50 – 90
630 µm	25 - 65
315 µm	10 – 35
160 µm	2 - 10

Please refer to project specifications or jurisdiction for exact specifications.