



Copithorne Gravel Pit

Transportation Impact Assessment Update

Final report

Prepared for
Roy & Judy Copithorne

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02-20-055

CORPORATE AUTHORIZATION

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1. EXECUTIVE SUMMARY

An update to the 2016 dated Copithorne Gravel Pit TIA has been requested by the business owners in light of new available traffic data. This updated TIA is provided for use by the owners of the gravel pit site located at SW-18-24-04-W5M. The site is located 5 kilometres west of Highway 22 on Township Road 242.

This updated Transportation Impact Assessment (TIA) is submitted in accordance with County Servicing Standards. Bunt & Associates Engineering (AB) Ltd. was retained to undertake this study. The key findings and corresponding recommendations are summarized below.

1.1 Proposed Development

The site will operate with the following conditions:

- Hauling will occur from 7:00-17:00 on Monday-Saturday. Generally hauling will occur from May 1 to October 30; however, dependant on weather, hauling may also fall outside this period.
- A total of 2 full time employees will work on-site, there are no extra employees when crushing is occurring.
- All traffic will access the site via the parcel to the east, which is also owned by Mr. Copithorne. Traffic will travel to Highway 22 via Township Road 242. This is the current route used by previous operations on the site.
- All outbound truck traffic will turn right at Highway 22 and proceed to the south on Highway 22. Trucks will enter the site from both the north and south on Highway 22.

Expected site trip generation is summarized in **Table 1.1**.

Table 1.1: Development Trip Generation

LAND USE	AM PEAK HOUR			PM PEAK HOUR			DAILY		
	Total	In	Out	Total	In	Out	Total	In	Out
Passenger Vehicle Trips	2	2	0	2	0	2	4	2	2
Truck Trips	9	5	4	9	4	5	90	45	45
Total Trips	11	7	4	11	4	7	94	47	47

1.2 Highway 22 & Township Road 242 Operations

Highway 22 & Township Road 242 has a type I intersection treatment. A review of existing, background, and post development conditions at the intersection indicated the following:

1.2.1 Collision History

A total of 5 collisions occurred at the intersection over a 14-year period. Three of the collisions were single-vehicle (struck animal; run-off-road). A collision occurred in 2005 that involved 3 vehicles with 4 injuries and 2 fatalities. A 2015 rear-end collision involving 2 trucks is also documented.

1.2.2 Sight Distance

All minimum and desirable sight distance requirements are met at the intersection.

1.2.3 Background Intersection Capacity Analysis

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the Opening Day (2020) and Long Term (2040) horizons.

1.2.4 Post Development Intersection Capacity Analysis

The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.

1.2.5 Alberta Transportation Intersection Warrant

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

1.2.6 Alberta Transportation Illumination Warrant

Street light illumination is not warranted at the intersection.

1.3 Roadway Link Capacity Analysis

The daily traffic volumes on Highway 22 and Township Road 242 were reviewed to determine the impact of background and post development traffic volumes.

1.3.1 Highway 22

The Background road link analysis suggests twinning of Highway 22 may be required by the Long Term Background horizon (2040). This matches with the recommendations from the *Highway 22 Functional Planning Study*. No appreciable change will occur due to development traffic volumes.

1.3.2 Township Road 242

The roadway is expected to continue operating within its daily vehicle capacity. Due to the expected truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.

2. INTRODUCTION

2.1 Scope of Work

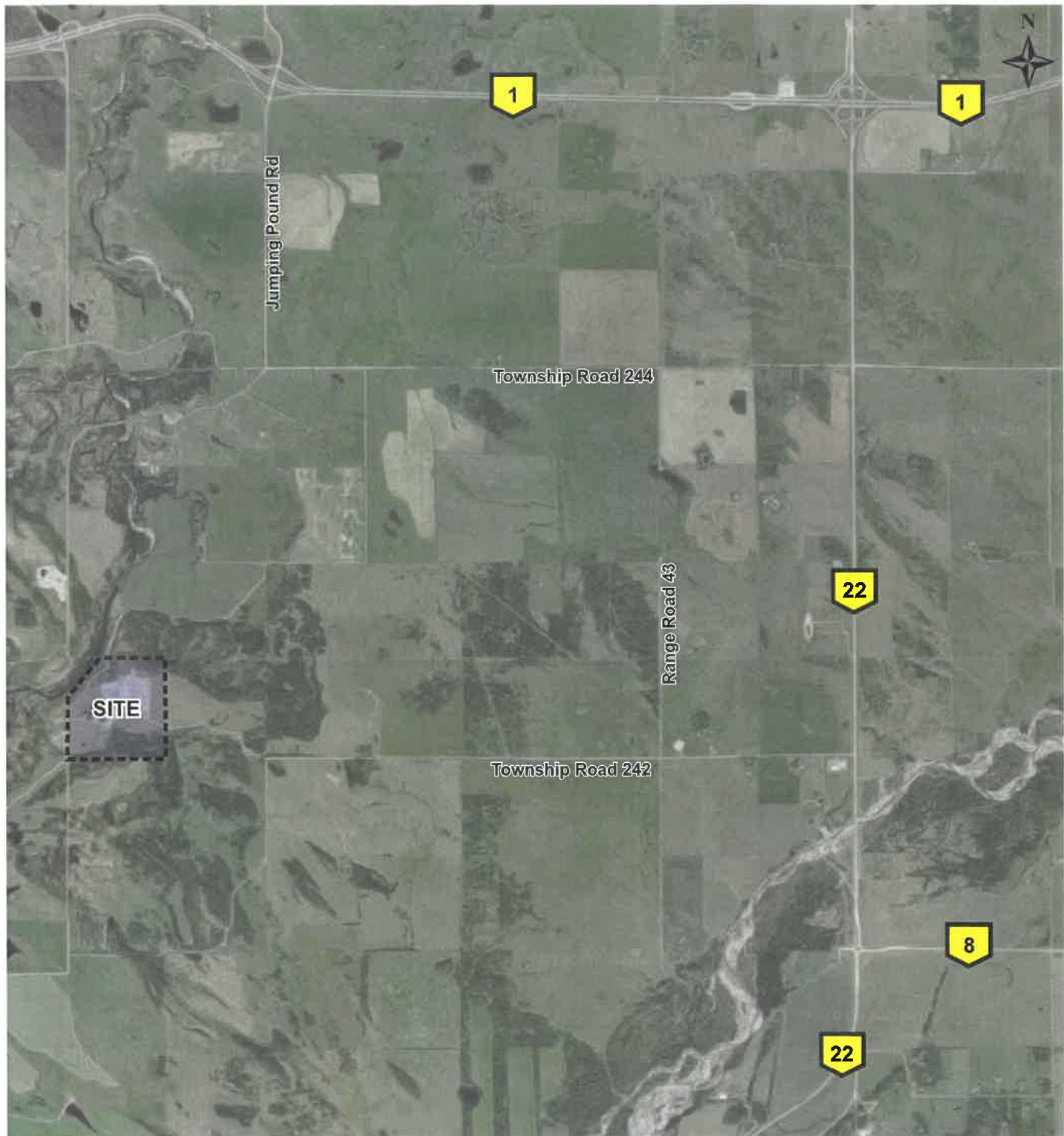
Based on discussions with Rocky View County and Alberta Transportation, the scope of work for the original TIA was confirmed as follows, noting that the analysis horizons have been adjusted for this TIA update:

- Determine expected site generated traffic volumes for the proposed uses based on first principles and in-house data.
- Assign expected site generated traffic volumes to the road network based on anticipated site traffic distribution.
- Complete capacity analysis for Opening Day (2020), and Long Term (2040) horizons during the AM and PM Peak hour at the following intersections:
 - Highway 22 & Township Road 242.
- Complete sight distance analysis.
- Review collision data at the intersection of Highway 22 & Township Road 242.
- Complete intersection type and illumination warrants for Highway 22 & Township Road 242.
- Identify transportation improvements required to support the development.

The study scope correspondence is included in **Appendix A**.

2.2 Site Context

The subject lands are zoned as NRI (Natural Resource Industrial District). Vehicular access to the development is provided from Township Road 242 via Highway 22. The site context and adjacent external road network is illustrated in **Exhibit 2.1**.



Base Map Source: Google Maps

Exhibit 2.1 Site Context

Copithorne Gravel Pit TIA Update
June 2020 Scale NTS



3. BACKGROUND TRAFFIC CONDITIONS

To assess the impacts of the proposed development on existing infrastructure, Bunt & Associates established the impacts of background traffic without consideration of site traffic. This provided the basis for comparing the incremental impact due to the proposed development.

3.1 Existing Road Network

The roadways in the vicinity of the site are described below:

- **Highway 22** is a two-lane undivided provincial Highway (control section #14) that runs in the north-south direction with a posted speed limit of 100 km/h near Township Road 242. The roadway's current cross-section near Township Road 242 has one driving lane per direction with shoulders on both sides of the roadway. Illumination is not provided on the roadway except near major intersection approaches (Highway 8 roundabout intersection; Highway 1 interchange).
- **Township Road 242** is a two-lane unpaved County roadway that runs the east-west direction from the site to Highway 22. The roadway has a right-of-way (ROW) of 20 metres and a posted a speed limit of 60 km/h. Non-regulatory signage is provided stating "film crew reduce speed 50 km/h." No illumination is provided on the roadway. The roadway is illustrated in **Figure 3.1** and **Figure 3.2**.

Figure 3.1: Township Road 242 (Facing west from Highway 22)



Figure 3.2: Township Road 242 at Range Road 43 (Facing East)



3.2 Existing Lane Configurations & Traffic Control

The following lane configurations and traffic control are in place at study area intersections:

- *Highway 22 & Township Road 242* – The intersection is unsignalized with stop control in the eastbound direction. Type I intersection treatment is provided.

The following other major intersections are located along Highway 22:

- *Highway 22 & Highway 1* – A cloverleaf interchange is provided. (Parclo interchange improvement is underway)
- *Highway 22 & Highway 8* – A single lane roundabout controls the intersection.

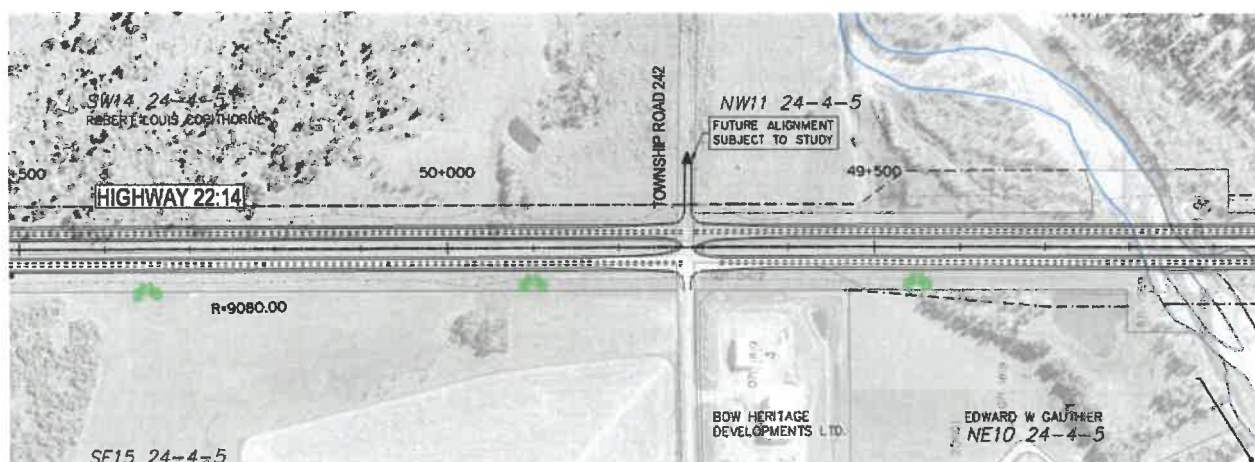
3.3 Highway 22 Functional Planning Study

ISL Engineering completed a *Highway 22 Functional Planning Study*¹ for Alberta Transportation. The functional planning study recommended the following:

- *At the interim stage (2030)* – Highway 22 be twinned to a four-lane divided arterial and be connected to Highway 1 via a newly upgraded Parclo A interchange. The design contains auxiliary turn lanes at intersections including Township Road 242.
- *At the ultimate stage (2050)* – Highway 22 be upgraded to a six-lane cross-section with auxiliary turn lanes at intersections.

The ultimate Highway 22 cross-section near Township Road 242 is illustrated in **Figure 3.3**.

Figure 3.3: Ultimate Highway 22 Cross-Section (Oriented to the East)



The traffic projections provided in the March 2014 report are summarized in **Table 3.1** and compared to Alberta Transportation AADT (Average annual daily traffic) data.

Table 3.1: Functional Study – Daily Traffic Projections (vehicles per day)

HIGHWAY	FUNCTIONAL STUDY HORIZONS				AT AADT DATA		
	2014	2030	2050 (Scenario 1)	2050 (Scenario 2)	2014	2015	2019
Highway 22 (South of Hwy 1)	9,700	15,200	20,000	22,000	10,340	12,140	10,140

¹ *Highway 22 Functional Planning Study south of Highway 8 to Town of Cochrane Corporate Limits*, ISL Engineering and Land Services, March 2014.

http://www.transportation.alberta.ca/projects/assets/Area_7_Calgary_Area/Hwy_22_twinning_from_Hwy_8_to_Cochrane/Executive%20Summary.pdf

Alberta Transportation AADT data indicates traffic volumes on Highway 22 had a spike in growth in 2015 and since then have been decreasing annually, resulting in an overall growth of approximately 1.5% per year when compared to the “current volumes” stated in the March 2014 functional study. If this rate of growth is to continue, twinning would be required at a later stage than the 2030 horizon provided in the functional study.

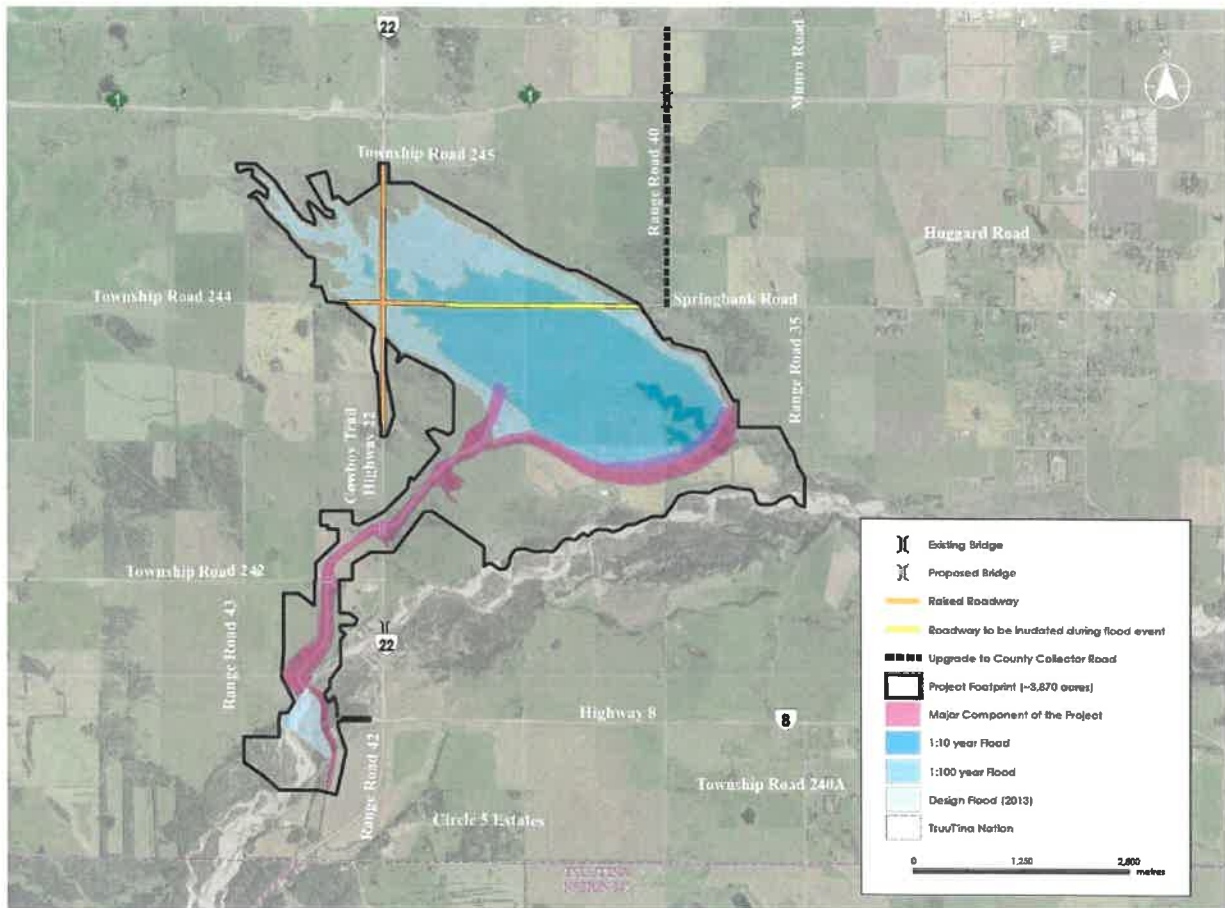
3.4 Springbank Off-stream Reservoir Project

The Springbank Off-stream Reservoir Project (SR1) is in development and will result in changes to the existing road network along Highway 22, Springbank Road, and Township Road 242. The project will result in the following new infrastructure:

- Raise portion of Highway 22 north and south of Springbank Road and shift west to accommodate future twinning.
- Raise portion of Springbank Road east and west of Highway 22.
- New bridge to be constructed along Highway 22, north of Township Road 242.
- New bridge to be constructed along Township Road 242, west of Highway 22.

The above changes to the road network will not impact the analysis completed in this study, nor will impact the intersection of Highway 22 & Township Road 242 in terms of geometry and operation. The proposed SR1 upgrades are presented graphically in **Figure 3.4**.

Figure 3.4: Springbank Off-stream Reservoir Project Proposed Upgrades



3.5 Collision Data

Safety performance at Highway 22 & Township Road 242 was reviewed based on data obtained from Alberta Transportation for the latest 10-Year period available at the time of the original TIA (2003 to 2012). This data is now supplemented with new available data from years up to and including 2016. There were a total of 5 collisions reported at Highway 22 & Township Road 242 between 2003 – 2016 as summarized in **Table 3.2**. The raw collision data is included in **Appendix B**.

Table 3.2: Highway 22 & Township Road 242 Collisions

YEAR	# OF COLLISIONS	TYPE OF COLLISION REPORTED
2004	1	Struck Object – Animal
2005	1	Head-on (2 fatalities)
2009	1	Run-off-road
2013	1	Struck Object – Animal
2015	1	Rear-end (non injury)

The 2005 collision involved 3 vehicles with 4 injuries and 2 fatalities. The collision report states "V3 slowed for left turning vehicle in front; V1 struck rear of V3; V3 went into ditch; V1 crossed centre line and struck head-on into opposing V2." The 2004, 2009, and 2013 collisions were single-vehicle collisions. The 2015 collision was a rear-end collision of a right turning truck from Highway 22.

3.6 Sight Distance Requirements

A sight distance review was undertaken at Highway 22 & Township Road 242 based on the *Alberta Transportation Highway Geometric Guide* and *TAC Geometric Design Guide for Canadian Roads* to confirm the safety of turning movements and through movements on Highway 22. Sight distance requirements are based on the following:

Minimum Stopping Sight Distance (SSD), which is the distance a vehicle travels from the instant the driver sights an object and decides to stop, to the instant the vehicle comes to a complete stop after applying breaks. This distance is usually sufficient to allow reasonably competent and alert drivers to come to a hurried stop under ordinary conditions. Minimum stopping sight distances based on speeds are:

- 100 km/h = 200 metres
- 110 km/h = 235 metres

Desirable Decision Sight Distance (DSD), which is utilized in complex situations and is the distance required for a driver to detect an information source or hazard that is difficult to perceive in a roadway environment that might be visually cluttered, recognize the hazard or its threat potential, selection an appropriate action, and complete the manoeuvre safely and efficiently. A range of distances is provided with lower ranges appropriate for less complex situations and the higher range appropriate for more complex situations. Decisions sight distances based on speeds are:

- 100 km/h = 300 to 390 metres
- 110 km/h = 330 to 430 metres

Intersection Sight Distance (ISD), which is defined as the sight distance required for a vehicle to complete either a crossing or turning manoeuvre safely. Intersection sight distances based on speeds and vehicle types are:

- 100 km/h = 210 metres for passenger vehicle and 270 metres for a single-unit truck design vehicle
- 110 km/h = 230 metres for passenger vehicle and 290 metres for a single-unit truck design vehicle

Assuming a design speed of 110 km/h (posted speed limit of 100 km/h) along Highway 22, the sight distance requirements at the study area intersections are outlined in **Table 3.3**.

Table 3.3: Intersection Sight Distance

INTERSECTION	DESIGN SPEED	SSD	DSD	ISD		AVAILABLE SIGHT DIST.	
				Car	SU Truck	To West	To East
Hwy 22 & TWP Rd 242	110km/h	235m	330-430m	230m	290m	>500m	>500m

The review confirms all minimum and desirable sight distance requirements are met at Highway 22 & Township Road 242.

3.7 Traffic Volumes

3.7.1 Existing Data

Bunt & Associates conducted intersection turning movement counts on Wednesday February 17, 2016 morning (7:00-9:00), mid-day (11:00-13:00), and afternoon (16:00-18:00) periods to document existing traffic movements at the intersection of Highway 22 & Township Road 242. This data is used in this study only to estimate the existing turning movement volumes in and out of Township Road 242 to/from Highway 22. The observed 2016 through volumes at this intersection were disregarded and replaced by 2019 Alberta Transportation 100th Highest Hour counts of Highway 22 & Highway 8. Traffic count data is provided in **Appendix C**.

3.7.2 Background Growth

Background growth is the result of traffic that would be present on the road network in future years regardless of the development of the site. This traffic is representative of yearly growth on the roadways as well as other residential, commercial, or industrial developments that have been approved in the area.

Alberta Transportation AADT data for Highway 22 (north of Highway 8) and Highway 8 (east of Highway 22) was obtained as summarized in **Table 3.4**.

Table 3.4: Highway 22 AADT Data

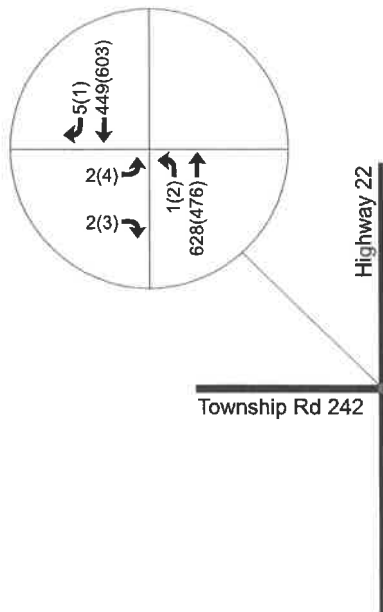
YEAR	HIGHWAY 22 AADT (NORTH OF HIGHWAY 8)	HIGHWAY 8 AADT (EAST OF HWY 22)
2002	-	6,872
2003	-	6,942
2004	-	6,860
2005	-	6,656
2006	-	6,539
2007	9,350	6,510
2008	9,070	6,313
2009	9,110	6,385
2010	9,440	6,627
2011	9,300	6,560
2012	9,300	6,459
2013	9,480	6,396
2014	9,520	6,525
2015	11,320	7,131
2016	10,900	7,450
2017	10,820	7,230
2018	9,930	5,980
2019	9,290	4,960
5-Year Growth Rate (2014-2019)	-0.5% per year	-4.7% per year
All Data Growth Rate to 2019	-0.1% per year	-1.6% per year
5-Year Growth Rate (2012-2017)	3.3% per year	2.4% per year
All Data Growth Rate to 2017	1.6% per year	0.3% per year

As shown in the above table, the most recent 5-year growth rate (2014-2019) exemplifies that traffic is currently decreasing annually along Highway 22 and Highway 8. Traffic volumes along Highway 22 are expected to experience stifled growth in the coming years due to the inclusion of the Calgary Southwest Ring Road project that will pull existing traffic from Highway 8 and Highway 22 instead to Highway 1 exclusively. With this noted, some growth is still expected to occur along Highway 22 over the coming years; therefore a growth rate of 1.5% per year was developed by averaging all data up to 2017, before a significant drop in traffic volumes was observed.

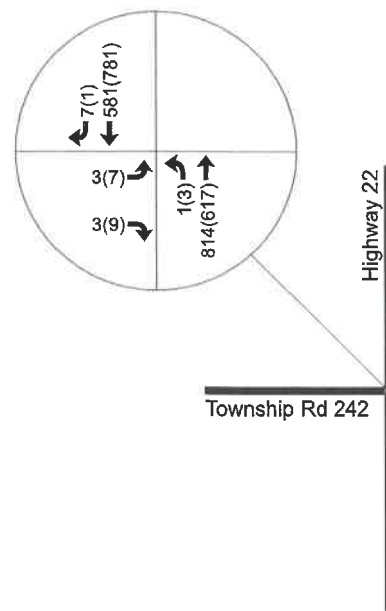
This linear growth rate is applied to all movements at the intersection of Highway 22 & Township Road 242 to develop 2020 and 2040 Background volumes. The resulting Background traffic volumes are summarized in **Exhibit 3.1**.



2020 Background Traffic Volumes



2040 Background Traffic Volumes



LEGEND

↙ ↑ ↘	Vehicle Volumes
XX	AM Peak Hour
(YY)	PM Peak Hour

Exhibit 3.1

Background Traffic Volumes

Copithorne Gravel Pit TIA Update
June 2020 Scale NTS



3.8 Intersection Capacity Analysis

Intersection capacity analysis was undertaken for the study area intersections using Synchro 8.0, a traffic analysis software package based on the methods outlined in the Highway Capacity Manual (HCM) 2000. This model uses standard procedures to determine the Volume to Capacity (v/c) and the corresponding delay-based traffic Level of Service (LOS) for movements at each intersection in the study network.

Alberta Transportation's guidelines for unsignalized intersections accept a v/c ratio of 0.85 or less and a LOS C at highway access intersections with a LOS D on any single approach at full build-out depending on location.

For unsignalized intersections, the LOS methodology considers intersection geometry, traffic volumes, speed limit, and type of intersection control. For signalized intersections, the LOS methodology considers intersection geometry, traffic volumes, speed limit, and signal timing plan. Delays range from LOS 'A' conditions with minimal delay to LOS 'F' which represents significant control delay. The LOS criteria for unsignalized and signalized intersections are summarized in **Table 3.5**.

Table 3.5: HCM Level of Service Summary

LEVEL OF SERVICE (LOS)	AVERAGE CONTROL DELAY FOR UNSIGNALIZED INTERSECTION MOVEMENTS	AVERAGE CONTROL DELAY FOR SIGNALIZED INTERSECTION MOVEMENTS
A	≤ 10 seconds per vehicle	≤ 10 seconds per vehicle
B	> 10 – 15 seconds per vehicle	> 10 – 20 seconds per vehicle
C	> 15 – 25 seconds per vehicle	> 20 – 35 seconds per vehicle
D	> 25 – 35 seconds per vehicle	> 35 – 55 seconds per vehicle
E	> 35 – 50 seconds per vehicle	> 55 – 80 seconds per vehicle
F	> 50 seconds per vehicle	> 80 seconds per vehicle

The results of the intersection capacity analysis were based on expected traffic volumes, traffic control, and lane arrangement at the study intersections. The analysis is completed as per Alberta Transportation TIA guidelines with a saturation flow of 1850 vehicles per hour and peak hour factor of 0.93.

The volume to capacity ratio, level of service, average control delay (in seconds), and 95th percentile queues (in metres) are summarized in the body of this report. Synchro output summaries are provided in **Appendix D**.

The Background intersection capacity analysis is summarized in **Table 3.6**. The 2020 analysis is completed based on existing lane configurations. The 2040 analysis is completed based on both the existing lane configuration and with a twinned Highway 22 as per the recommendations from the *Highway 22 Functional Planning Study*.

Table 3.6: Background Intersection Capacity Analysis (Highway 22 & Township Road 242)

INTERSECTION	MOVEMENT & LANES		AM PEAK HOUR				PM PEAK HOUR			
			v/c	LOS	Delay	Queue	v/c	LOS	Delay	Queue
2020 Background	EB	1	0.03	C	18	<5	0.05	C	18	<5
	NB	1	0.01	A	0	<5	0.01	A	0	<5
	SB	1	0.29	A	0	<5	0.38	A	0	<5
	Overall		-	A	0.2	-	-	A	0.3	-
2040 Background (Existing Hwy 22)	EB	1	0.05	C	25	<5	0.09	D	25	<5
	NB	1	0.01	A	0	<5	0.01	A	0	<5
	SB	1	0.37	A	0	<5	0.49	A	0	<5
	Overall		-	A	0.3	-	-	A	0.4	-
2040 Background (Twinned Hwy 22)	EB	1	0.03	C	17	<5	0.06	C	19	<5
	NB	2	0.34	A	0	<5	0.26	A	0	<5
	SB	2	0.24	A	0	<5	0.33	A	0	<5
	Overall		-	A	0.2	-	-	A	0.3	-

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the 2020 and 2040 horizons in both the existing geometry and twinned highway scenarios.

3.9 Road Link Analysis

The volume of daily traffic on a roadway is one of the factors that aids in determining road classifications and appropriate lane requirements. To confirm roadway capacities, the background daily traffic volumes were calculated in vehicles per day (vpd) and compared to the environmental guidelines from the *Alberta Transportation Highway Geometric Design Guide* and *Rocky View County Servicing Standards*. The environmental guidelines represent the limit of comfortable operation of the roadway under most conditions, while the actual physical capacity of the roadway can be considerably higher.

The analysis is completed assuming a 1.5% growth rate on Highway 22. Traffic growth on Township Road 242 will be from development (i.e. the proposed gravel pit) and therefore is included in the Post Development analysis. The results of the daily link analysis are summarized in **Table 3.7**.

Table 3.7: Background Road Link Analysis

ROADWAY LINK	CLASSIFICATION	GUIDELINE (VPD)	BACKGROUND DAILY TRAFFIC VOLUMES (VPD)		
			2019	2020	2040
Highway 22 North of Hwy 8	2-Lane Highway	~12,000	9,300	9,400	12,200
Township Road 242 West of Hwy 22	2-lane Regional Moderate Volume	<500	115	No change	No change

The road link analysis suggests twinning of Highway 22 may be required by the 2040 Background horizon. This matches with the recommendations from the *Highway 22 Functional Planning Study*.

3.10 Alberta Transportation Warrants

3.10.1 Intersection Turning Warrants

Intersection type warrant was completed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The AT intersection warrant analysis is utilized at unsignalized at-grade intersections to determine if a left turn is required to eliminate interference caused by standing vehicles waiting to turn or a right turn lane is required to reduce obstruction to through movements.

Right Turn Warrants

The AT exclusive right turn lane warrant for a two-lane undivided highway states that three separate conditions should generally be met in order to warrant the need for such a lane. These criteria are:

- Average Annual Daily Traffic (AADT) volumes on the main road are greater than or equal to 1,800 vehicles per day (vpd);
- The intersected road exhibits daily traffic volumes greater than or equal to 900 vpd; and
- The right turn movement in question is greater than or equal to 360 vpd.

Based on this criteria, an exclusive right is not warranted as the intersected road daily traffic volumes will remain less than 900 vpd and the right turn movement in question will remain less than 360 vpd.

Left Turn Warrants

According to AT guidelines, the following two conditions should generally be met to warrant the need to construct an exclusive left turn taper and/or by-pass through lane on a two-lane highway:

- The peak hour opposing traffic volumes are greater than or equal to 100 vpd; and
- A minimum of five percent of advancing traffic is left turning during the peak hour periods.

Opposing traffic volumes are greater than 100 vpd in all horizons. However, left turning volumes are below 1% of advancing volumes. Since the left turning percentage is less than 5%, an upgraded intersection is not required based on technical warrant analysis, however AT guidelines state that engineering judgement may be used on roadways where main road volumes are greater than 4,000 vehicles per day, such as Highway 22, to determine the appropriate intersection treatment.

In the original TIA, Bunt & Associates recommended a Type IIIa intersection to provide separation from the through lane for vehicles turning left off Highway 22, this supported by a high number of through traffic volumes along Highway 22. Since the original TIA, it has been observed that traffic volumes along Highway 22 are not growing at as large of a rate initially projected, and that traffic volumes are lower now than they were in 2016, as well, the Rocky View County gravel pit that once operated along Township Road 242 is no longer in operation. Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

3.10.2 Illumination Warrant

A street light illumination warrant calculation was completed at the Highway 22 & Township Road 242 based on the *Transportation Association of Canada Illumination of Isolated Rural Intersections* guidelines, which are referred to in the AT Highway Lighting Guide. The warrant for illumination is used to determine if lighting an intersection is required based on several different factors such as geometrics, operations, environmental issues as well as collision history. The analysis (included in **Appendix E**) confirmed illumination is not warranted at any background horizon.

4. PROPOSED DEVELOPMENT

4.1 Land Use Information

The development permit is for a gravel pit operation. As per the development permit conditions, the hours of operation of the gravel pit (including aggregate excavating, hauling, crushing, stock piling, and stripping of overburden) are as follows:

- Hauling is permitted only from 7:00-17:00 on Monday-Saturday;
- Crushing is permitted only from 7:00-19:00 on Monday-Saturday; and
- No operation will occur on Sundays and statutory holidays.

The facility will operate with the following constraints:

- All traffic will access the site via the parcel to the east, which is also owned by Mr. Copithorne. Traffic will travel to Highway 22 via Township Road 242. This is the current route used by previous operations on the site.
- All outbound truck traffic will turn right at Highway 22 and proceed to the south on Highway 22. Trucks will enter the site from both the north and south on Highway 22

The facility will have the following number of employees:

- A total of 2 full time employees will work on-site, there are no extra employees when crushing is occurring.
- No shift changes will occur.

Based on the 2019 haul data for the gravel pit provided in **Appendix C**, the facility will have the following truck trips:

- A maximum of 45 loads in one hauling day, which results in up to 90 truck trips per day (45 In, 45 Out). It is noted that this was the busiest day for the site in 2019, and that the average amount of loads processed on haul days in 2019 was 13. The truck trip generation for the site is therefore based on the worse-case scenario.
- All activity expected to be completed by 5 truck drivers.

4.2 Trip Generation

Trip generation for the proposed development permit was calculated based on first principles. The vehicular trip generation rates used in this study are as follows:

Employees (Passenger vehicle trips) – Based on employment hours, it is expected that both full time employees will arrive before the AM peak hour (7:30-8:30) and leave during the PM peak hour (16:15-

17:15). To be conservative, it is assumed that the employees arrive during the AM peak hour and leave during the PM peak hour.

- AM Peak Hour: 2 trips (2 In, 0 Out)
- PM Peak Hour: 2 trips (0 In, 2 Out)
- Daily: 4 trips (2 In, 2 Out)

Hauling (Truck trips) – Based on site operation information, a worst-case total of 90 daily trips are expected over the daily 10-hour operating period. This results in 9 truck trips during both peak hours ($90/10 = 9$).

- AM Peak Hour: 9 trips (5 In, 4 Out)
- PM Peak Hour: 9 trips (4 In, 5 Out)
- Daily: 90 trips (45 In, 45 Out)

Vehicular trip generation for the proposed development is summarized in **Table 4.1**.

Table 4.1: Trip Generation

LAND USE	AM PEAK HOUR			PM PEAK HOUR			DAILY		
	Total	In	Out	Total	In	Out	Total	In	Out
Passenger Vehicle Trips	2	2	0	2	0	2	4	2	2
Truck Trips	9	5	4	9	4	5	90	45	45
Total Trips	11	7	4	11	4	7	94	47	47

4.3 Trip Distribution

Site traffic was distributed to the roadway network based on previous traffic patterns and indicated truck routing (i.e. trips oriented to the north/east will use the Highway 8 roundabout intersection to either make a U-turn or turn left). The trip distribution utilized is summarized in **Table 4.2**.

Table 4.2: Gravel Pit Trip Distribution

DIRECTION	AM PEAK HOUR		PM PEAK HOUR	
	In	Out	In	Out
Employee Trips				
To/from the north on Highway 22	40%	40%	40%	40%
To/from the south on Highway 22	60%	60%	60%	60%
Total	100%	100%	100%	100%
Truck Trips				
From the north on Highway 22	50%	-	50%	-
To/from the south on Highway 22	50%	80%	50%	80%
To north on Highway 22 via Hwy 8 roundabout	-	20%	-	20%
Total	100%	100%	100%	100%

5. POST DEVELOPMENT TRAFFIC CONDITIONS

Site generated traffic volumes were superimposed onto background traffic volumes to obtain Post Development traffic volumes as summarized in **Exhibit 5.1**. The Long Term analysis is completed assuming the site will still be operating by 2040.

5.1 Intersection Capacity Analysis

The Post Development intersection capacity analysis is summarized in **Table 5.1**.

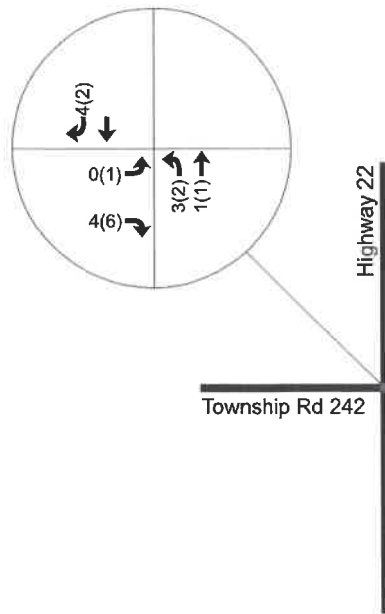
Table 5.1: Post Development Intersection Capacity Analysis (Highway 22 & Township Road 242)

INTERSECTION	MOVEMENT & LANES		AM PEAK HOUR				PM PEAK HOUR			
			v/c	LOS	Delay	Queue	v/c	LOS	Delay	Queue
2020 Post Development	EB	1	0.04	C	17	<5	0.08	C	18	<5
	NB	1	0.01	A	0	<5	0.01	A	0	<5
	SB	1	0.29	A	0	<5	0.38	A	0	<5
	Overall		-	A	0.3	-	-	A	0.4	-
2040 Post Development (Existing Hwy 22)	EB	1	0.07	C	22	<5	0.12	C	25	<5
	NB	1	0.01	A	0	<5	0.01	A	0	<5
	SB	1	0.38	A	0	<5	0.49	A	0	<5
	Overall		-	A	0.4	-	-	A	0.5	-
2040 Post Development (Twinned Hwy 22)	EB	1	0.04	C	16	<5	0.08	C	18	<5
	NB	2	0.34	A	0	<5	0.26	A	0	<5
	SB	2	0.24	A	0	<5	0.33	A	0	<5
	Overall		-	A	0.2	-	-	A	0.4	-

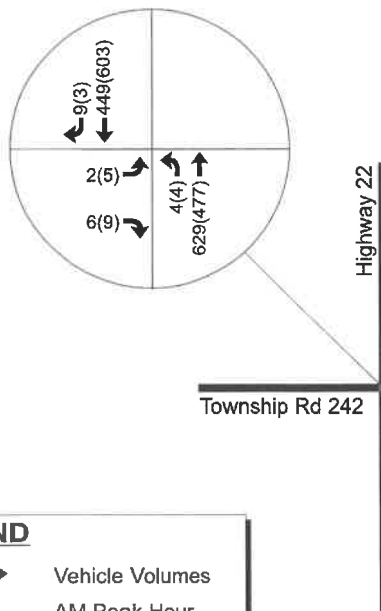
The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.



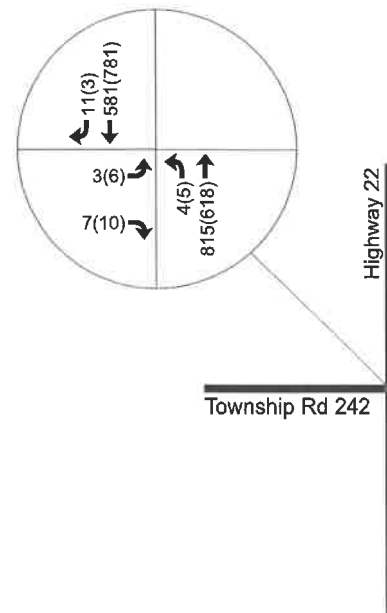
Site Traffic Volumes



2020 Post Development Traffic Volumes



2040 Post Development Traffic Volumes



LEGEND

↶ ↷ ↸	Vehicle Volumes
XX	AM Peak Hour
(YY)	PM Peak Hour

Exhibit 5.1

Post Development Traffic Volumes

Copithorne Gravel Pit TIA Update
June 2020 Scale NTS



5.2 Road Links

5.2.1 Daily Volume Analysis

Post development daily traffic volumes are summarized in **Table 5.2**. The volumes presented for Township Road 242 represent the average and peak hauling days and not average annual daily traffic (AADT), which would be less than the average haul day as the site would only be operating half of the year.

Table 5.2: Post Development Road Link Analysis

ROADWAY LINK	CLASSIFICATION	GUIDELINE (VPD)	POST DEVELOPMENT DAILY TRAFFIC VOLUMES		
			2019	2020	2040
Highway 22 North of Hwy 8	2-Lane Highway	~12,000	9,400	9,500	12,300
Township Road 242 West of Hwy 22	2-lane Regional Moderate Volume	<500	145 (Average) 210 (Peak)	No change	No change

The analysis confirmed all roadway links in the study area would continue to carry traffic volumes within their respective environmental guidelines after the addition of the proposed development. Therefore, current roadway classifications are adequate to accommodate site traffic volumes.

5.2.2 Township Road 242

Township Road 242 is expected to have an AADT of less than 145 vehicles per day. AADT calculations would normally consider that the proposed gravel pit operations will only operate half of the year. However, this calculation assumes the proposed gravel pit will operate at capacity over the entire operating season.

Due to truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.

5.3 Alberta Transportation Warrants

5.3.1 Intersection Turning Warrants

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

5.3.2 Illumination Warrant

A street light illumination warrant calculation was completed at the Highway 22 & Township Road 242 confirmed illumination is not warranted at both the 2020 and 2040 Post Development horizons.

6. FINDINGS AND RECOMMENDATIONS

The findings and recommendations made as a result of the TIA update are listed below:

6.1.1 Collision History

A total of 5 collisions occurred at the intersection over a 14-year period. Three of the collisions were single-vehicle (struck animal; run-off-road). A collision occurred in 2005 that involved 3 vehicles with 4 injuries and 2 fatalities. A 2015 rear-end collision involving 2 trucks is also documented.

6.1.2 Sight Distance

All minimum and desirable sight distance requirements are met at the intersection.

6.1.3 Background Intersection Capacity Analysis

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the Opening Day (2020) and Long Term (2040) horizons.

6.1.4 Post Development Intersection Capacity Analysis

The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.

6.1.5 Alberta Transportation Intersection Warrant

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

6.1.6 Alberta Transportation Illumination Warrant

Street light illumination is not warranted at the intersection.

6.1.7 Highway 22

The Background road link analysis suggests twinning of Highway 22 may be required by the Long Term Background horizon (2040). This matches with the recommendations from the *Highway 22 Functional Planning Study*. No appreciable change will occur due to development traffic volumes.

6.1.8 Township Road 242

The roadway is expected to continue operating within its daily vehicle capacity. Due to the expected truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.

APPENDIX A

Scope of Work

Friday, 12 February, 2016 2:16:54 PM Mountain Standard Time

Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242
Date: Monday, 8 February, 2016 9:50:19 AM Mountain Standard Time
From: AYurkowski@rockyview.ca
To: Ezekiel Dada
CC: Trevor.Richelhof@gov.ab.ca, Amrit Uppal

Thanks Ezekiel, sounds good.

Angela

ANGELA YURKOWSKI (BOOTH), P.ENG
Municipal Engineer | Engineering Services

ROCKY VIEW COUNTY
911 - 32 Avenue NE | Calgary | AB | T2E 6X6
Phone: 403-520-7289
ayurkowski@rockyview.ca | www.rockyview.ca

-
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From: Ezekiel Dada [mailto:edada@bunteng.com]
Sent: Friday, February 5, 2016 10:31 AM
To: Angela Yurkowski
Cc: Trevor.Richelhof@gov.ab.ca; Amrit Uppal
Subject: Re: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Angela,

Thanks for your reply. We will prepare the TIA to meet both the County and AT's TIA guidelines.

Regarding horizon, we plan on analyzing "Opening Day" that is, existing plus site traffic and 20-year, that is, Highway 22 traffic plus growth plus site traffic (assuming the gravel pit can be mined for more than 20 years).

Thank you.

Ezekiel

From: "AYurkowski@rockyview.ca" <AYurkowski@rockyview.ca>
Date: Friday, February 5, 2016 at 9:54 AM

To: Ezekiel Dada <edada@bunteng.com>
Cc: Amrit Uppal <auppal@bunteng.com>, Trevor Richelhof <Trevor.Richelhof@gov.ab.ca>
Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Ezekiel,

The county requirements for TIA's are outlined in our servicing standards (Section 401).

<http://www.rockyview.ca/Portals/0/Files/BuildingPlanning/Standards/Servicing-Standards.pdf>

Specifically for this application, we want to see a review of TWP 242 to confirm appropriate road designation and identify any road improvements that may be needed based on volume and vehicle classification.

Also, what horizons will you be looking at?

Thanks

ANGELA YURKOWSKI (BOOTH), P.ENG
Municipal Engineer | Engineering Services

ROCKY VIEW COUNTY
911 - 32 Avenue NE | Calgary | AB | T2E 6X6
Phone: 403-520-7289
ayurkowski@rockyview.ca | www.rockyview.ca

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From: Ezekiel Dada [<mailto:edada@bunteng.com>]
Sent: Thursday, February 4, 2016 3:45 PM
To: Trevor Richelhof; Angela Yurkowski
Cc: Vince Diot; Jerry Lau; Clarke Bullock; Amrit Uppal
Subject: Re: Scope of TIA - Copithorne Gravel Pit at Twp 242

Thanks Trevor,

Ezekiel

From: Trevor Richelhof <Trevor.Richelhof@gov.ab.ca>
Date: Thursday, February 4, 2016 at 3:31 PM
To: Ezekiel Dada <edada@bunteng.com>, "ayurkowski@rockyview.ca" <ayurkowski@rockyview.ca>
Cc: "vdidot@rockyview.ca" <vdidot@rockyview.ca>, Jerry Lau <Jerry.Lau@gov.ab.ca>, Clarke Bullock <clarke.bullock@gov.ab.ca>, Amrit Uppal <auppal@bunteng.com>
Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242

Ezekiel, for the intersection of Highway 22 and Township Road 242, I would recommend the scope of

the study follows the attached guideline, in addition to any other requirements of Rocky View County.

Regards,

Trevor Richelhof

Development / Planning Technologist
Delivery Services, Southern Region
Alberta Transportation
Government of Alberta
2nd Floor, 803 Manning Road NE, Calgary AB T2E 7M8

Tel 403-297-6311

Fax 403-297-7682

Trevor.Richelhof@gov.ab.ca

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From: Ezekiel Dada [<mailto:edada@bunteng.com>]

Sent: Thursday, February 04, 2016 2:59 PM

To: ayurkowski@rockyview.ca; Trevor Richelhof

Cc: vdilot@rockyview.ca; Jerry Lau; Clarke Bullock; Amrit Uppal

Subject: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Angela and Trevor,

We have been retained by Roy Copithorne to provide a TIA requested by the County in support of a development application for ongoing gravel pit operation located west of Highway 22 on Township Road 242. I attached the Development Appeals Board order for your reference.

Please find below our suggested scope for the study and let me know if it is acceptable to the County and Alberta Transportation.

?

? **Undertake 6-hour weekday intersection traffic counts for the following intersections:**

? Highway 22 & Twp 242

? Range road 45 & Twp 242 (if necessary)

Up-to-date road information, including traffic delays, is a click or a call away. Call 5-1-1 toll-free, visit 511.alberta.ca or follow us on Twitter [@511Alberta](https://twitter.com/511Alberta) to get on the road to safer travel.

<http://511.alberta.ca/ab/en.html>

<https://twitter.com/511Alberta>

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APPENDIX B

Collision Data

[illegible]

APPENDIX C

Traffic Data



Intersection Turning Movement Count Summary:

Count Date:		Count Date:	
N/S Road:	Highway 22	AM Peak Hr:	7:30 AM to 8:30 AM
E/W Road:	Township Road 24-2	Mid-day Peak Hr:	11:00 AM to 12:00 PM
	February 17, 2016	PM Peak Hr:	4:15 PM to 5:15 PM

Count Date: February 17, 2016
Wednesday

Weather: Clear

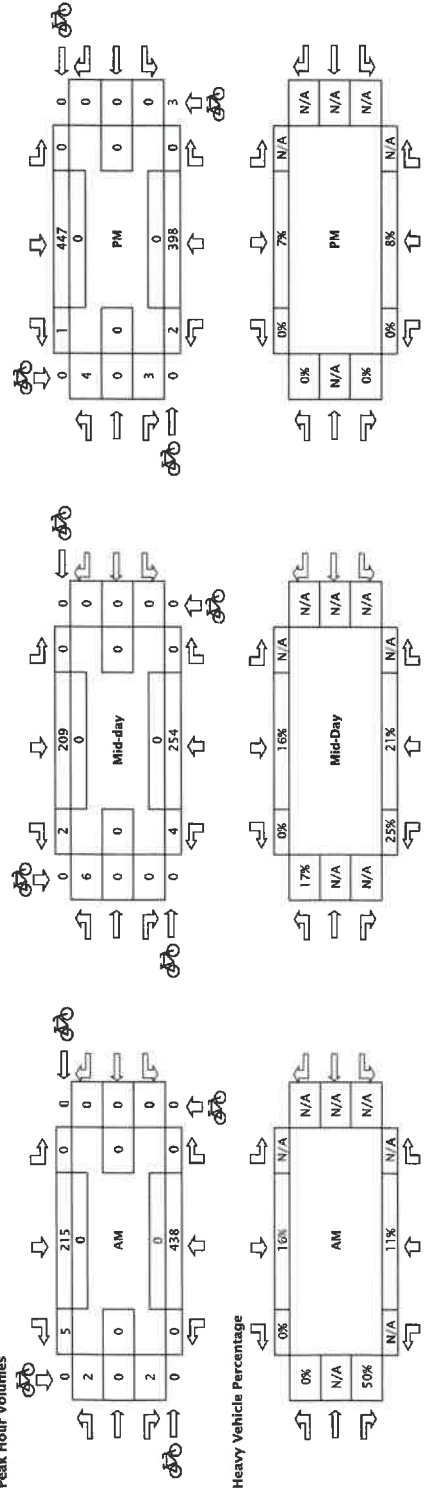
Road Cond: Good

Project #: 1202-07

PHF (AM Peak Hr):	0.95
PHF (Mid-day Peak Hr):	0.92
PHF (PM Peak Hr):	0.93

[illegible]

Peak Hour Volumes



Turning Movement Summary Diagram

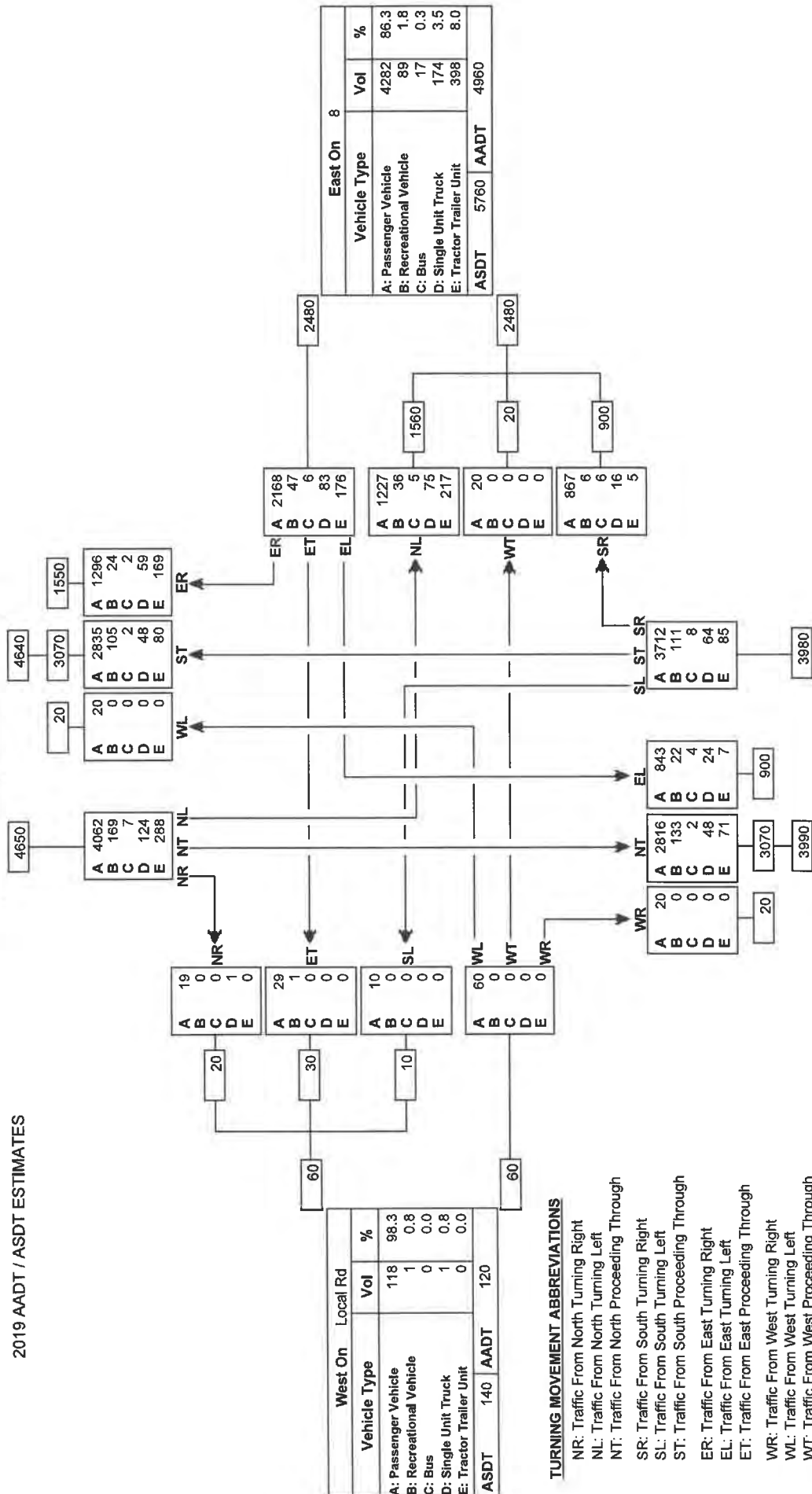
North On 22		
Vehicle Type	Vol	%
A: Passenger Vehicle	8213	88.4
B: Recreational Vehicle	298	3.2
C: Bus	11	0.1
D: Single Unit Truck	231	2.5
E: Tractor Trailer Unit	537	5.8
ASDT	10790	AADT 9290

2019 AADT / ASDT ESTIMATES

Reference No.: 60200

Intersection of:

8 & 22 NE OF BRAGG CREEK

**TURNING MOVEMENT ABBREVIATIONS**

AADT: Annual Average Daily Traffic

Average daily traffic expressed as vehicles per day for period of January 1 to December 31 (365 days)

ASDT: Average Summer Daily Traffic

Average daily traffic expressed as vehicles per day for period of May 1 to September 30 (153 days)

Turning Movement Summary Diagram

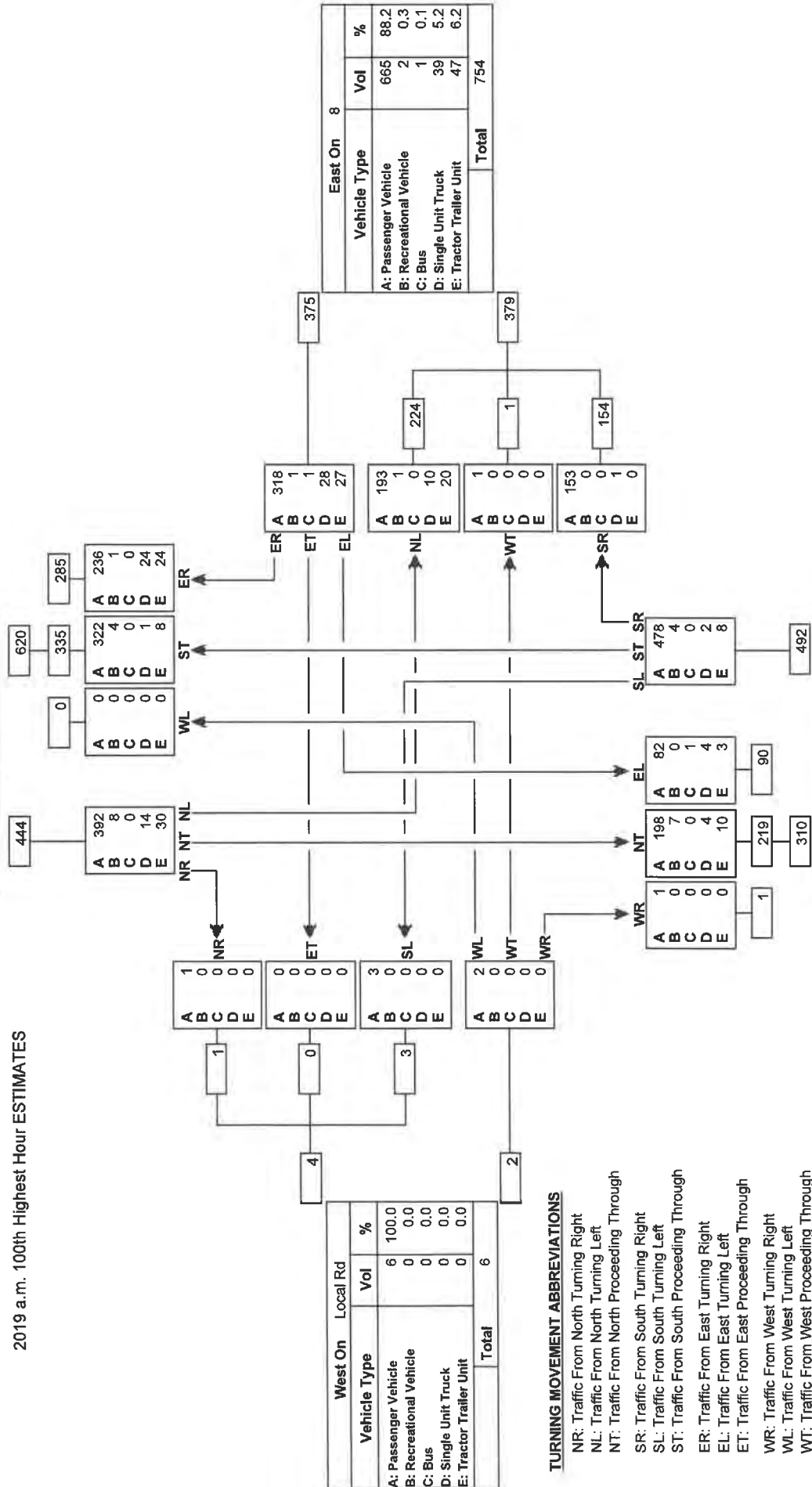
North On 22		
Vehicle Type	Vol	%
A: Passenger Vehicle	950	89.3
B: Recreational Vehicle	13	1.2
C: Bus	0	0.0
D: Single Unit Truck	39	3.7
E: Tractor Trailer Unit	62	5.8
Total	1064	

Reference No.: 60200

Intersection of:

8 & 22 NE OF BRAGG CREEK

2019 a.m. 100th Highest Hour ESTIMATES



Turning Movement Summary Diagram

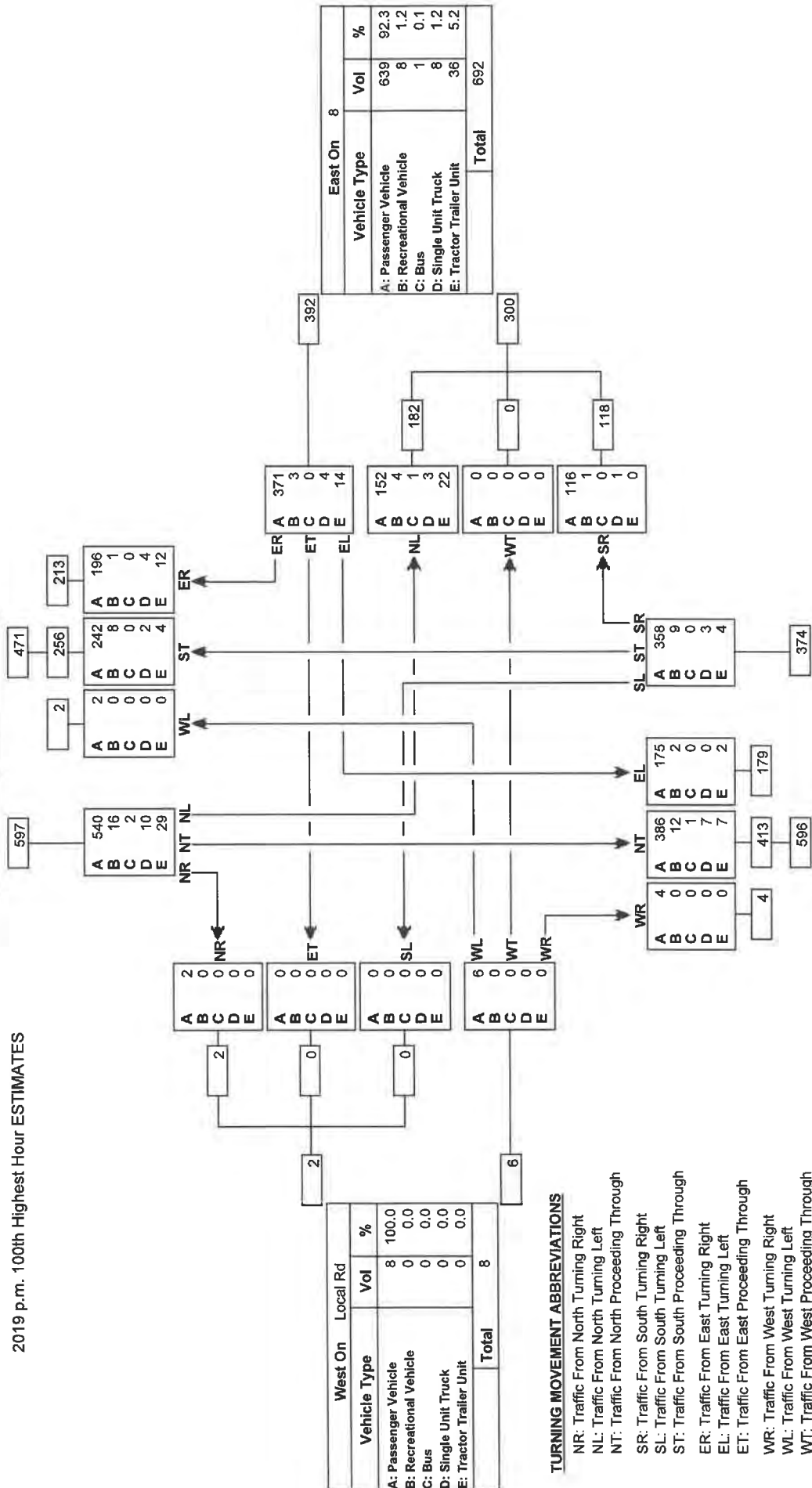
North On 22		
Vehicle Type	Vol	%
A: Passenger Vehicle	980	91.8
B: Recreational Vehicle	25	2.3
C: Bus	2	0.2
D: Single Unit Truck	16	1.5
E: Tractor Trailer Unit	45	4.2
Total		1068

2019 p.m. 100th Highest Hour ESTIMATES

Reference No.: 60200

Intersection of:

8 & 22 NE OF BRAGG CREEK



TURNING MOVEMENT ABBREVIATIONS

- NR: Traffic From North Turning Right
- NL: Traffic From North Turning Left
- NT: Traffic From North Proceeding Through
- SR: Traffic From South Turning Right
- SL: Traffic From South Turning Left
- ST: Traffic From South Proceeding Through
- ER: Traffic From East Turning Right
- EL: Traffic From East Turning Left
- ET: Traffic From East Proceeding Through
- WR: Traffic From West Turning Right
- WL: Traffic From West Turning Left
- WT: Traffic From West Proceeding Through

Copithorne Pit **2019 Truck Haul Summary**

Month of May		One-Way		Two-Way	
		Trips		Trips	
13/05/2019		11		22	
14/05/2019		4		8	
<i>2 days of haul in May</i>					
<i>27 permitted haul days in June</i>					
Month of June		One-Way		Two-Way	
		Trips		Trips	
04/06/2019		6		12	
05/06/2019		12		24	
06/06/2019		23		46	
07/06/2019		3		6	
10/06/2019		9		18	
12/06/2019		26		52	
13/06/2019		14		28	
17/06/2019		3		6	
18/06/2019		5		10	
20/06/2019		2		4	
25/06/2019		3		6	
27/06/2019		1		2	
29/06/2019		5		10	
<i>13 days of haul in June</i>					
<i>25 permitted haul days in June</i>					
Month of July		One-Way		Two-Way	
		Trips		Trips	
02/07/2019		2		4	
03/07/2019		4		8	
12/07/2019		5		10	
17/07/2019		8		16	
18/07/2019		10		20	
19/07/2019		16		32	
22/07/2019		7		14	
23/07/2019		11		22	
24/07/2019		31		62	
25/07/2019		21		42	
26/07/2019		22		44	
27/07/2019		10		20	
29/07/2019		14		28	
30/07/2019		25		50	
31/07/2019		24		48	
<i>15 days of haul in July</i>					
<i>26 permitted haul days in July</i>					

Copithorne Pit 2019 Truck Haul Summary

Month of August	One-Way Trips	Two-Way Trips
01/08/2019	28	56
02/08/2019	19	38
06/08/2019	4	8
07/08/2019	18	36
08/08/2019	16	32
09/08/2019	4	8
10/08/2019	6	12
13/08/2019	15	30
14/08/2019	15	30
15/08/2019	14	28
16/08/2019	4	8
19/08/2019	14	28
20/08/2019	25	50
21/08/2019	32	64
22/08/2019	14	28
23/08/2019	8	16
26/08/2019	16	32
27/08/2019	16	32
28/08/2019	21	42
29/08/2019	25	50
30/08/2019	8	16
21 days of haul in August		
26 permitted haul days in August		

Month of September	One-Way Trips	Two-Way Trips
03/09/2019	19	38
04/09/2019	25	50
05/09/2019	13	26
06/09/2019	19	38
09/09/2019	1	2
12/09/2019	6	12
13/09/2019	10	20
14/09/2019	3	6
16/09/2019	14	28
17/09/2019	20	40
18/09/2019	20	40
19/09/2019	5	10
20/09/2019	1	2
21/09/2019	2	4
23/09/2019	27	54
24/09/2019	17	34
25/09/2019	13	26
26/09/2019	8	16
27/09/2019	23	46
28/09/2019	25	50
20 days of haul in September		
24 permitted haul days in September		

Month of October	One-Way Trips	Two-Way Trips
01/10/2019	5	10
02/10/2019	15	30
03/10/2019	22	44
04/10/2019	30	60
05/10/2019	4	8
07/10/2019	12	24
09/10/2019	1	2
10/10/2019	20	40
11/10/2019	5	10
12/10/2019	5	10
15/10/2019	7	14
16/10/2019	2	4
17/10/2019	10	20
18/10/2019	2	4
19/10/2019	3	6
21/10/2019	8	16
22/10/2019	3	6
23/10/2019	3	6
28/10/2019	2	4
29/10/2019	5	10
30/10/2019	21	42
31/10/2019	38	76
22 days of haul in October		
27 permitted haul days in October		

Copithorne Pit 2019 Truck Haul Summary

2019 Haul Summary			
Avg. Trucks per Hour (Days Hauled)			
Month of November	One-Way		Two-Way
	Trips	Trips	
01/11/2019	22	44	90
02/11/2019	16	32	32
04/11/2019	3	6	36
05/11/2019	7	14	2
06/11/2019	7	14	62
07/11/2019	38	76	6
08/11/2019	5	10	24
12/11/2019	9	18	50
13/11/2019	16	32	14
14/11/2019	13	26	
15/11/2019	12	24	
18/11/2019	7	14	
21/11/2019	20	40	
22/11/2019	4	8	
25/11/2019	44	88	
26/11/2019	24	48	
27/11/2019	17	34	
28/11/2019	17	34	
29/11/2019	26	52	
30/11/2019	17	34	
20 days of haul in November			
26 permitted haul days in November			
Month of December			
Month of December	One-Way		Two-Way
	Trips	Trips	
02/12/2019	45	90	
03/12/2019	16	32	
04/12/2019	18	36	
05/12/2019	1	2	
07/12/2019	31	62	
09/08/2019	3	6	
10/12/2019	12	24	
11/12/2019	25	50	
12/12/2019	7	14	
9 days of haul in December			
26 permitted haul days in December			
Season Average		1.34	2.68
Number of Days Hauled in 2019 Season			
Permitted Haul Days in 2019			122
Permitted Haul Days in 2019			304










*** no pit activity from January to April, 2019.**

APPENDIX D

Synchro Output Summaries










1: Highway 22 & Township Road 242
08-26-2020

AM Peak Hour
2020 Background

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	5	5	625	447	9
Future Volume (Veh/h)	5	5	5	625	447	9
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	5	5	672	481	10
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1168	486	491			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1168	486	491			
tC, single (s)	6.4	6.8	4.5			
tC, 2 stage (s)						
tF (s)	3.5	3.8	2.6			
p0 queue free %	98	99	99			
cM capacity (veh/h)	210	480	902			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	10	677	491			
Volume Left	5	5	0			
Volume Right	5	0	10			
cSH	292	902	1700			
Volume to Capacity	0.03	0.01	0.29			
Queue Length 95th (m)	0.8	0.1	0.0			
Control Delay (s)	17.8	0.1	0.0			
Lane LOS	C	A				
Approach Delay (s)	17.8	0.1	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			47.9%	ICU Level of Service		A
Analysis Period (min)			15			










1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2020 Background

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	6	8	5	476	599	5
Future Volume (Veh/h)	6	8	5	476	599	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	6	9	5	512	644	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1168	646	649			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1168	646	649			
tC, single (s)	6.4	6.6	4.3			
tC, 2 stage (s)						
tF (s)	3.5	3.6	2.4			
p0 queue free %	97	98	99			
cM capacity (veh/h)	209	415	857			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	15	517	649			
Volume Left	6	5	0			
Volume Right	9	0	5			
cSH	298	857	1700			
Volume to Capacity	0.05	0.01	0.38			
Queue Length 95th (m)	1.3	0.1	0.0			
Control Delay (s)	17.7	0.2	0.0			
Lane LOS	C	A				
Approach Delay (s)	17.7	0.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization		42.7%		ICU Level of Service	A	
Analysis Period (min)		15				










1: Highway 22 & Township Road 242
08-26-2020

AM Peak Hour
2040 Background

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	5	5	810	579	11
Future Volume (Veh/h)	5	5	5	810	579	11
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	5	5	871	623	12
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1510	629	635			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1510	629	635			
tC, single (s)	6.4	6.8	4.5			
tC, 2 stage (s)						
tF (s)	3.5	3.8	2.6			
p0 queue free %	96	99	99			
cM capacity (veh/h)	130	392	790			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	10	876	635			
Volume Left	5	5	0			
Volume Right	5	0	12			
cSH	195	790	1700			
Volume to Capacity	0.05	0.01	0.37			
Queue Length 95th (m)	1.3	0.2	0.0			
Control Delay (s)	24.5	0.2	0.0			
Lane LOS	C	A				
Approach Delay (s)	24.5	0.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay		0.3				
Intersection Capacity Utilization		57.9%		ICU Level of Service		B
Analysis Period (min)		15				










1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2040 Background

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	7	9	5	617	776	5
Future Volume (Veh/h)	7	9	5	617	776	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	10	5	663	834	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1510	836	839			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1510	836	839			
tC, single (s)	6.4	6.5	4.3			
tC, 2 stage (s)						
tF (s)	3.5	3.6	2.4			
p0 queue free %	94	97	99			
cM capacity (veh/h)	130	324	723			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	18	668	839			
Volume Left	8	5	0			
Volume Right	10	0	5			
cSH	194	723	1700			
Volume to Capacity	0.09	0.01	0.49			
Queue Length 95th (m)	2.4	0.2	0.0			
Control Delay (s)	25.4	0.2	0.0			
Lane LOS	D	A				
Approach Delay (s)	25.4	0.2	0.0			
Approach LOS	D					
Intersection Summary						
Average Delay		0.4				
Intersection Capacity Utilization		52.3%		ICU Level of Service	A	
Analysis Period (min)		15				










1: Highway 22 & Township Road 242
08-26-2020

AM Peak Hour
2020 Background - Twinned Hwy 22

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	5	5	810	579	11
Future Volume (Veh/h)	5	5	5	810	579	11
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	5	5	871	623	12
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1074	318	635			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1074	318	635			
tC, single (s)	6.9	8.1	4.9			
tC, 2 stage (s)						
tF (s)	3.5	3.9	2.6			
p0 queue free %	98	99	99			
cM capacity (veh/h)	208	534	727			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	10	295	581	415	220	
Volume Left	5	5	0	0	0	
Volume Right	5	0	0	0	12	
cSH	300	727	1700	1700	1700	
Volume to Capacity	0.03	0.01	0.34	0.24	0.13	
Queue Length 95th (m)	0.8	0.2	0.0	0.0	0.0	
Control Delay (s)	17.4	0.3	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	17.4	0.1		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			36.6%	ICU Level of Service		A
Analysis Period (min)			15			










1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2040 Background - Twinned Hwy 22

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	7	9	5	617	776	5
Future Volume (Veh/h)	7	9	5	617	776	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	10	5	663	834	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1178	420	839			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1178	420	839			
tC, single (s)	6.9	7.6	4.5			
tC, 2 stage (s)						
tF (s)	3.5	3.6	2.4			
p0 queue free %	96	98	99			
cM capacity (veh/h)	178	504	686			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	18	226	442	556	283	
Volume Left	8	5	0	0	0	
Volume Right	10	0	0	0	5	
cSH	278	686	1700	1700	1700	
Volume to Capacity	0.06	0.01	0.26	0.33	0.17	
Queue Length 95th (m)	1.7	0.2	0.0	0.0	0.0	
Control Delay (s)	18.9	0.3	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	18.9	0.1		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			32.2%	ICU Level of Service		A
Analysis Period (min)			15			










1: Highway 22 & Township Road 242
08-26-2020

AM Peak Hour
2020 Post Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	8	7	626	447	13
Future Volume (Veh/h)	5	8	7	626	447	13
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	9	8	673	481	14
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1177	488	495			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1177	488	495			
tC, single (s)	6.4	7.0	4.6			
tC, 2 stage (s)						
tF (s)	3.5	4.1	2.7			
p0 queue free %	98	98	99			
cM capacity (veh/h)	206	443	861			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	14	681	495			
Volume Left	5	8	0			
Volume Right	9	0	14			
cSH	314	861	1700			
Volume to Capacity	0.04	0.01	0.29			
Queue Length 95th (m)	1.1	0.2	0.0			
Control Delay (s)	17.0	0.2	0.0			
Lane LOS	C	A				
Approach Delay (s)	17.0	0.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay		0.3				
Intersection Capacity Utilization		49.6%		ICU Level of Service		A
Analysis Period (min)		15				

1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2020 Post Development










						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	7	14	5	477	599	4
Future Volume (Veh/h)	7	14	5	477	599	4
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	15	5	513	644	4
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1169	646	648			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1169	646	648			
tC, single (s)	6.4	6.7	4.5			
tC, 2 stage (s)						
tF (s)	3.5	3.8	2.6			
p0 queue free %	96	96	99			
cM capacity (veh/h)	209	396	781			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	23	518	648			
Volume Left	8	5	0			
Volume Right	15	0	4			
cSH	302	781	1700			
Volume to Capacity	0.08	0.01	0.38			
Queue Length 95th (m)	2.0	0.2	0.0			
Control Delay (s)	17.9	0.2	0.0			
Lane LOS	C	A				
Approach Delay (s)	17.9	0.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization		42.6%		ICU Level of Service	A	
Analysis Period (min)		15				

1: Highway 22 & Township Road 242

AM Peak Hour










08-26-2020

2040 Post Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	9	7	811	579	15
Future Volume (Veh/h)	5	9	7	811	579	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	10	8	872	623	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1519	631	639			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1519	631	639			
tC, single (s)	6.4	7.0	4.6			
tC, 2 stage (s)						
tF (s)	3.5	4.0	2.7			
p0 queue free %	96	97	99			
cM capacity (veh/h)	127	372	753			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	15	880	639			
Volume Left	5	8	0			
Volume Right	10	0	16			
cSH	227	753	1700			
Volume to Capacity	0.07	0.01	0.38			
Queue Length 95th (m)	1.7	0.3	0.0			
Control Delay (s)	22.0	0.3	0.0			
Lane LOS	C	A				
Approach Delay (s)	22.0	0.3	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay		0.4				
Intersection Capacity Utilization		59.6%		ICU Level of Service		B
Analysis Period (min)		15				










1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2040 Post Development

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	8	15	6	618	776	5
Future Volume (Veh/h)	8	15	6	618	776	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	9	16	6	665	834	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1514	836	839			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1514	836	839			
tC, single (s)	6.4	6.7	4.5			
tC, 2 stage (s)						
tF (s)	3.5	3.7	2.6			
p0 queue free %	93	95	99			
cM capacity (veh/h)	129	308	654			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	25	671	839			
Volume Left	9	6	0			
Volume Right	16	0	5			
cSH	205	654	1700			
Volume to Capacity	0.12	0.01	0.49			
Queue Length 95th (m)	3.3	0.2	0.0			
Control Delay (s)	25.0	0.3	0.0			
Lane LOS	C	A				
Approach Delay (s)	25.0	0.3	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay		0.5				
Intersection Capacity Utilization		52.3%		ICU Level of Service		A
Analysis Period (min)		15				










1: Highway 22 & Township Road 242
08-26-2020

AM Peak Hour
2040 Post Development - Twinned Hwy 22

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	9	7	811	579	15
Future Volume (Veh/h)	5	9	7	811	579	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	5	10	8	872	623	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1083	320	639			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1083	320	639			
tC, single (s)	6.9	8.4	5.1			
tC, 2 stage (s)						
tF (s)	3.5	4.0	2.7			
p0 queue free %	98	98	99			
cM capacity (veh/h)	205	502	679			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	15	299	581	415	224	
Volume Left	5	8	0	0	0	
Volume Right	10	0	0	0	16	
cSH	338	679	1700	1700	1700	
Volume to Capacity	0.04	0.01	0.34	0.24	0.13	
Queue Length 95th (m)	1.1	0.3	0.0	0.0	0.0	
Control Delay (s)	16.1	0.4	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	16.1	0.1		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			38.1%	ICU Level of Service		A
Analysis Period (min)			15			

1: Highway 22 & Township Road 242
08-26-2020

PM Peak Hour
2040 Post Development - Twinned Hwy 22

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	8	15	6	618	776	5
Future Volume (Veh/h)	8	15	6	618	776	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	9	16	6	665	834	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1181	420	839			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1181	420	839			
tC, single (s)	6.9	7.8	4.9			
tC, 2 stage (s)						
tF (s)	3.5	3.8	2.6			
p0 queue free %	95	97	99			
cM capacity (veh/h)	177	475	589			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	25	228	443	556	283	
Volume Left	9	6	0	0	0	
Volume Right	16	0	0	0	5	
cSH	296	589	1700	1700	1700	
Volume to Capacity	0.08	0.01	0.26	0.33	0.17	
Queue Length 95th (m)	2.2	0.2	0.0	0.0	0.0	
Control Delay (s)	18.3	0.4	0.0	0.0	0.0	
Lane LOS	C	A				
Approach Delay (s)	18.3	0.1		0.0		
Approach LOS	C					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			32.2%	ICU Level of Service		A
Analysis Period (min)			15			

APPENDIX E

Illumination Warrants

Illumination of Isolated Rural Intersections

LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with *Illumination of Isolated Rural Intersections*, Transportation Association of Canada, February 2001.

Please enter information in the cells with yellow background

INTERSECTION CHARACTERISTICS

Highway 22	Main Road
Township Road 242	Minor Road
Rocky View County	City/Town

Date June 2, 2020
Other Horizon: Long Term Background

GEOMETRIC FACTORS

	Value	Rating	Weight	Comments	Check	Score
Channelization Rating	Descriptive	0		Refer to Table 1(A) to determine rating value	OK	
Presence of raised channelization? (Y/N)	n				OK	
Highest operating speed on raised, channelized approach (km/h)	0		5		OK	
Channelization Factor					OK	0
Approach Sight Distance on most constrained approach (%)	100	0	10	Relative to the recommended minimum sight distance	OK	0
Posted Speed limit (in 10's of km/h)	100				OK	
Radius of Horizontal Curve (m)	T			Enter "T" for tangent (no horizontal curve at the intersection)	OK	
Posted Speed Category =		0				
Posted Speed Category =	B	0				
Posted Speed Category =		0				
Posted Speed Category =		0				
Horizontal Curvature Factor		0	5		OK	0
Angle of Intersection (10's of Degrees)	90	0	5		OK	0
Downhill Approach Grade (x.x%)	0.0	0	3	Rounded to nearest tenth of a percent	OK	0
Number of Intersection Legs	4	2	3	Number of legs = 3 or more	OK	6
Geometric Factors Subtotal						6

OPERATIONAL FACTORS

Is the intersection signalized? (Y/N)	N			Calculate the Signalization Warrant Factor		
AADT on Major Road (2-way)	12,200	4	10		OK	40
AADT on Minor Road (2-way)	115	0	20		OK	0
Signalization Warrant	Descriptive	0	30	Either Use the two AADT inputs OR the Descriptive Signalization Warrant (Unused values should be set to Zero) Refer to Table 1(B) for description and rating values for signalization warrant.	OK	0
Night-Time Hourly Pedestrian Volume	0	0	10	Refer to Table 1(B), note #2, to account for children and seniors	OK	0
Intersecting Roadway Classification	Descriptive	1	5	Refer to Table 1(B) for ratings.	OK	5
Operating Speed or Posted Speed on Major Road (km/h)	110	4	5	Refer to Table 1(B), note #3	OK	20
Operating Speed on Minor Road (km/h)	60	1	5	Refer to Table 1(B), note #3	OK	5
Operational Factors Subtotal						70

ENVIRONMENTAL FACTOR

Lighted Developments within 150 m radius of intersection	0	0	5	Maximum of 4 quadrants	OK	0
Environmental Factor Subtotal						0

COLLISION HISTORY

Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole #)	0.0	0	0	Enter either the annual frequency (See Table 1(C), note #4)	OK	0
OR				OR the number of collisions / MEV		
Collision Rate over last 3 years, due to inadequate lighting (/MEV)	0	0	0	(Unused values should be set to Zero)	OK	0
Is the average ratio of all night to day collisions >= 1.5 (Y/N)	n	0			OK	
Collision History Subtotal						0

Check Intersection Signalization:
Intersection is not Signalized

LIGHTING IS NOT WARRANTED

SUMMARY

Geometric Factors Subtotal	6
Operational Factor Subtotal	70
Environmental Factor Subtotal	0
Collision History Subtotal	0

TOTAL POINTS 76

Illumination of Isolated Rural Intersections

LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with *Illumination of Isolated Rural Intersections*, Transportation Association of Canada, February 2001.

Please enter information in the cells with yellow background

INTERSECTION CHARACTERISTICS

Highway 22	Main Road
Township Road 242	Minor Road
Rocky View County	City/Town

Date: June 2, 2020
Other: Horizon: Long Term Post Development

GEOMETRIC FACTORS

	Value	Rating	Weight	Comments	Check	Score
Channelization Rating	Descriptive	0		Refer to Table 1(A) to determine rating value	OK	
Presence of raised channelization? (Y/N)	n				OK	
Highest operating speed on raised, channelized approach (km/h)	0		5		OK	
Channelization Factor					OK	0
Approach Sight Distance on most constrained approach (%)	100	0	10	Relative to the recommended minimum sight distance	OK	0
Posted Speed limit (in 10's of km/h)	100				OK	
Radius of Horizontal Curve (m)	T			Enter "T" for tangent (no horizontal curve at the intersection)	OK	
Posted Speed Category =		0				
Posted Speed Category =	B	0				
Posted Speed Category =		0				
Posted Speed Category =		0				
Horizontal Curvature Factor		0	5		OK	0
Angle of Intersection (10's of Degrees)	90	0	5		OK	0
Downhill Approach Grade (x.x%)	0.0	0	3	Rounded to nearest tenth of a percent	OK	0
Number of Intersection Legs	4	2	3	Number of legs = 3 or more	OK	6
Geometric Factors Subtotal						6

OPERATIONAL FACTORS

Is the intersection signalized? (Y/N)	N			Calculate the Signalization Warrant Factor		
AADT on Major Road (2-way)	12,300	4	10		OK	40
AADT on Minor Road (2-way)	210	0	20		OK	0
Signalization Warrant	Descriptive	0	30	Either Use the two AADT inputs OR the Descriptive Signalization Warrant (Unused values should be set to Zero) Refer to Table 1(B) for description and rating values for signalization warrant.	OK	0
Night-Time Hourly Pedestrian Volume	0	0	10	Refer to Table 1(B), note #2, to account for children and seniors	OK	0
Intersecting Roadway Classification	Descriptive	1	5	Refer to Table 1(B) for ratings.	OK	5
Operating Speed or Posted Speed on Major Road (km/h)	110	4	5	Refer to Table 1(B), note #3	OK	20
Operating Speed on Minor Road (km/h)	50	1	5	Refer to Table 1(B), note #3	OK	5
Operational Factors Subtotal						70

ENVIRONMENTAL FACTOR

Lighted Developments within 150 m radius of intersection	0	0	5	Maximum of 4 quadrants	OK	0
Environmental Factor Subtotal						0

COLLISION HISTORY

Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole #)	0.0	0	0	Enter either the annual frequency (See Table 1(C), note #4)	OK	0
OR				OR the number of collisions / MEV		
Collision Rate over last 3 years, due to inadequate lighting (/MEV)	0	0	0	(Unused values should be set to Zero)	OK	0
Is the average ratio of all night to day collisions >= 1.5 (Y/N)	n	0			OK	
Collision History Subtotal						0

Check Intersection Signalization:
Intersection is not Signalized

LIGHTING IS NOT WARRANTED

SUMMARY

Geometric Factors Subtotal	6
Operational Factor Subtotal	70
Environmental Factor Subtotal	0
Collision History Subtotal	0

TOTAL POINTS **76**