

Copithorne Gravel Pit

Transportation Impact Assessment Update

Final report

Prepared for

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Date

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Project Number

02-20-055

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1. EXECUTIVE SUMMARY

An update to the 2016 dated Copithorne Gravel Pit TIA has been requested by the business owners in light of new available traffic data. This updated TIA is provided for use by the owners of the gravel pit site located at SW-18-24-04-W5M. The site is located 5 kilometres west of Highway 22 on Township Road 242.

This updated Transportation Impact Assessment (TIA) is submitted in accordance with County Servicing Standards. Bunt & Associates Engineering (AB) Ltd. was retained to undertake this study. The key findings and corresponding recommendations are summarized below.

1.1 Proposed Development

The site will operate with the following conditions:

- Hauling will occur from 7:00-17:00 on Monday-Saturday. Generally hauling will occur from May 1 to October 30; however, dependant on weather, hauling may also fall outside this period.
- A total of 2 full time employees will work on-site, there are no extra employees when crushing is occurring.
- All traffic will access the site via the parcel to the east, which is also owned by Mr. Copithorne. Traffic
 will travel to Highway 22 via Township Road 242. This is the current route used by previous
 operations on the site.
- All outbound truck traffic will turn right at Highway 22 and proceed to the south on Highway 22.
 Trucks will enter the site from both the north and south on Highway 22.

Expected site trip generation is summarized in Table 1.1.

Table 1.1: Development Trip Generation

| LAND USE | AM PEAK HOUR | | | PM PEA | PM PEAK HOUR | | | DAILY | | |
|-------------------------|--------------|----|-----|--------|--------------|-----|-------|-------|-----|--|
| | Total | In | Out | Total | In | Out | Total | In | Out | |
| Passenger Vehicle Trips | 2 | 2 | 0 | 2 | 0 | 2 | 4 | 2 | 2 | |
| Truck Trips | 9 | 5 | 4 | 9 | 4 | 5 | 90 | 45 | 45 | |
| Total Trips | 11 | 7 | 4 | 11 | 4 | 7 | 94 | 47 | 47 | |

1.2 Highway 22 & Township Road 242 Operations

Highway 22 & Township Road 242 has a type I intersection treatment. A review of existing, background, and post development conditions at the intersection indicated the following:

1.2.1 Collision History

A total of 5 collisions occurred at the intersection over a 14-year period. Three of the collisions were single-vehicle (struck animal; run-off-road). A collision occurred in 2005 that involved 3 vehicles with 4 injuries and 2 fatalities. A 2015 rear-end collision involving 2 trucks is also documented.

1.2.2 Sight Distance

All minimum and desirable sight distance requirements are met at the intersection.

1.2.3 Background Intersection Capacity Analysis

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the Opening Day (2020) and Long Term (2040) horizons.

1.2.4 Post Development Intersection Capacity Analysis

The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.

1.2.5 Alberta Transportation Intersection Warrant

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

1.2.6 Alberta Transportation Illumination Warrant

Street light illumination is not warranted at the intersection.

1.3 Roadway Link Capacity Analysis

The daily traffic volumes on Highway 22 and Township Road 242 were reviewed to determine the impact of background and post development traffic volumes.

1.3.1 Highway 22

The Background road link analysis suggests twinning of Highway 22 may be required by the Long Term Background horizon (2040). This matches with the recommendations from the *Highway 22 Functional Planning Study*. No appreciable change will occur due to development traffic volumes.

1.3.2 Township Road 242

The roadway is expected to continue operating within its daily vehicle capacity. Due to the expected truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.

2. INTRODUCTION

2.1 Scope of Work

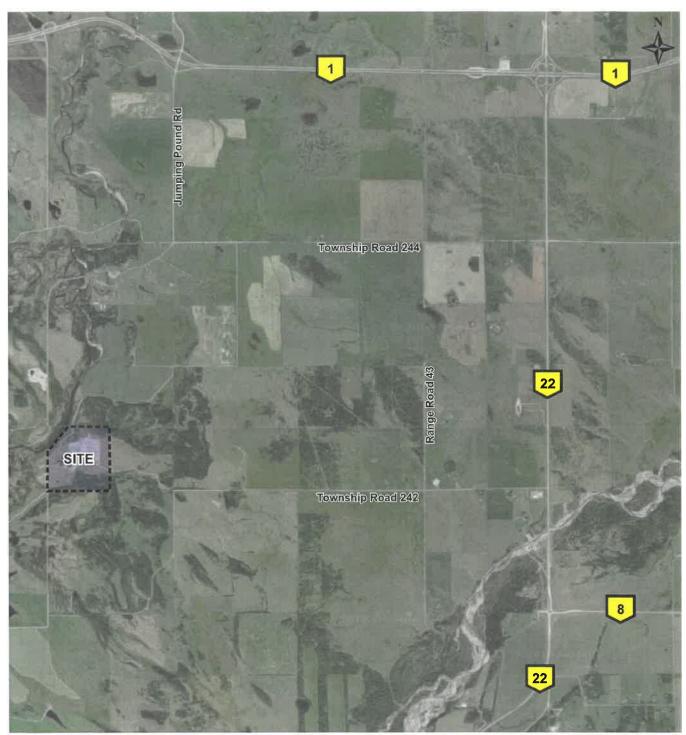
Based on discussions with Rocky View County and Alberta Transportation, the scope of work for the original TIA was confirmed as follows, noting that the analysis horizons have been adjusted for this TIA update:

- Determine expected site generated traffic volumes for the proposed uses based on first principles and in-house data.
- Assign expected site generated traffic volumes to the road network based on anticipated site traffic distribution.
- Complete capacity analysis for Opening Day (2020), and Long Term (2040) horizons during the AM and PM Peak hour at the following intersections:
 - o Highway 22 & Township Road 242.
- Complete sight distance analysis.
- Review collision data at the intersection of Highway 22 & Township Road 242.
- Complete intersection type and illumination warrants for Highway 22 & Township Road 242.
- Identify transportation improvements required to support the development.

The study scope correspondence is included in Appendix A.

2.2 Site Context

The subject lands are zoned as NRI (Natural Resource Industrial District). Vehicular access to the development is provided from Township Road 242 via Highway 22. The site context and adjacent external road network is illustrated in **Exhibit 2.1**.



Base Map Source: Google Maps

Exhibit 2.1 Site Context

Copithorne Gravel Pit TIA Update June 2020 Scale NTS



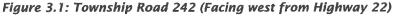
3. BACKGROUND TRAFFIC CONDITIONS

To assess the impacts of the proposed development on existing infrastructure, Bunt & Associates established the impacts of background traffic without consideration of site traffic. This provided the basis for comparing the incremental impact due to the proposed development.

3.1 Existing Road Network

The roadways in the vicinity of the site are described below:

- **Highway 22** is a two-lane undivided provincial Highway (control section #14) that runs in the north-south direction with a posted speed limit of 100 km/h near Township Road 242. The roadway's current cross-section near Township Road 242 has one driving lane per direction with shoulders on both sides of the roadway. Illumination is not provided on the roadway except near major intersection approaches (Highway 8 roundabout intersection; Highway 1 interchange).
- Township Road 242 is a two-lane unpaved County roadway that runs the east-west direction from the site to Highway 22. The roadway has a right-of-way (ROW) of 20 metres and a posted a speed limit of 60 km/h. Non-regulatory signage is provided stating "film crew reduce speed 50 km/h." No illumination is provided on the roadway. The roadway is illustrated in Figure 3.1 and Figure 3.2.





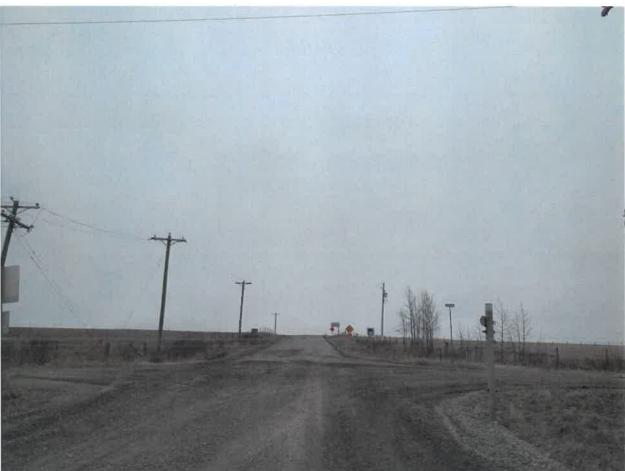


Figure 3.2: Township Road 242 at Range Road 43 (Facing East)

3.2 Existing Lane Configurations & Traffic Control

The following lane configurations and traffic control are in place at study area intersections:

• Highway 22 & Township Road 242 - The intersection is unsignalized with stop control in the eastbound direction. Type I intersection treatment is provided.

The following other major intersections are located along Highway 22:

- Highway 22 & Highway 1 A cloverleaf interchange is provided. (Parclo interchange improvement is underway)
- Highway 22 & Highway 8 A single lane roundabout controls the intersection.

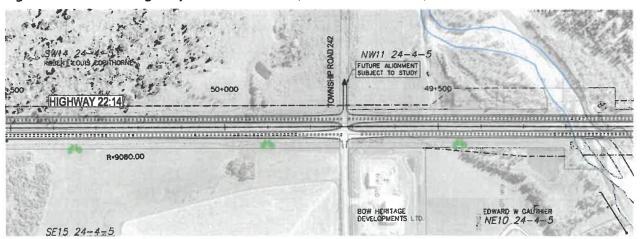
3.3 Highway 22 Functional Planning Study

ISL Engineering completed a *Highway 22 Functional Planning Study*¹ for Alberta Transportation. The functional planning study recommended the following:

- At the interim stage (2030) Highway 22 be twinned to a four-lane divided arterial and be connected to Highway 1 via a newly upgraded Parclo A interchange. The design contains auxiliary turn lanes at intersections including Township Road 242.
- At the ultimate stage (2050) Highway 22 be upgraded to a six-lane cross-section with auxiliary turn lanes at intersections.

The ultimate Highway 22 cross-section near Township Road 242 is illustrated in Figure 3.3.

Figure 3.3: Ultimate Highway 22 Cross-Section (Oriented to the East)



The traffic projections provided in the March 2014 report are summarized in **Table 3.1** and compared to Alberta Transportation AADT (Average annual daily traffic) data.

Table 3.1: Functional Study - Daily Traffic Projections (vehicles per day)

| HIGHWAY | FUNCTI | ONAL STUD | AT AADT DATA | | | | |
|--------------------------------|--------|-----------|----------------------|----------------------|--------|--------|--------|
| | 2014 | 2030 | 2050 (Scenario 1) | 2050 (Scenario 2) | 2014 | 2015 | 2019 |
| Highway 22 (South of Hwy 1) | 9,700 | 15,200 | 20,000 | 22,000 | 10,340 | 12,140 | 10,140 |

Highway 22 Functional Planning Study south of Highway 8 to Town of Cochrane Corporate Limits, ISL Engineering and Land Services, March 2014.

http://www.transportation.alberta.ca/projects/assets/Area_7_Calgary_Area/Hwy_22_twinning_from_Hwy_8_to_Cochrane/Executive%20Summary.pdf

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Alberta Transportation AADT data indicates traffic volumes on Highway 22 had a spike in growth in 2015 and since then have been decreasing annually, resulting in an overall growth of approximately 1.5% per year when compared to the "current volumes" stated in the March 2014 functional study. If this rate of growth is to continue, twinning would be required at a later stage than the 2030 horizon provided in the functional study.

3.4 Springbank Off-stream Reservoir Project

The Springbank Off-stream Reservoir Project (SR1) is in development and will result in changes to the existing road network along Highway 22, Springbank Road, and Township Road 242. The project will result in the following new infrastructure:

- Raise portion of Highway 22 north and south of Springbank Road and shift west to accommodate future twinning.
- Raise portion of Springbank Road east and west of Highway 22.
- New bridge to be constructed along Highway 22, north of Township Road 242.
- New bridge to be constructed along Township Road 242, west of Highway 22.

The above changes to the road network will not impact the analysis completed in this study, nor will impact the intersection of Highway 22 & Township Road 242 in terms of geometry and operation. The proposed SR1 upgrades are presented graphically in **Figure 3.4**.

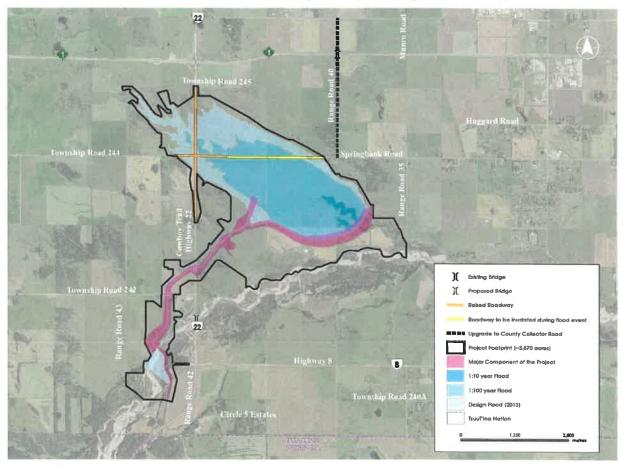


Figure 3.4: Springbank Off-stream Reservoir Project Proposed Upgrades

3.5 Collision Data

Safety performance at Highway 22 & Township Road 242 was reviewed based on data obtained from Alberta Transportation for the latest 10-Year period available at the time of the original TIA (2003 to 2012). This data is now supplemented with new available data from years up to and including 2016. There were a total of 5 collisions reported at Highway 22 & Township Road 242 between 2003 – 2016 as summarized in **Table 3.2.** The raw collision data is included in **Appendix B**.

Table 3.2: Highway 22 & Township Road 242 Collisions

| YEAR | # OF COLLISIONS | TYPE OF COLLISION REPORTED | |
|------|-----------------|----------------------------|--|
| 2004 | 1 | Struck Object - Animal | |
| 2005 | 1 | Head-on (2 fatalities) | |
| 2009 | 1 | Run-off-road | |
| 2013 | 1 | Struck Object - Animal | |
| 2015 | 1 | Rear-end (non injury) | |

The 2005 collision involved 3 vehicles with 4 injuries and 2 fatalities. The collision report states "V3 slowed for left turning vehicle in front; V1 struck rear of V3; V3 went into ditch; V1 crossed centre line and struck head-on into opposing V2." The 2004, 2009, and 2013 collisions were single-vehicle collisions. The 2015 collision was a rear-end collision of a right turning truck from Highway 22.

3.6 Sight Distance Requirements

A sight distance review was undertaken at Highway 22 & Township Road 242 based on the *Alberta Transportation Highway Geometric Guide* and *TAC Geometric Design Guide for Canadian Roads* to confirm the safety of turning movements and through movements on Highway 22. Sight distance requirements are based on the following:

Minimum Stopping Sight Distance (SSD), which is the distance a vehicle travels from the instant the driver sights an object and decides to stop, to the instant the vehicle comes to a complete stop after applying breaks. This distance is usually sufficient to allow reasonably competent and alert drivers to come to a hurried stop under ordinary conditions. Minimum stopping sight distances based on speeds are:

- 100 km/h = 200 metres
- 110 km/h = 235 metres

Desirable Decision Sight Distance (DSD), which is utilized in complex situations and is the distance required for a driver to detect an information source or hazard that is difficult to perceive in a roadway environment that might be visually cluttered, recognize the hazard or its threat potential, selection an appropriate action, and complete the manoeuvre safely and efficiently. A range of distances is provided with lower ranges appropriate for less complex situations and the higher range appropriate for more complex situations. Decisions sight distances based on speeds are:

- 100 km/h = 300 to 390 metres
- 110 km/h = 330 to 430 metres

Intersection Sight Distance (ISD), which is defined as the sight distance required for a vehicle to complete either a crossing or turning manoeuvre safely. Intersection sight distances based on speeds and vehicle types are:

- 100 km/h = 210 metres for passenger vehicle and 270 metres for a single-unit truck design vehicle
- 110 km/h = 230 metres for passenger vehicle and 290 metres for a single-unit truck design vehicle

Assuming a design speed of 110 km/h (posted speed limit of 100 km/h) along Highway 22, the sight distance requirements at the study area intersections are outlined in **Table 3.3**.

Table 3.3: Intersection Sight Distance

| INTERSECTION | DESIGN SSD SPEED | | DSD | ISD | | AVAILABLE SIGHT DIST. | |
|---------------------|---------------------|------|----------|------|----------|-----------------------|---------|
| | | | | Car | SU Truck | To West | To East |
| Hwy 22 & TWP Rd 242 | 110km/h | 235m | 330-430m | 230m | 290m | >500m | >500m |

The review confirms all minimum and desirable sight distance requirements are met at Highway 22 & Township Road 242.

3.7 Traffic Volumes

3.7.1 Existing Data

Bunt & Associates conducted intersection turning movement counts on Wednesday February 17, 2016 morning (7:00-9:00), mid-day (11:00-13:00), and afternoon (16:00-18:00) periods to document existing traffic movements at the intersection of Highway 22 & Township Road 242. This data is used in this study only to estimate the existing turning movement volumes in and out of Township Road 242 to/from Highway 22. The observed 2016 through volumes at this intersection were disregarded and replaced by 2019 Alberta Transportation 100th Highest Hour counts of Highway 22 & Highway 8. Traffic count data is provided in **Appendix C**.

3.7.2 Background Growth

Background growth is the result of traffic that would be present on the road network in future years regardless of the development of the site. This traffic is representative of yearly growth on the roadways as well as other residential, commercial, or industrial developments that have been approved in the area.

Alberta Transportation AADT data for Highway 22 (north of Highway 8) and Highway 8 (east of Highway 22) was obtained as summarized in **Table 3.4**.

Table 3.4: Highway 22 AADT Data

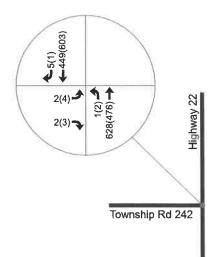
| YEAR | HIGHWAY 22 AADT | HIGHWAY 8 AADT |
|--------------------------------|----------------------|------------------|
| | (NORTH OF HIGHWAY 8) | (EAST OF HWY 22) |
| 2002 | - | 6,872 |
| 2003 | - | 6,942 |
| 2004 | | 6,860 |
| 2005 | - | 6,656 |
| 2006 | | 6,539 |
| 2007 | 9,350 | 6,510 |
| 2008 | 9,070 | 6,313 |
| 2009 | 9,110 | 6,385 |
| 2010 | 9,440 | 6,627 |
| 2011 | 9,300 | 6,560 |
| 2012 | 9,300 | 6,459 |
| 2013 | 9,480 | 6,396 |
| 2014 | 9,520 | 6,525 |
| 2015 | 11,320 | 7,131 |
| 2016 | 10,900 | 7,450 |
| 2017 | 10,820 | 7,230 |
| 2018 | 9,930 | 5,980 |
| 2019 | 9,290 | 4,960 |
| 5-Year Growth Rate (2014-2019) | -0.5% per year | -4.7% per year |
| All Data Growth Rate to 2019 | -0.1% per year | -1.6% per year |
| 5-Year Growth Rate (2012-2017) | 3.3% per year | 2.4% per year |
| All Data Growth Rate to 2017 | 1.6% per year | 0.3% per year |

As shown in the above table, the most recent 5-year growth rate (2014-2019) exemplifies that traffic is currently decreasing annually along Highway 22 and Highway 8. Traffic volumes along Highway 22 are expected to experience stifled growth in the coming years due to the inclusion of the Calgary Southwest Ring Road project that will pull existing traffic from Highway 8 and Highway 22 instead to Highway 1 exclusively. With this noted, some growth is still expected to occur along Highway 22 over the coming years; therefore a growth rate of 1.5% per year was developed by averaging all data up to 2017, before a significant drop in traffic volumes was observed.

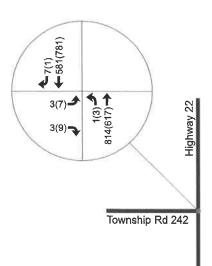
This linear growth rate is applied to all movements at the intersection of Highway 22 & Township Road 242 to develop 2020 and 2040 Background volumes. The resulting Background traffic volumes are summarized in **Exhibit 3.1**.



2020 Background Traffic Volumes



2040 Background Traffic Volumes



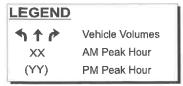


Exhibit 3.1

Background Traffic Volumes

Copithorne Gravel Pit TIA Update June 2020 Scale NTS



3.8 Intersection Capacity Analysis

Intersection capacity analysis was undertaken for the study area intersections using Synchro 8.0, a traffic analysis software package based on the methods outlined in the Highway Capacity Manual (HCM) 2000. This model uses standard procedures to determine the Volume to Capacity (v/c) and the corresponding delay-based traffic Level of Service (LOS) for movements at each intersection in the study network.

Alberta Transportation's guidelines for unsignalized intersections accept a v/c ratio of 0.85 or less and a LOS C at highway access intersections with a LOS D on any single approach at full build-out depending on location.

For unsignalized intersections, the LOS methodology considers intersection geometry, traffic volumes, speed limit, and type of intersection control. For signalized intersections, the LOS methodology considers intersection geometry, traffic volumes, speed limit, and signal timing plan. Delays range from LOS 'A' conditions with minimal delay to LOS 'F' which represents significant control delay. The LOS criteria for unsignalized and signalized intersections are summarized in **Table 3.5**.

Table 3.5: HCM Level of Service Summary

| LEVEL OF SERVICE | AVERAGE CONTROL DELAY FOR | AVERAGE CONTROL DELAY FOR |
|------------------|-------------------------------|-----------------------------------|
| (LOS) | UNSIGNALIZED INTERSECTION | SIGNALIZED INTERSECTION MOVEMENTS |
| | MOVEMENTS | |
| A | ≤ 10 seconds per vehicle | ≤ 10 seconds per vehicle |
| В | > 10 - 15 seconds per vehicle | > 10 – 20 seconds per vehicle |
| С | > 15 - 25 seconds per vehicle | > 20 - 35 seconds per vehicle |
| D | > 25 - 35 seconds per vehicle | > 35 - 55 seconds per vehicle |
| E | > 35 - 50 seconds per vehicle | > 55 - 80 seconds per vehicle |
| F | > 50 seconds per vehicle | > 80 seconds per vehicle |

The results of the intersection capacity analysis were based on expected traffic volumes, traffic control, and lane arrangement at the study intersections. The analysis is completed as per Alberta Transportation TIA guidelines with a saturation flow of 1850 vehicles per hour and peak hour factor of 0.93.

The volume to capacity ratio, level of service, average control delay (in seconds), and 95th percentile queues (in metres) are summarized in the body of this report. Synchro output summaries are provided in **Appendix D**.

The Background intersection capacity analysis is summarized in **Table 3.6**. The 2020 analysis is completed based on existing lane configurations. The 2040 analysis is completed based on both the existing lane configuration and with a twinned Highway 22 as per the recommendations from the *Highway 22 Functional Planning Study*.

Table 3.6: Background Intersection Capacity Analysis (Highway 22 & Township Road 242)

| INTERSECTION | MOVEN | IENT | AM PEA | K HOU | R | | PM PEAK HOUR | | | |
|-------------------|---------|------|--------|-------|-------|-------|--------------|-----|-------|-------|
| | & LANE | S | v/c | LOS | Delay | Queue | v/c | LOS | Delay | Queue |
| 2020 Background | EB | 1 | 0.03 | С | 18 | <5 | 0.05 | С | 18 | <5 |
| - | NB | 1 | 0.01 | Α | 0 | <5 | 0.01 | Α | 0 | <5 |
| | SB | 1 | 0.29 | Α | 0 | <5 | 0.38 | Α | 0 | <5 |
| | Overall | | 2 | Α | 0.2 | - | - | Α | 0.3 | |
| 2040 Background | EB | 1 | 0.05 | С | 25 | <5 | 0.09 | D | 25 | <5 |
| (Existing Hwy 22) | NB | 1 | 0.01 | Α | 0 | <5 | 0.01 | Α | 0 | <5 |
| | SB | 1 | 0.37 | Α | 0 | <5 | 0.49 | Α | 0 | <5 |
| | Overall | | - | Α | 0.3 | - | - | Α | 0.4 | - |
| 2040 Background | EB | 1 | 0.03 | C | 17 | <5 | 0.06 | C | 19 | <5 |
| (Twinned Hwy 22) | NB | 2 | 0.34 | Α | 0 | <5 | 0.26 | Α | 0 | <5 |
| | SB | 2 | 0.24 | Α | 0 | <5 | 0.33 | Α | 0 | <5 |
| | Overall | | - | Α | 0.2 | - | - | Α | 0.3 | - |

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the 2020 and 2040 horizons in both the existing geometry and twinned highway scenarios.

3.9 Road Link Analysis

The volume of daily traffic on a roadway is one of the factors that aids in determining road classifications and appropriate lane requirements. To confirm roadway capacities, the background daily traffic volumes were calculated in vehicles per day (vpd) and compared to the environmental guidelines from the *Alberta Transportation Highway Geometric Design Guide* and *Rocky View County Servicing Standards*. The environmental guidelines represent the limit of comfortable operation of the roadway under most conditions, while the actual physical capacity of the roadway can be considerably higher.

The analysis is completed assuming a 1.5% growth rate on Highway 22. Traffic growth on Township Road 242 will be from development (i.e. the proposed gravel pit) and therefore is included in the Post Development analysis. The results of the daily link analysis are summarized in **Table 3.7**.

Table 3.7: Background Road Link Analysis

| ROADWAY LINK | CLASSIFICATION | GUIDELINE | BACKGROUND DAILY TRAFFIC VOLUMES (VPD) | | | | |
|----------------------------------|------------------------------------|-----------|--|-----------|-----------|--|--|
| | | (VPD) | 2019 | 2020 | 2040 | | |
| Highway 22 North of Hwy 8 | 2-Lane Highway | ~12,000 | 9,300 | 9,400 | 12,200 | | |
| Township Road 242 West of Hwy 22 | 2-lane Regional Moderate Volume | <500 | 115 | No change | No change | | |

The road link analysis suggests twinning of Highway 22 may be required by the 2040 Background horizon. This matches with the recommendations from the *Highway 22 Functional Planning Study*.

3.10 Alberta Transportation Warrants

3.10.1 Intersection Turning Warrants

Intersection type warrant was completed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The AT intersection warrant analysis is utilized at unsignalized at-grade intersections to determine if a left turn is required to eliminate interference caused by standing vehicles waiting to turn or a right turn lane is required to reduce obstruction to through movements.

Right Turn Warrants

The AT exclusive right turn lane warrant for a two-lane undivided highway states that three separate conditions should generally be met in order to warrant the need for such a lane. These criteria are:

- Average Annual Daily Traffic (AADT) volumes on the main road are greater than or equal to 1,800 vehicles per day (vpd);
- The intersected road exhibits daily traffic volumes greater than or equal to 900 vpd; and
- The right turn movement in question is greater than or equal to 360 vpd.

Based on this criteria, an exclusive right is not warranted as the intersected road daily traffic volumes will remain less than 900 vpd and the right turn movement in question will remain less than 360 vpd.

Left Turn Warrants

According to AT guidelines, the following two conditions should generally be met to warrant the need to construct an exclusive left turn taper and/or by-pass through lane on a two-lane highway:

- The peak hour opposing traffic volumes are greater than or equal to 100 vpd; and
- A minimum of five percent of advancing traffic is left turning during the peak hour periods.

Opposing traffic volumes are greater than 100 vpd in all horizons. However, left turning volumes are below 1% of advancing volumes. Since the left turning percentage is less than 5%, an upgraded intersection is not required based on technical warrant analysis, however AT guidelines state that engineering judgement may be used on roadways where main road volumes are greater than 4,000 vehicles per day, such as Highway 22, to determine the appropriate intersection treatment.

In the original TIA, Bunt & Associates recommended a Type IIIa intersection to provide separation from the through lane for vehicles turning left off Highway 22, this supported by a high number of through traffic volumes along Highway 22. Since the original TIA, it has been observed that traffic volumes along Highway 22 are not growing at as large of a rate initially projected, and that traffic volumes are lower now than they were in 2016, as well, the Rocky View County gravel pit that once operated along Township Road 242 is no longer in operation. Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

3.10.2 Illumination Warrant

A street light illumination warrant calculation was completed at the Highway 22 & Township Road 242 based on the *Transportation Association of Canada Illumination of Isolated Rural Intersections* guidelines, which are referred to in the AT Highway Lighting Guide. The warrant for illumination is used to determine if lighting an intersection is required based on several different factors such as geometrics, operations, environmental issues as well as collision history. The analysis (included in **Appendix E**) confirmed illumination is not warranted at any background horizon.

4. PROPOSED DEVELOPMENT

4.1 Land Use Information

The development permit is for a gravel pit operation. As per the development permit conditions, the hours of operation of the gravel pit (including aggregate excavating, hauling, crushing, stock piling, and stripping of overburden) are as follows:

- Hauling is permitted only from 7:00-17:00 on Monday-Saturday;
- Crushing is permitted only from 7:00-19:00 on Monday-Saturday; and
- · No operation will occur on Sundays and statutory holidays.

The facility will operate with the following constraints:

- All traffic will access the site via the parcel to the east, which is also owned by Mr. Copithorne. Traffic
 will travel to Highway 22 via Township Road 242. This is the current route used by previous
 operations on the site.
- All outbound truck traffic will turn right at Highway 22 and proceed to the south on Highway 22. Trucks will enter the site from both the north and south on Highway 22

The facility will have the following number of employees:

- A total of 2 full time employees will work on-site, there are no extra employees when crushing is
 occurring.
- No shift changes will occur.

Based on the 2019 haul data for the gravel pit provided in **Appendix** C, the facility will have the following truck trips:

- A maximum of 45 loads in one hauling day, which results in up to 90 truck trips per day (45 In, 45 Out). It is noted that this was the busiest day for the site in 2019, and that the average amount of loads processed on haul days in 2019 was 13. The truck trip generation for the site is therefore based on the worse-case scenario.
- All activity expected to be completed by 5 truck drivers.

4.2 Trip Generation

Trip generation for the proposed development permit was calculated based on first principles. The vehicular trip generation rates used in this study are as follows:

Employees (Passenger vehicle trips) - Based on employment hours, it is expected that both full time employees will arrive before the AM peak hour (7:30-8:30) and leave during the PM peak hour (16:15-

17:15). To be conservative, it is assumed that the employees arrive during the AM peak hour and leave during the PM peak hour.

AM Peak Hour: 2 trips (2 In, 0 Out)

PM Peak Hour: 2 trips (0 In, 2 Out)

• Daily: 4 trips (2 In, 2 Out)

Hauling (Truck trips) – Based on site operation information, a worst-case total of 90 daily trips are expected over the daily 10-hour operating period. This results in 9 truck trips during both peak hours (90/10 = 9).

AM Peak Hour: 9 trips (5 In, 4 Out)

• PM Peak Hour: 9 trips (4 In, 5 Out)

Daily: 90 trips (45 In, 45 Out)

Vehicular trip generation for the proposed development is summarized in Table 4.1.

Table 4.1: Trip Generation

| LAND USE | AM PEA | 1 | PM PEA | PM PEAK HOUR | | | DAILY | | |
|-------------------------|--------|----|--------|--------------|----|-----|-------|----|-----|
| | Total | In | Out | Total | In | Out | Total | In | Out |
| Passenger Vehicle Trips | 2 | 2 | 0 | 2 | 0 | 2 | 4 | 2 | 2 |
| Truck Trips | 9 | 5 | 4 | 9 | 4 | 5 | 90 | 45 | 45 |
| Total Trips | 11 | 7 | 4 | 11 | 4 | 7 | 94 | 47 | 47 |

4.3 Trip Distribution

Site traffic was distributed to the roadway network based on previous traffic patterns and indicated truck routing (i.e. trips oriented to the north/east will use the Highway 8 roundabout intersection to either make a U-turn or turn left). The trip distribution utilized is summarized in **Table 4.2**.

Table 4.2: Gravel Pit Trip Distribution

| DIRECTION | AM PEAK | HOUR | PM PEAK HOUR | |
|---|---------|------|--------------|------|
| | In | Out | In | Out |
| Employee Trips | | - ' | | |
| To/from the north on Highway 22 | 40% | 40% | 40% | 40% |
| To/from the south on Highway 22 | 60% | 60% | 60% | 60% |
| Total | 100% | 100% | 100% | 100% |
| Truck Trips | | | | |
| From the north on Highway 22 | 50% | - | 50% | - |
| To/from the south on Highway 22 | 50% | 80% | 50% | 80% |
| To north on Highway 22 via Hwy 8 roundabout | - | 20% | a | 20% |
| Total | 100% | 100% | 100% | 100% |

5. POST DEVELOPMENT TRAFFIC CONDITIONS

Site generated traffic volumes were superimposed onto background traffic volumes to obtain Post Development traffic volumes as summarized in **Exhibit 5.1**. The Long Term analysis is completed assuming the site will still be operating by 2040.

5.1 Intersection Capacity Analysis

The Post Development intersection capacity analysis is summarized in Table 5.1.

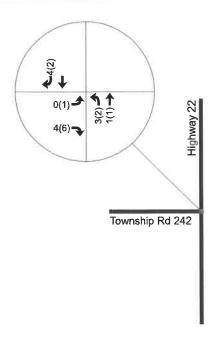
Table 5.1: Post Development Intersection Capacity Analysis (Highway 22 & Township Road 242)

| INTERSECTION | MOVEMENT | | AM PEAK HOUR | | | | PM PEAK HOUR | | | |
|--|----------|---|--------------|-----|-------|-------|--------------|-----|-------|-------|
| | & LANE | S | v/c | LOS | Delay | Queue | v/c | LOS | Delay | Queue |
| 2020 Post Development | EB | 1 | 0.04 | С | 17 | <5 | 0.08 | С | 18 | <5 |
| · | NB | 1 | 0.01 | Α | 0 | <5 | 0.01 | Α | 0 | <5 |
| | SB | 1 | 0.29 | Α | 0 | <5 | 0.38 | Α | 0 | <5 |
| | Overall | | - | Α | 0.3 | - | - | Α | 0.4 | - |
| 2040 Post Development (Existing Hwy 22) | EB | 1 | 0.07 | С | 22 | <5 | 0.12 | С | 25 | <5 |
| | NB | 1 | 0.01 | Α | 0 | <5 | 0.01 | Α | 0 | <5 |
| | SB | 1 | 0.38 | Α | 0 | <5 | 0.49 | Α | 0 | <5 |
| | Overall | | - | Α | 0.4 | 740 | - | Α | 0.5 | |
| 2040 Post Development (Twinned Hwy 22) | EB | 1 | 0.04 | С | 16 | <5 | 0.08 | С | 18 | <5 |
| | NB | 2 | 0.34 | Α | 0 | <5 | 0.26 | Α | 0 | <5 |
| | SB | 2 | 0.24 | Α | 0 | <5 | 0.33 | Α | 0 | <5 |
| | Overall | | - | Α | 0.2 | - | - | Α | 0.4 | |

The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.

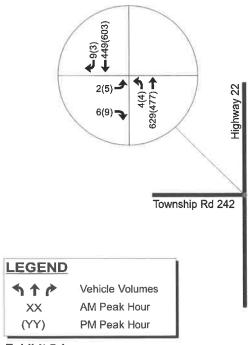


Site Traffic Volumes



2020 Post Development Traffic Volumes

2040 Post Development Traffic Volumes



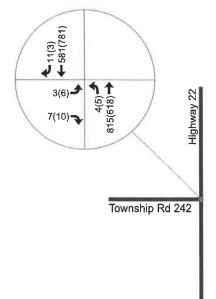


Exhibit 5.1

Post Development Traffic Volumes

Copithorne Gravel Pit TIA Update June 2020 Scale NTS



5.2 Road Links

5.2.1 Daily Volume Analysis

Post development daily traffic volumes are summarized in **Table 5.2**. The volumes presented for Township Road 242 represent the average and peak hauling days and not average annual daily traffic (AADT), which would be less than the average haul day as the site would only be operating half of the year.

Table 5.2: Post Development Road Link Analysis

| ROADWAY LINK | CLASSIFICATION | GUIDELINE (VPD) | POST DEVELOPMENT DAILY TRAFFIC VOLUMES | | | |
|-------------------------------------|------------------------------------|--------------------|--|-----------|-----------|--|
| | | | 2019 | 2020 | 2040 | |
| Highway 22 North of Hwy 8 | 2-Lane Highway | ~12,000 | 9,400 | 9,500 | 12,300 | |
| Township Road 242 West of Hwy 22 | 2-lane Regional Moderate Volume | <500 | 145 (Average) 210 (Peak) | No change | No change | |

The analysis confirmed all roadway links in the study area would continue to carry traffic volumes within their respective environmental guidelines after the addition of the proposed development. Therefore, current roadway classifications are adequate to accommodate site traffic volumes.

5.2.2 Township Road 242

Township Road 242 is expected to have an AADT of less than 145 vehicles per day. AADT calculations would normally consider that the proposed gravel pit operations will only operate half of the year. However, this calculation assumes the proposed gravel pit will operate at capacity over the entire operating season.

Due to truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.

5.3 Alberta Transportation Warrants

5.3.1 Intersection Turning Warrants

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

5.3.2 Illumination Warrant

A street light illumination warrant calculation was completed at the Highway 22 & Township Road 242 confirmed illumination is not warranted at both the 2020 and 2040 Post Development horizons.

FINDINGS AND RECOMMENDATIONS

The findings and recommendations made as a result of the TIA update are listed below:

6.1.1 Collision History

A total of 5 collisions occurred at the intersection over a 14-year period. Three of the collisions were single-vehicle (struck animal; run-off-road). A collision occurred in 2005 that involved 3 vehicles with 4 injuries and 2 fatalities. A 2015 rear-end collision involving 2 trucks is also documented.

6.1.2 Sight Distance

All minimum and desirable sight distance requirements are met at the intersection.

6.1.3 Background Intersection Capacity Analysis

The Background intersection capacity analysis indicates Highway 22 & Township Road 242 will continue operating within acceptable capacity parameters during the Opening Day (2020) and Long Term (2040) horizons.

6.1.4 Post Development Intersection Capacity Analysis

The Post Development intersection capacity analysis indicates no appreciable change in peak hour operations will occur at the intersection of Highway 22 & Township Road 242 due to the proposed development.

6.1.5 Alberta Transportation Intersection Warrant

Intersection type warrant were reviewed for the intersection of Highway 22 & Township Road 242 following the AT Highway Geometric Design Guide. The left turning volume will remain below 5% of advancing volumes (at approximately 1%). Given the low volume of traffic along Highway 242, and low turning volume off of Highway 22 onto Township Road 242, as well as the consideration of future planned improvements along Highway 22 at this intersection; Bunt & Associates recommends that the existing Type 1 intersection treatment be maintained at this location.

6.1.6 Alberta Transportation Illumination Warrant

Street light illumination is not warranted at the intersection.

6.1.7 Highway 22

The Background road link analysis suggests twinning of Highway 22 may be required by the Long Term Background horizon (2040). This matches with the recommendations from the *Highway 22 Functional Planning Study*. No appreciable change will occur due to development traffic volumes.

6.1.8 Township Road 242

The roadway is expected to continue operating within its daily vehicle capacity. Due to the expected truck traffic, annual dust control (through the application of Calcium Chloride) will be required on Township Road 242.

It is recommended that the gravel pit operator require all truck drivers operate at a maximum speed of 50 km/h along Township Road 242. The roadway has rolling terrain, as such a reduction in operating speed from 60 km/h to 50 km/h would improve stopping sight distances by 20 metres. This could also be implemented for all road users by reducing the speed limit on the roadway to 50 km/h.



APPENDIX A

Scope of Work

Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242

Monday, 8 February, 2016 9:50:19 AM Mountain Standard Time Date:

From: AYurkowski@rockyview.ca

Ezekiel Dada To:

CC: Trevor.Richelhof@gov.ab.ca, Amrit Uppal

Thanks Ezekiel, sounds good.

Angela

ANGELA YURKOWSKI (BOOTH), P.Eng Municipal Engineer | Engineering Services

ROCKY VIEW COUNTY 911 - 32 Avenue NE | Calgary | AB | T2E 6X6

Phone: 403-520-7289

ayurkowski@rockyview.ca | www.rockyview.ca

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From: Ezekiel Dada [mailto:edada@bunteng.com]

Sent: Friday, February 5, 2016 10:31 AM

To: Angela Yurkowski

Cc: Trevor.Richelhof@gov.ab.ca; Amrit Uppal

Subject: Re: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Angela,

Thanks for your reply. We will prepare the TIA to meet both the County and AT's TIA guidelines.

Regarding horizon, we plan on analyzing "Opening Day" that is, existing plus site traffic and 20-year, that is, Highway 22 traffic plus growth plus site traffic (assuming the gravel pit can be mined for more than 20 years).

Thank you.

Ezekiel

From: "AYurkowski@rockyview.ca" <AYurkowski@rockyview.ca>

Date: Friday, February 5, 2016 at 9:54 AM

To: Ezekiel Dada <edada@bunteng.com>

Cc: Amrit Uppal auppal@bunteng.com, Trevor Richelhof Trevor.Richelhof@gov.ab.ca

Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Ezekiel,

The county requirements for TIA's are outlined in our servicing standards (Section 401). http://www.rockyview.ca/Portals/0/Files/BuildingPlanning/Standards/Servicing-Standards.pdf

Specifically for this application, we want to see a review of TWP 242 to confirm appropriate road designation and identify any road improvements that may be needed based on volume and vehicle classification.

Also, what horizons will you be looking at?

Thanks

ANGELA YURKOWSKI (BOOTH), P.ENG Municipal Engineer | Engineering Services

ROCKY VIEW COUNTY

911 - 32 Avenue NE | Calgary | AB | T2E 6X6

Phone: 403-520-7289

ayurkowski@rockyview.ca | www.rockyview.ca

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From: Ezekiel Dada [mailto:edada@bunteng.com]
Sent: Thursday, February 4, 2016 3:45 PM

To: Trevor Richelhof; Angela Yurkowski

Cc: Vince Diot; Jerry Lau; Clarke Bullock; Amrit Uppal

Subject: Re: Scope of TIA - Copithorne Gravel Pit at Twp 242

Thanks Trevor,

Ezekiel

From: Trevor Richelhof < Trevor. Richelhof@gov.ab.ca>

Date: Thursday, February 4, 2016 at 3:31 PM

To: Ezekiel Dada <<u>edada@bunteng.com</u>>, "<u>ayurkowski@rockyview.ca</u>" <<u>ayurkowski@rockyview.ca</u>> Cc: "<u>vdiot@rockyview.ca</u>" <<u>vdiot@rockyview.ca</u>>, Jerry Lau <<u>Jerry.Lau@gov.ab.ca</u>>, Clarke Bullock

<clarke.bullock@gov.ab.ca>, Amrit Uppal <auppal@bunteng.com>

Subject: RE: Scope of TIA - Copithorne Gravel Pit at Twp 242

Ezekiel, for the intersection of Highway 22 and Township Road 242, I would recommend the scope of

the study follows the attached guideline, in addition to any other requirements of Rocky View County.

Regards,

Trevor Richelhof

Development / Planning Technologist
Delivery Services, Southern Region
Alberta Transportation
Government of Alberta
2nd Floor, 803 Manning Road NE, Calgary AB T2E 7M8

Tel 403-297-6311 Fax 403-297-7682 Trevor.Richelhof@gov.ab.ca

511 Alberta - Alberta's Official Road Reports Go to 511.alberta.ca and follow @511Alberta



From: Ezekiel Dada [mailto:edada@bunteng.com]
Sent: Thursday, February 04, 2016 2:59 PM
To: ayurkowski@rockyview.ca; Trevor Richelhof

Cc: vdiot@rockyview.ca; Jerry Lau; Clarke Bullock; Amrit Uppal Subject: Scope of TIA - Copithorne Gravel Pit at Twp 242

Hi Angela and Trevor,

We have been retained by Roy Copithorne to provide a TIA requested by the County in support of a development application for ongoing gravel pit operation located west of Highway 22 on Township Road 242. I attached the Development Appeals Board order for your reference.

Please find below our suggested scope for the study and let me know if it is acceptable to the County and Alberta Transportation.

?

- ? Undertake 6-hour weekday intersection traffic counts for the following intersections:
 - ? Highway 22 & Twp 242
 - ? Range road 45 & Twp 242 (if necessary)

Up-to-date road information, including traffic delays, is a click or a call away. Call 5-1-1 toll-free, visit 511.alberta.ca or follow us on Twitter @511Alberta to get on the road to safer travel.

http://511.alberta.ca/ab/en.html https://twitter.com/511Alberta

ATTACHMENT 'A': UPDATED TRAFFIC IMPACT ASSESSMENT

L-1 Page 36 of 62

This email and any files transmitted with it are confidential and intended solely for the use of the individual or entity to whom they are addressed. If you have received this email in error please notify the system manager. This message contains confidential information and is intended only for the individual named. If you are not the



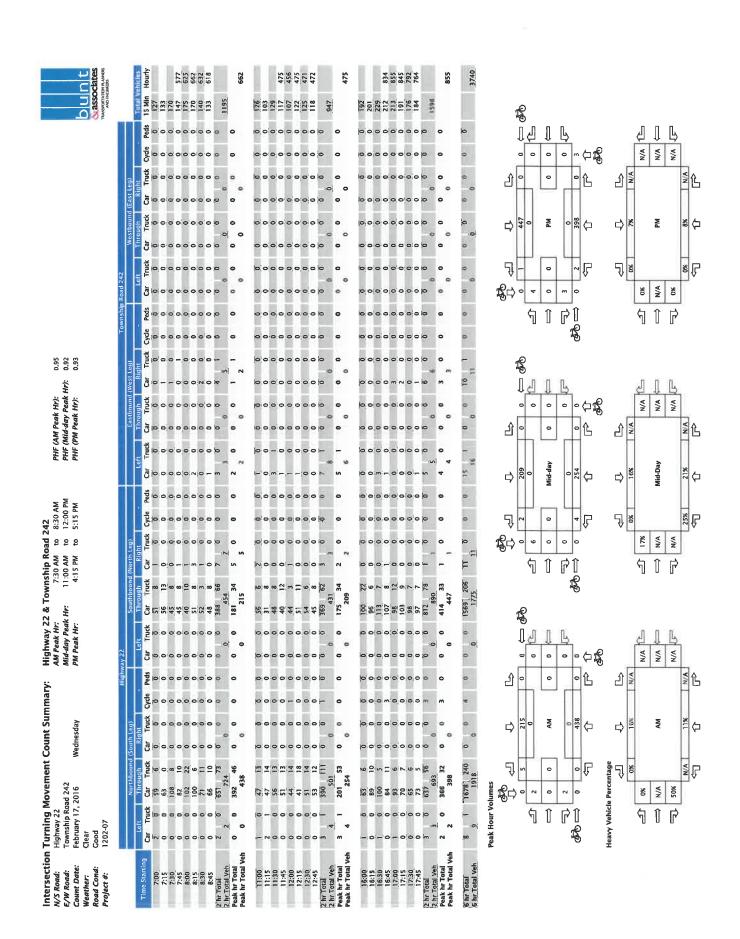
APPENDIX B

Collision Data

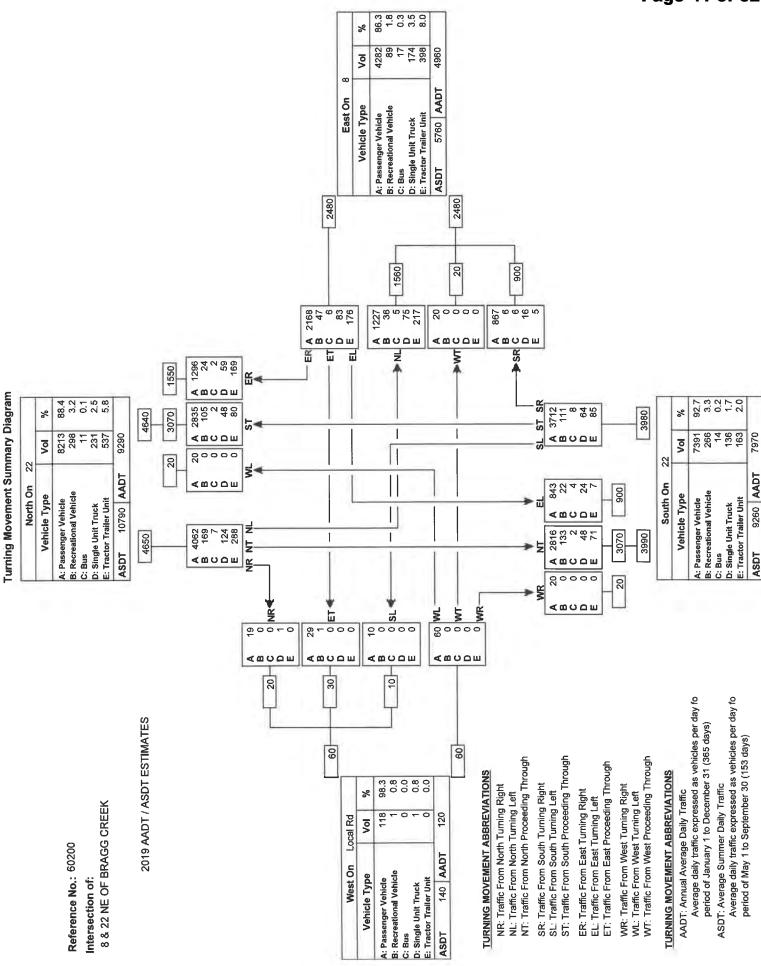
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| and a coffee | an condition | , | e 17 | n n | en e | ٠. | | | - | | | | | | | 1 | 4 | | | | | ~ | | | | - | - | | | | | ٠ ، | - | | | | | , | | | | | | | - | | ~ - | | | | | | | | 1 | | ON THE LEFT F | SHIP BOAD 243 | SHIP BOAD 242 I THE LEFT FRO | TRUCTION ZON | TRUCTION ZON | STOPPING DA | OLE STOPPING | |
| deministra | condition | 31 | 9 69 | 203 | 649 | 924 | 908 | 27 | 33.486 | 319 | 328 | 262 | 970 | 2 2 | 174 | 232 | 34.58 | 35 | 35,715 | 707 | 200 | SQE | 514 | 282 | 4,139 | 873 | 33,282 | 212 | 55,54 | 35,724 | 822 | 34.428 | 415 | 13.151 | 33,323 | 35.695 | 256 | 33.273 | 281 | 33.347 | 34,194 | 55.723 | 35,692 | 33.531 | 33.923 | 34.216 | 15,702 | 34.692 | 35,691 | 35.406 | 548 | 38.939 | 34.654 | 217 | 100 | 100 | NATTHING DUE | PART OT NEW | WINTO TOWN TITNG DEER O | SWOO - EH3A O | D VDH3 - CONS | DUND, VEHICL | facultib, VEH | |
| | section for plast | | | 10 33. | | | | | | | | 20 33 | 15 320 | 15 35 | 15 35 | 15 35/ | 15 34 | 2 2 | 2 2 | | | | 15 30 | | | | 10 30 | 15 35. | 10 35 | 15 35. | | | | | | | | | | | | | 12: | | | | | | | | 2 20 | | | 2 2 | 2 2 | 15 34. | FROM DITOR | MALES TO TUR | MALED TO TUR RDM DITCH ME | DOPPED SENIN | MINDEGRAMO | INVAY WESTE | IIGHWAY WES | |
| major | | Ξ: | 2 2 | 3 2 | 3 : | \$ 5 | 2.5 | 3 | 2 2 | 3 3 | 2 | 3.3 | Ξ | 3 3 | 2 : | 2 3 | 3 3 | 2 | Z Z | Ξ: | 3 2 | Z: | 2 2 | Ξ: | 3 3 | 11 | 2.2 | 7 | 3 3 | 3 | 3.3 | 2.2 | 2 2 | 1 | 1 1 | 2 3 | 1 1 | 3 3 | 1 1 | 2 2 | : : | # # | : :: : | 2 2 | 14 | 4 2 | 3.3 | Ξ. | 2 2 | 33 | 1 2 | 2 2 | 2 : | 2.2 | 3,3 | 2 | UNIVERSION | CY 22, IT SIG | IV 22, IT SIG MIPED DUTTE | WGER, NO II | THE WASSI | ING THE NG | SSING THE H | |
| 1 | | | 2 22 | :: :: a a | 22 12 | 77 77 89 | 22 25 | 22 | # # # # | 2 22 | 2 2 2 | 2 2 2 | 121 | នេះ នេះ | 22 | 2 22 2 28 | 22 22 | 2 2 2 2 | 2 2 | 22 16 | 2 2 2 | 94 22 | 12 12 12 12 13 12 | 94 22 | 2 2 3 3 | 22 22 | 2 22 | 22 | 12 12 13 14 15 15 15 15 15 15 15 15 15 15 15 15 15 | M 22 | z z | 3 22 E | | | 2 2 2 3 | | | | | 46 22 | 2 2 | X 2 | 2 22 2 | 3 23 3 3 3 | 22 | 22 22 | 22 22 | 2 2 | = = = = | 96 | : : : : | 22 22 36 35 | 11 | z z | 22 22 | Z : | THEN DEER) | DEER, I PAS OF HIGHWA | OF HOUGHWAX ON DEEN AUS. | FR. 1 PASSE 242 - SB. V | 242 - SB. V | EER CROSSI | A DEER CRO | |
| 1 collision | 10 | | | | | | | | | | | | | | | 45 | | 30 | | | | | | | | | 47 | | | | Ę | ř | | | | 4 | ř | | | | | r | ä | | ā | | | | | | | | | | 10 | 42 | OF HWY I W | LING SOUTH | HAND SOUTH I | CI WITH DE | OF TWP RD | STRUCKAD | 22, STRUCK | |
| Cooke | N N | 42 | 38 | 5 5 | 42 | 47 | 2 4 | 2 | ÷ ÷ | 47 | 1.5 | 4 4 | 4 | 40 | 69 | 5 5 | 5 5 | \$ \$ | 0 0 | 47 | 47 | 43 | 43 | 43 | 42 | z | 5 5 | 43 | 47 | 4 | G C | ÷ 5 | 4 5 | 4 | E 6 | 4 1 | 4 6 | 47 | 4 2 | 47 | 7 | 41 | 1 12 1 | 47 | 47 | 44 | 2.5 | 44 | ÷ ÷ | Q. | ÷ ÷ | 4 | 47 | 12 | 47 | - | HUTOS MUS | WY 22, COL | WAS TRAWFUL MISDUTH OF | FZZ, COLLED JUST NONTH | ILUST MORTH | IGHWAY 22. | NHIGHWAY | |
| 199 | | | | | | | | | | | | | | | | = | | 0. | | | | | | | | | и | | | | | ^ | | | | | - | | | | | e | | ı | 3 | | | | | | | | | | | e e | N HWY 22 - 3 | ACK TRUCK V | ACK TRUCK I | UND DRI HAV ON HAVY 22 | ON HWY 22 | NO DWINDS | THREE LIND O | |
| Web2 | | | - 4 | a | | Dr vet | - 0 | - | e v | | | ~ 0 | n en | | - | v + | e4 s | n e | s = | | en en | *** | | 7 | w v | , an | v 0 | - 10 | v = | - | | | en e | n 10 | e4 (2) | 10.1 | h sh | | h el | en 10 | ۰. | | n in | er so | - | 10 F | | n 40 | un un | + | n m | an | ю. | gh wi | etie | in. | HEDUND O | TIT NORTH | DODODADIM SOUND ON | ARYCLIGHT | AFFIC LIGHT | ING SOUTH | ELLING SOU | |
| 1 | direction | | 9.0 | | | - o | W 6 | | | 2 = | | ٦. | | - | _ | - 10 | | 9 5 | ,,, | | E +1 | vet - | | _ | ~ = | . 6 | - | - | | | | - 2 | - 0 | h | 1 2 | - | 9 6 | -1 4 | | ø - | | | | | _ | - 0 | | | | | | | | | 2.5 | 10 | ELLING SOUT | DODOSTORICO | DOOSTOCOO | VEHICLE II | SPPED AT TR | ICUE TRAVELL | EH/CLE TRAY | |
| - | i A | m | e N | | m | | | m | en en | 2 5 | 5 FF | | n m | | | m m | | 9 60 | m, m | т, | ** | - | m m | m | en et | en en | n 0 | 100 | m m | m | m r | 5 11 | en e | | m m | | | | | m n | 9 19 | en r | 9 19 | m m | en E e | m r | | n re | m m | | n e | m m | | ~ ~ | e) - | i et | DIE 1 TRAW | 14 035.32 | 14 035.320 E 1 TRAVEU | 0100000000 01VEH3 STC | DIVERB STO | DOOD VEHI | Motocoo | |
| | The Street | 53 | 14.05 | 57 st | | 15.45 | 23.4 | 7.15 | 23.3 | 5.3 | 17.3 | 21.3 | 19.3 | 7.8 | 16.05 | 3.3 | 00 2 | 93 | 21,45 | 5.3 | 23.34 | 4 | 2.5 | 17.3 | 9 6 | 6.11 | 15.05 | 1112 | 145 | 5.45 | 19 | 16 | 513 | 18 | 113 | 22 | 17.3 | 18.3 | 16,45 | a : | 7.15 | 17.15 | 2 2 | 25.3 | 2 | 20.45 | 22.00 | 7,06 | B 15 B | 20.15 | ត្ <u>ន</u> | 2 S | Z . | 4 ~ | 613 | 7.04 | DODDESSWITH | 035.4610031 360734022 (| 369734022 000 VEHIC | 905.4610010 | 0000000000 | 1000101000 | 000101000 | |
| - | | | | | 90-01 | 200 | 07:13 | 13.30 | 2012-12-31 | 11.04 | 171 | 10-20 | 1941 | 12-05 | 11-23 | 07-19 M-18 | 10.23 | | | | 2019-07-20 | | 21-05 | 20-17 | | | | M-75 | 20-02 | 11-00 | 13-11 | 10-60 | 98-26 | 01-14 | 00-31 | 90-00 | 12.23 | 90-90 | 05-11 | 61.50 | 11-10 | 12.27 | 7002-07-01 7002-07-01 | 92-20 | 92:50 | 99.16 | 11-07 | 01-50 | 13-18 | 97.11 | 2004-08-30 | 08-06 | 08-27 | 90-50 | 10-14 | 10-15 | 0000010000 | 94022 D14 2.003060703 | 2.003060703 | 94022 014 | 135,453,000,01 | 01A 035.30 | 1014 035.M | |
| | date | 2022 | 2012 | 2022 | 2022 | 2002 | 2002 | 1017 | 102 | 1022 | 2013 | 2011 | 2002 | tint | 2011 | 2010 | 2010 | 2010 | dipe | 2010 | 2010 | | | 5002 | 2002 | 2002 | 2009 | 2003 | MODE MODE | 2008 | 2008 | 3000 | 3000 | 2007 | 2007 | 5000 | 2002 | 2006 | 2002 | 2002 | 2005 | 2005 | 502 | 5002 | 700% | 2005 | 7007 | 2007 | 7000 | 2009 | 200 | 2003 | 2003 | 2002 | 2003 | 2003 | 7003 014 035.42 | ALSZOLDI 40 | 2015/10/08: | 28022 014 | 28022 014 | 105 47 9402 | 105 47 9402 | |
| | special reference location description | | | | | | | | | | | | | | | | | | | | | 3 | 2 IOM SOUTH OF WOUND ABOUT HEADYNG NDRTH | | | | | | | | | | | | | | | | | | | | | | ı | | HWYS | | | | | | | | | MUDGE | 3105 47 94022 | BETWEEN INGHWAY BAND SPRINGBANK ROAD 2015/10/1006.45,20101 47 94022 014 035 46150; BY VOLIDER STEVBLITURINDEF - IDWINSHP ROAD 24.2 ARLS/10/0812.000060703360734022 | AD 242 2005 47 14022 | 2015/10/1001 | 402080505474 | /11/2821 0030 | 71/2811.0030 | |
| | ovence locards | | | | | WYB | | | CORCLI | | | | | | | | | | | | | STWN Black CRC & COCHAME | DOMD ABOU | | | | | | | | - | TO MICH | MONT | | | g | | | | 1 | NC NC | | | | OF HWY 8 | | HIGHWAY 22 AND 2 KNS IN OF HWY 8 | 300 | | 242 | | | | BAIDGE | | SOUTH END OF ELBOW RIVER BRIDGE | 07/1511.003 | UD DWANSHIP RO | DWINSHIP RC 02/15/11.008 | UD 213/07/0317 | 713/01/01/2 | 2013 | 2011 | |
| | special rel | NEAR HWY B. | H467Y 22 | S OF HIGHWAY B | | LUST NONTH OF HMYT | | | NORTH OF TRAFFIC CIRCLE | | | | | | | | | TRAFFICCIRGLE | | | | Y Blucs CR | SOUTHOU | | S CORP COST OF STATE | | | | | | | NORTH PLUFFIC CIRCLE | WORTH DE ROUNAABOUT | | | SPRING BANK ROAD | | | | and the same of the same of | E TO HAVE A | | | | MORTHBOUND NOF HWY 8 | | WAY 22 AM | ELBOW RIVER GRIDGE | | DWHSHIP ROAD 242 | | | | ON ELBOW RIVER BRIDGE | KEAR HAVY E | THE WOOF | 2015 | WGBADHK NO. TURNOFF - 1 | TURNOFF - 1 | VCALDER RO | ~ ~ | 4 | | |
| 8 | | | 2603 | Left. | | TSUL | | | MOR | | | 100 | | | | | i | TRA | | N/B | | WT9 | ZIDN | | 14.44 | Š | 5 | ŧ | 8 | | - | NON | MDR | WS DRIVE | | SPRII | | | | | | | w | | NOK | | HIGH | ELBIO | | WCI | | | | ND D | KEN | nos | | ATR STEVEN | KERSTEVEN | HIGHWAY II AND SPAINGASHIC ROAD | | 51.03784 -114.48675 | 114.45576 | |
| at intersects | reference point | - CENTRAL PROPERTY. | | TRAFFIC CIRCL | | | | | CREEK | | _ | BRAGG CREEK TURN OFF | CALLEA | | | | | | _ [| | | | | BRASS ORTEK | CHEEK | 2142 | 9100 | | COCHRAME | and the same of th | | | | WOOD MEADOWS DRIVE | CREEK | | | BRAGG CREEK | CHEEK | EAGC CHEEK | MAYER BIRD | | OF PLEMERO WALCE BLYNNY | CAMP GARDENER SPIING BANK ND | A Abelia | HANN SE | HWY B | MYR | | | TWI. | N 1. BRAGIT CREST | AV PS | AY RG | | | COCHEAGE AS | EN HAGHWAY BY VOL | BY VOL | TH HEGITWAY | | 53.03784 | 51.03784 | |
| | | | HWV | HWY B | PWW 8 | HWY 1 | BWY B | HWY 1 | BRAGG CREEK | HWY 3 | HWY B | BRAGE | WWY 3 | WWY 3 | HWW 3 | HWY 1 | HWP7 3 | HWY 3 | HANT 28 | i | | | HAVE | BRASE | BIMGG | TWP RD 242 | HAM'S COCHESIES | DF RIVER | COCHRAME | HWYB | HWYE | HMYB | *************************************** | REDWA | BRAGG CREEK | HWTB | HANT'S | BRAGG | BRAGG | ENGO | HAVYE | LAMPY 1 | ST FOW | SPRING | Total S | WH HIS | HWY B | BRAGG CIRE HIGHWAY & | HWITE | | DF B1 HWY HWY 1 | HWY 1 | HIGHWAY PS | HBGSTWAY PR | 200 000 | HWY 13 | | | | BETWEEN | 10 242 | WY B | HAY 8 | |
| not at | Intersection Intersection Intersection | Total Control | | 2 | × | ы | 16 1 | 1 16 | 2 × | Σ, | e se | ad a | d 2d | set se | e set | né né | 1 340 1 | × × | = 1 | | 3 . | | w w | saf. | × | ž | w 1 | , 5 | lat h | e se | ъ. | м м | 1 | жм | 23 | i | | ъ. | ыы | м | w w | M | w w | ы м | | w 1 | е м | u S | ым | 4 | мж | мъ | 4 34 | × | , | e S | 4 005.005 ISHWY 1 | TOWNSHIP ROAD 242 000.00 | 2 DDD. | 30 THE MITWE | IOO, DON MITWP RD 242 | CONTRHICATOR | DON ENGLY | |
| nol at | in Intersects | The same | ss 98 | \$ 000 | 1.5 | 7.5 | N 2 | 100 | 103 1 M | 152.4.5 | 2.5 | 305 | N N N | NS. N. P. | | 15 E | E | 22 | N I | N | 13 | | 2 2 | 10 E | 10 E | N 005 | N. I | N 005 | 15.5 | 0.02 N | N E | 2 2 | | Z Z | 3.5 8 | 2 10 | 1 N N | N OI | 10 2 | 10 W | 2 K S | 99 | in in | 25 N | 1 | 0.5 H | N 2 | 10 N | 35 N | E. | 05 10 10 W | 4 4 5 | - N | N 11 | - Taken | H 005 | 14.5 | 1P ROAD 24 | HIP ROAD 24 | 1000 | 8 | out. | 100 | |
| | | | | | | | | | | - | | | | | | | | | | | | | | | | as. | | æ | | ď | | | | | | | | | | | _ 48 | | _ | 6 | | | | | | | _ | | | ~ | | | | POVMS | TOWNS | | | | | |
| | 1 | | | | | | | | 22 22 | ω. | 108 22 | 22 | UNIE ZZ | TAME 22 | VAIE 22 | WHE 22 | VAIR 22 | NAVE 22 | TANE ZZ | 1CK 22 | TOUR IN | VAIE 22 | 200 | 5 72 | 2 2 2 | JAME 22 | TAME ZZ | 4CR 22 | TAME 22 | 000 22 | 5Dk 22 | TANK 22 | 308 | NAME 21 | 200 | MAR 22 | 87 22 87 22 | 5CE 32 | SAME 22 | 200 | TCR 22 | NAIL 22 | PANE 22 | MARE 22 | N. | | 4 60 | | | | 22 22 | | - 10 | | | | | | | | | | | |
| | Hope Place Name | 2 000/80/81 | 2 PEDWO | 2 BM65 CR | 2 0004 | 2 5556 | 2 COCHUM | Z BRAGG C | 2 BRAGG CR | 2 COCHAM | Z BRAGG CS | Z PRIDDIS | Z COCHUM | 2 COCHAM | 2 COCHAN | 2 COD/RAN | 2 COCHANI | 2 COOHBIN | 2 CDCHRAN | 2 BANGG CI | 2 COCHAN | 2 COCHRANE | 2 BRAGG CF | 2 PREDDIS | Z PREDDIS | 2 COCHRAM | 2 COCHRAM | 2 BRAGGUR | 2 COCHINA | 2 REDW. | 3 BRAGG CH | 2 COCHISAN 2 COCHISAN | 2 BRAGG CR | 2 CDCHRAN | 2 88464 0 | 2 CDCHRAMI | 2 CALGARY | 2 BRAGG C | 2 COCHBAN 2 BRAGGCH | 2 BRACGCR | 2 COCHEAN 2 BRAGGCS | 2 000/898 | 2 CDCHBANE 2 CDCHBANE | 2 BRAGGICS 2 CDCHRAN | 2 COCHANIE | 2 CALGASS | 2 COCHIMIS | 2 BRASECH 2 SANCH | 2 BRAGE CR | 2 CDCHRNIG | 2 BRAGGOR | 2 BRAGG CB | 2 CDO-WAY | Z BRAGG CR | Z REDWOOL | 2 CODBINE | 2 COOH | 65 CR022 | WANTED22 | GG CROD2 | WEAMEO 22 | 4000 022 | WOOD 022 | |
| | Police | | | | | | | | £ 12 | | | | je je | 191 | i ie | 192 | | G | 787 | | 78. | | CPL SX | | 635 | ē | le l | ē | | - | Ř | R R | | 2 2 | 787 | 787 | 787 | 787 | 767 | 757 | G G | 767 | 2 % | | 100 | | | | | | | | | | | | 8 267 2 000 0010767200C/WANE022 | 0010767289A6G CR022 | DESCRIPTION OF STREET | ANDETERMA. | 7207672000 | 20767200C | 1107672780 | |
| 1202-07 | Year Choe Na. | 2012 2731455 | 2012 2676346 2012 2625817 | 2012 2702570 | 2012 2704)46 | 2012 2704290 | 2012 2787315 | 2012 Z79A400 | 2012 2795367 | 2011 ZANDAL | 2011 2762594 | 2011 2766758 | 2011 2757566 | 2012 2742362 | Z011 27444090 | S100530 1102 | 2010 2442922 | 2010 2393308 | 2010 2731429 | 2010 2728751 | 2010 272930% | 2010 2443434 | 2009 2553091 | 2009 2699073 | 2009 2706068 | 2003 2003605 | 2009 2038623 | 2008 2589975 | 2003 2597209 | 2008 2815309 | 2008 2000040 | 2003 2102103 | 2008 2727227 | 2007 2578599 | 2007 2547602 | 2007 1967368 | 2007 2005704 | 2006 2571906 | 2006 2597136 | 2006 2560513 | 2006 2564957 2006 2564957 | 2005 2513494 | 2005 2525622 | 2025 2535696 | HOPE HANGES | 2005 2545946 | 2004 1597276 | 2004 1378756 | 7004 2525101 | 2004 2505791 | 2004 2505602 | 9505052 9007 | 2003 2447107 | 2003 2428454 | 2002 2472384 | 2003 1405730 | 2015/05/05/16/3 | 2015060892500 | | 20150608929 0020767288AGG CN022 | 201325 LS280 00207672CDCHBAME022 | 10132515250 B 101302515550 B | 20130281585 O. | |

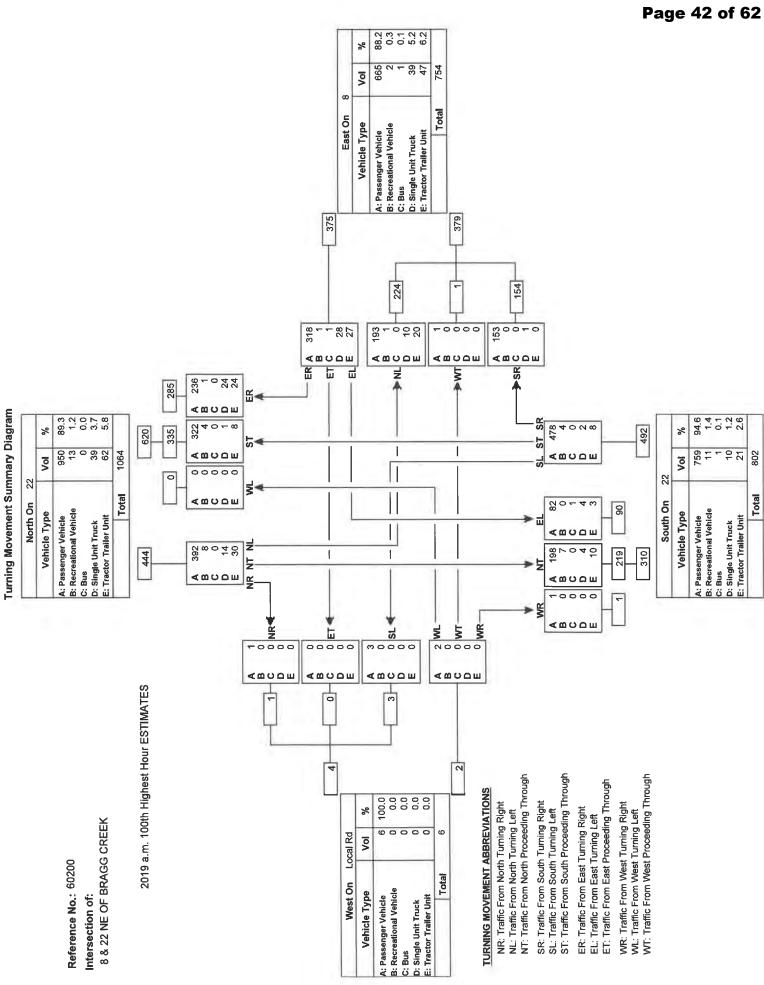
APPENDIX C

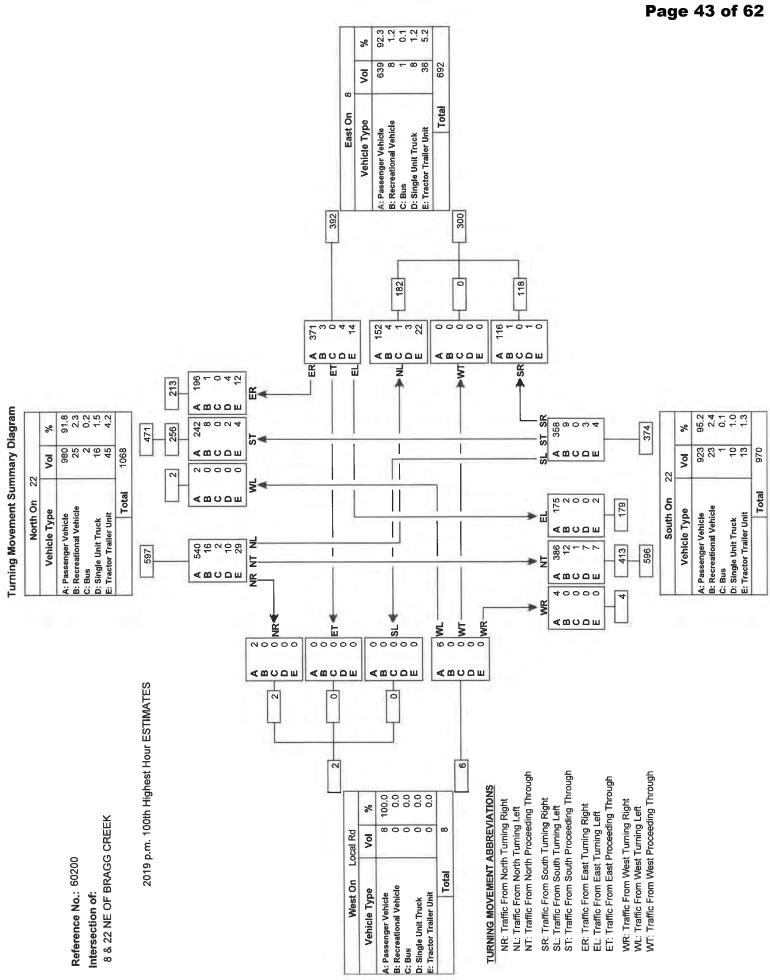
Traffic Data



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26 permitted haul days in July

Copithorne Pit 2019 Truck Haul Summary

| One-Way | One-Way Two-Way | | One-Way Two-Way | Two-Way | | One-Way Two-Way | Two-Wa |
|---------|-----------------|--------------------------------|-----------------|---------|-------------------------|-----------------|--------|
| Trips | Trips | Month of June | Trips | Trips | Month of July | Trips | Trips |
| 11 | 22 | 04/06/2019 | 9 | 12 | 02/07/2019 | 2 | 4 |
| 4 | 00 | 05/06/2019 | 12 | 24 | 03/07/2019 | 4 | ∞ |
| | | 06/06/2019 | 23 | 46 | 12/07/2019 | 5 | 10 |
| in June | | 07/06/2019 | ĸ | 9 | 17/07/2019 | ∞ | 16 |
| | | 10/06/2019 | 6 | 18 | 18/07/2019 | 10 | 20 |
| | | 12/06/2019 | 26 | 52 | 19/07/2019 | 16 | 32 |
| | | 13/06/2019 | 14 | 28 | 22/07/2019 | 7 | 14 |
| | | 17/06/2019 | 3 | 9 | 23/07/2019 | 11 | 22 |
| | | 18/06/2019 | 5 | 10 | 24/07/2019 | 31 | 62 |
| | | 20/06/2019 | 2 | 4 | 25/07/2019 | 21 | 42 |
| | | 25/06/2019 | 3 | 9 | 26/07/2019 | 22 | 44 |
| | | 27/06/2019 | 1 | 2 | 27/07/2019 | 10 | 20 |
| | | 29/06/2019 | 5 | 10 | 29/07/2019 | 14 | 28 |
| | | 13 days of haul in June | ß. | | 30/07/2019 | 25 | 20 |
| | | 25 permitted haul days in June | s in June | | 31/07/2019 | 24 | 48 |
| | | | | | 15 days of haul in July | | |

27 permitted haul days in June

2 days of haul in May

14/05/2019

Month of May 13/05/2019

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27 permitted haul days in October

Copithorne Pit 2019 Truck Haul Summary

| | One-Way Two-Way | Two-Way | | One-Way Two-Way | Two-Way | | One-Way Two-Way | Two-Way |
|----------------------------------|-----------------|---------|-------------------------------------|-----------------|---------|----------------------------|-----------------|----------|
| Month of August | Trips | Trips | Month of September | Trips | Trips | Month of October | Trips | Trips |
| 01/08/2019 | 28 | 56 | 03/09/2019 | 19 | 38 | 01/10/2019 | ις | 10 |
| 02/08/2019 | 19 | 38 | 04/09/2019 | 25 | 20 | 02/10/2019 | 15 | 30 |
| 06/08/2019 | 4 | ∞ | 05/09/2019 | 13 | 26 | 03/10/2019 | 22 | 44 |
| 07/08/2019 | 18 | 36 | 06/09/2019 | 19 | 38 | 04/10/2019 | 30 | 09 |
| 08/08/2019 | 16 | 32 | 09/09/2019 | 1 | 2 | 05/10/2019 | 4 | ∞ |
| 09/08/2019 | 4 | 00 | 12/09/2019 | 9 | 12 | 07/10/2019 | 12 | 24 |
| 10/08/2019 | 9 | 12 | 13/09/2019 | 10 | 20 | 09/10/2019 | П | 2 |
| 13/08/2019 | 15 | 30 | 14/09/2019 | က | 9 | 10/10/2019 | 20 | 40 |
| 14/08/2019 | 15 | 30 | 16/09/2019 | 14 | 28 | 11/10/2019 | 5 | 10 |
| 15/08/2019 | 14 | 28 | 17/09/2019 | 20 | 40 | 12/10/2019 | Ŋ | 10 |
| 16/08/2019 | 4 | 00 | 18/09/2019 | 20 | 40 | 15/10/2019 | 7 | 14 |
| 19/08/2019 | 14 | 28 | 19/09/2019 | 2 | 10 | 16/10/2019 | 2 | 4 |
| 20/08/2019 | 25 | 50 | 20/09/2019 | ᠳ | 2 | 17/10/2019 | 10 | 20 |
| 21/08/2019 | 32 | 64 | 21/09/2019 | 7 | 4 | 18/10/2019 | 2 | 4 |
| 22/08/2019 | 14 | 28 | 23/09/2019 | 27 | 54 | 19/10/2019 | ĸ | 9 |
| 23/08/2019 | œ | 16 | 24/09/2019 | 17 | 34 | 21/10/2019 | ∞ | 16 |
| 26/08/2019 | 16 | 32 | 25/09/2019 | 13 | 56 | 22/10/2019 | က | 9 |
| 27/08/2019 | 16 | 32 | 26/09/2019 | ∞ | 16 | 23/10/2019 | æ | 9 |
| 28/08/2019 | 21 | 42 | 27/09/2019 | 23 | 46 | 28/10/2019 | 2 | 4 |
| 29/08/2019 | 25 | 50 | 28/09/2019 | 25 | 20 | 29/10/2019 | 5 | 10 |
| 30/08/2019 | 00 | 16 | 20 days of haul in September | ber | | 30/10/2019 | 21 | 42 |
| 21 days of haul in August | ıst | | 24 permitted haul days in September | September | | 31/10/2019 | 38 | 9/ |
| 26 permitted haul days in August | in August | | | | | 22 days of haul in October | | |

3.51

2.68

1.34

Season Average

122

Number of Days Hauled in 2019 Season

2.03

1.01

November December

2.71

1.36

September October

1.53

August

2.80

1.40

0.86

June July

Avg. Trucks per Hour (Days Hauled)

2019 Haul Summary

One-Way Two-Way

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Copithorne Pit 2019 Truck Haul Summary

| | One-Way Two-Way | Two-Way | | One-Way | One-Way Two-Way |
|-------------------|-----------------|---------|------------------------------------|------------|-----------------|
| Month of November | Trips | Trips | Month of December | Trips | Trips |
| 01/11/2019 | 22 | 44 | 02/12/2019 | 45 | 96 |
| 02/11/2019 | 16 | 32 | 03/12/2019 | 16 | 32 |
| 04/11/2019 | က | 9 | 04/12/2019 | 18 | 36 |
| 05/11/2019 | 7 | 14 | 05/12/2019 | ⊣ | 2 |
| 06/11/2019 | 7 | 14 | 07/12/2019 | 31 | 62 |
| 07/11/2019 | 38 | 92 | 09/08/2019 | က | 9 |
| 08/11/2019 | 2 | 10 | 10/12/2019 | 12 | 24 |
| 12/11/2019 | 6 | 18 | 11/12/2019 | 25 | 20 |
| 13/11/2019 | 16 | 32 | 12/12/2019 | 7 | 14 |
| 14/11/2019 | 13 | 26 | 9 days of haul in December | er | |
| 15/11/2019 | 12 | 24 | 26 permitted haul days in December | n December | |
| 18/11/2019 | 7 | 14 | | | |
| 21/11/2019 | 20 | 40 | | | |
| 22/11/2019 | 4 | ∞ | | | |
| 25/11/2019 | 44 | 88 | | | |
| 26/11/2019 | 24 | 48 | | | |
| 27/11/2019 | 17 | 34 | | | |
| 28/11/2019 | 17 | 34 | | | |
| 29/11/2019 | 56 | 52 | | | |
| 30/11/2019 | 17 | 34 | | | |

* no pit activity from January to April, 2019.

26 permitted haul days in November

20 days of haul in November



APPENDIX D

Synchro Output Summaries

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2020 Background

| | • | • | 4 | 1 | ļ | 1 |
|---------------------------------|------|------|-------|--------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Y | | | स | ĵ» | |
| Traffic Volume (veh/h) | 5 | 5 | 5 | 625 | 447 | 9 |
| Future Volume (Veh/h) | 5 | 5 | 5 | 625 | 447 | 9 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 5 | 5 | 672 | 481 | 10 |
| Pedestrians | | | | | - | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 140116 | 140116 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1168 | 486 | 491 | | | |
| | 1100 | 400 | 431 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | 4400 | 400 | 404 | | | |
| vCu, unblocked vol | 1168 | 486 | 491 | | | |
| tC, single (s) | 6.4 | 6.8 | 4.5 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.8 | 2.6 | | | |
| p0 queue free % | 98 | 99 | 99 | | | |
| cM capacity (veh/h) | 210 | 480 | 902 | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 10 | 677 | 491 | | | |
| Volume Left | 5 | 5 | 0 | | | |
| Volume Right | 5 | 0 | 10 | | | |
| cSH | 292 | 902 | 1700 | | | |
| Volume to Capacity | 0.03 | 0.01 | 0.29 | | | |
| Queue Length 95th (m) | 0.8 | 0.1 | 0.0 | | | |
| Control Delay (s) | 17.8 | 0.1 | 0.0 | | | |
| Lane LOS | C | A | | | | |
| Approach Delay (s) | 17.8 | 0.1 | 0.0 | | | |
| Approach LOS | C | 3.1 | 0.0 | | | |
| | Ü | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.2 | | | |
| Intersection Capacity Utilizati | ion | | 47.9% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2020 Background

| | • | * | 1 | † | 1 | 1 |
|-------------------------------|-------|------|-------|----------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 4 | B | |
| Traffic Volume (veh/h) | 6 | 8 | 5 | 476 | 599 | 5 |
| Future Volume (Veh/h) | 6 | 8 | 5 | 476 | 599 | 5 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 6 | 9 | 5 | 512 | 644 | 5 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 110110 | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1168 | 646 | 649 | | | |
| vC1, stage 1 conf vol | 1100 | 010 | 070 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1168 | 646 | 649 | | | |
| tC, single (s) | 6.4 | 6.6 | 4.3 | | | |
| tC, 2 stage (s) | 0,1 | 0.0 | 1.0 | | | |
| tF(s) | 3.5 | 3.6 | 2.4 | | | |
| p0 queue free % | 97 | 98 | 99 | | | |
| cM capacity (veh/h) | 209 | 415 | 857 | | | |
| | | | | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 15 | 517 | 649 | | | |
| Volume Left | 6 | 5 | 0 | | | |
| Volume Right | 9 | 0 | 5 | | | |
| cSH | 298 | 857 | 1700 | | | |
| Volume to Capacity | 0.05 | 0.01 | 0.38 | | | |
| Queue Length 95th (m) | 1.3 | 0.1 | 0.0 | | | |
| Control Delay (s) | 17.7 | 0.2 | 0.0 | | | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 17.7 | 0.2 | 0.0 | | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.3 | | | |
| Intersection Capacity Utiliza | ation | | 42.7% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |
| Alialysis Fellou (IIIIII) | | | 13 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2040 Background

| | ۶ | 7 | 4 | 1 | Į. | 1 |
|-------------------------------|--------|------|-------|--------|------------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 4 | 1 > | |
| Traffic Volume (veh/h) | 5 | 5 | 5 | 810 | 579 | 11 |
| Future Volume (Veh/h) | 5 | 5 | 5 | 810 | 579 | 11 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 5 | 5 | 871 | 623 | 12 |
| Pedestrians | III TX | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 140110 | 110110 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1510 | 629 | 635 | | | |
| vC1, stage 1 conf vol | 1510 | 023 | 000 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1510 | 629 | 635 | | | |
| tC, single (s) | 6.4 | 6.8 | 4.5 | | | |
| | 0.4 | 0.0 | 4.5 | | | |
| tC, 2 stage (s) | 3.5 | 3.8 | 2.6 | | | |
| tF (s) | 96 | 99 | 99 | | | |
| p0 queue free % | | 392 | | | | |
| cM capacity (veh/h) | 130 | | 790 | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 10 | 876 | 635 | | | |
| Volume Left | 5 | 5 | 0 | | | |
| Volume Right | 5 | 0 | 12 | | | |
| cSH | 195 | 790 | 1700 | | | |
| Volume to Capacity | 0.05 | 0.01 | 0.37 | | | |
| Queue Length 95th (m) | 1.3 | 0.2 | 0.0 | | | |
| Control Delay (s) | 24.5 | 0.2 | 0.0 | | | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 24.5 | 0.2 | 0.0 | | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.3 | | | |
| Intersection Capacity Utiliza | ation | | 57.9% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2040 Background

| | J | * | 4 | † | ļ | 1 |
|--------------------------------|-----------|------|-------|------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 4 | B | |
| Traffic Volume (veh/h) | 7 | 9 | 5 | 617 | 776 | 5 |
| Future Volume (Veh/h) | 7 | 9 | 5 | 617 | 776 | 5 |
| Sign Control | Stop | 4.5 | | Free | Free | 8 I |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 8 | 10 | 5 | 663 | 834 | 5 |
| Pedestrians | | | | 000 | 001 | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | None | None | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | 4540 | 000 | 000 | | | |
| vC, conflicting volume | 1510 | 836 | 839 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1510 | 836 | 839 | | | |
| tC, single (s) | 6.4 | 6.5 | 4.3 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.6 | 2.4 | | | |
| p0 queue free % | 94 | 97 | 99 | | | |
| cM capacity (veh/h) | 130 | 324 | 723 | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | 35-1 | | |
| Volume Total | 18 | 668 | 839 | | | |
| Volume Left | 8 | 5 | 0 | | | |
| Volume Right | 10 | 0 | 5 | | | |
| cSH | 194 | 723 | 1700 | | | |
| Volume to Capacity | 0.09 | 0.01 | 0.49 | | | |
| Queue Length 95th (m) | 2.4 | 0.2 | 0.0 | | | |
| Control Delay (s) | 25.4 | 0.2 | 0.0 | | | |
| Lane LOS | D | A | 0.0 | | | |
| Approach Delay (s) | 25.4 | 0.2 | 0.0 | | | |
| Approach LOS | 23.4 D | 0.2 | 0.0 | | | |
| //** | U | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | 1, 1 | | 0.4 | | | |
| Intersection Capacity Utilizat | tion | | 52.3% | K | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2020 Background - Twinned Hwy 22

| | <i>></i> | * | 4 | 1 | 1 | 1 |
|-------------------------------|-------------|------|-------|-------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 44 | 44 | |
| Traffic Volume (veh/h) | 5 | 5 | 5 | 810 | 579 | 11 |
| Future Volume (Veh/h) | 5 | 5 | 5 | 810 | 579 | 11 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 5 | 5 | 871 | 623 | 12 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1074 | 318 | 635 | | | |
| vC1, stage 1 conf vol | | | - | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1074 | 318 | 635 | | | |
| tC, single (s) | 6.9 | 8.1 | 4.9 | | | |
| tC, 2 stage (s) | | - 1 | 110 | | | |
| tF (s) | 3.5 | 3.9 | 2.6 | | | |
| p0 queue free % | 98 | 99 | 99 | | | |
| cM capacity (veh/h) | 208 | 534 | 727 | | | |
| | | | | 75.00 | AV 8 0 | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | \$8.1 | SB 2 | |
| Volume Total | 10 | 295 | 581 | 415 | 220 | |
| Volume Left | 5 | 5 | 0 | 0 | 0 | |
| Volume Right | 5 | 0 | 0 | 0 | 12 | |
| cSH | 300 | 727 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.03 | 0.01 | 0.34 | 0.24 | 0.13 | |
| Queue Length 95th (m) | 0.8 | 0.2 | 0.0 | 0.0 | 0.0 | |
| Control Delay (s) | 17.4 | 0.3 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 17.4 | 0.1 | | 0.0 | | |
| Approach LOS | С | | | | | |
| Intersection Summary | | | | | | 30,00 |
| Average Delay | | | 0.2 | | | |
| Intersection Capacity Utiliza | tion | | 36.6% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2040 Background - Twinned Hwy 22

| | 1 | ¥ | 4 | † | Ţ | 1 |
|-------------------------------|-------|------|-------|----------|-------------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 41 | 1 13 | |
| Traffic Volume (veh/h) | 7 | 9 | 5 | 617 | 776 | 5 |
| Future Volume (Veh/h) | 7 | 9 | 5 | 617 | 776 | 5 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 8 | 10 | 5 | 663 | 834 | 5 |
| Pedestrians | | | | | - | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| | | | | INOLIG | HOLLE | |
| Median storage veh) | | | | | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | 4470 | 400 | 000 | | | |
| vC, conflicting volume | 1178 | 420 | 839 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | 4480 | 100 | 000 | | | |
| vCu, unblocked vol | 1178 | 420 | 839 | | | |
| tC, single (s) | 6.9 | 7.6 | 4.5 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.6 | 2.4 | | | |
| p0 queue free % | 96 | 98 | 99 | | | |
| cM capacity (veh/h) | 178 | 504 | 686 | | | |
| Direction, Lane# | EB 1 | NB 1 | NB 2 | SB 1 | SB 2 | تجلسك |
| Volume Total | 18 | 226 | 442 | 556 | 283 | |
| Volume Left | 8 | 5 | 0 | 0 | 0 | |
| Volume Right | 10 | 0 | 0 | 0 | 5 | |
| cSH | 278 | 686 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.06 | 0.01 | 0.26 | 0.33 | 0.17 | |
| Queue Length 95th (m) | 1.7 | 0.2 | 0.0 | 0.0 | 0.0 | |
| Control Delay (s) | 18.9 | 0.3 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | C | A | 0.0 | 0.0 | 0.0 | |
| Approach Delay (s) | 18.9 | 0.1 | | 0.0 | | |
| Approach LOS | C | 0.1 | | 0.0 | | |
| | U | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.3 | | | |
| Intersection Capacity Utiliza | ition | | 32.2% | 10 | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2020 Post Development

| | Þ | * | 4 | † | ļ | 1 |
|-------------------------------|------|------|-------|----------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | स | 13 | |
| Traffic Volume (veh/h) | 5 | 8 | 7 | 626 | 447 | 13 |
| Future Volume (Veh/h) | 5 | 8 | 7 | 626 | 447 | 13 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 9 | 8 | 673 | 481 | 14 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1177 | 488 | 495 | | | |
| vC1, stage 1 conf vol | | 100 | 100 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1177 | 488 | 495 | | | |
| tC, single (s) | 6.4 | 7.0 | 4.6 | | | |
| tC, 2 stage (s) | V,-T | 1.0 | 7.0 | | | |
| tF (s) | 3.5 | 4.1 | 2.7 | | | |
| p0 queue free % | 98 | 98 | 99 | | | |
| cM capacity (veh/h) | 206 | 443 | 861 | | | |
| | | | | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 14 | 681 | 495 | | | |
| Volume Left | 5 | 8 | 0 | | | |
| Volume Right | 9 | 0 | 14 | | | |
| cSH | 314 | 861 | 1700 | | | |
| Volume to Capacity | 0.04 | 0.01 | 0.29 | | | |
| Queue Length 95th (m) | 1.1 | 0.2 | 0.0 | | | |
| Control Delay (s) | 17.0 | 0.2 | 0.0 | | | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 17.0 | 0.2 | 0.0 | | | |
| Approach LOS | С | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.3 | | | |
| Intersection Capacity Utiliza | tion | | 49.6% | 10 | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2020 Post Development

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|-------------------------------|-------|------|-------|--------|----------|------------|
| Movement | EBL | EBR | NBL | NB; | SBT | SBR |
| Lane Configurations | W | | | बी | 1> | |
| Traffic Volume (veh/h) | 7 | 14 | 5 | 477 | 599 | 4 |
| Future Volume (Veh/h) | 7 | 14 | 5 | 477 | 599 | 4 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 8 | 15 | 5 | 513 | 644 | 4 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 110110 | 110110 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1169 | 646 | 648 | | | |
| vC1, stage 1 conf vol | 1100 | 0-10 | 0.10 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1169 | 646 | 648 | | | |
| tC, single (s) | 6.4 | 6.7 | 4.5 | | | |
| tC, 2 stage (s) | 0.7 | 0.7 | 4.0 | | | |
| tF (s) | 3.5 | 3.8 | 2.6 | | | |
| p0 queue free % | 96 | 96 | 99 | | | |
| cM capacity (veh/h) | 209 | 396 | 781 | | | |
| | | | | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 23 | 518 | 648 | | | |
| Volume Left | 8 | 5 | 0 | | | |
| Volume Right | 15 | 0 | 4 | | | |
| cSH | 302 | 781 | 1700 | | | |
| Volume to Capacity | 80.0 | 0.01 | 0.38 | | | |
| Queue Length 95th (m) | 2.0 | 0.2 | 0.0 | | | |
| Control Delay (s) | 17.9 | 0.2 | 0.0 | | | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 17.9 | 0.2 | 0.0 | | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | | | تنفيل | | |
| Average Delay | | | 0.4 | | | |
| Intersection Capacity Utiliza | ation | | 42.6% | 10 | CU Level | of Service |
| Analysis Period (min) | www. | | 15 | | | |
| mayor renou (IIIII) | | | 10 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2040 Post Development

| | A | 1 | 4 | 1 | Ţ | 4 |
|------------------------------|-------|------|-------|--------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | बी | P | |
| Traffic Volume (veh/h) | 5 | 9 | 7 | 811 | 579 | 15 |
| Future Volume (Veh/h) | 5 | 9 | 7 | 811 | 579 | 15 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 10 | 8 | 872 | 623 | 16 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 110110 | 110110 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1519 | 631 | 639 | | | |
| vC1, stage 1 conf vol | 1010 | 001 | 000 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1519 | 631 | 639 | | | |
| tC, single (s) | 6.4 | 7.0 | 4.6 | | | |
| | 0.4 | 7.0 | 4.0 | | | |
| tC, 2 stage (s) tF (s) | 3.5 | 4.0 | 2.7 | | | |
| | 96 | 97 | 99 | | | |
| p0 queue free % | | 372 | | | | |
| cM capacity (veh/h) | 127 | | 753 | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | | | |
| Volume Total | 15 | 880 | 639 | | | |
| Volume Left | 5 | 8 | 0 | | | |
| Volume Right | 10 | 0 | 16 | | | |
| cSH | 227 | 753 | 1700 | | | |
| Volume to Capacity | 0.07 | 0.01 | 0.38 | | | |
| Queue Length 95th (m) | 1.7 | 0.3 | 0.0 | | | |
| Control Delay (s) | 22.0 | 0.3 | 0.0 | | | |
| Lane LOS | C | A | | | | |
| Approach Delay (s) | 22.0 | 0.3 | 0.0 | | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.4 | - | | |
| Intersection Capacity Utiliz | ation | | 59.6% | 16 | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |
| ruidiyala i cilod (itilil) | | | 10 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2040 Post Development

| | 1 | 1 | - | † | \ | 4 |
|-------------------------------|----------|------|-----------|----------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |)\dagger | | | स | ĵ. | |
| Traffic Volume (veh/h) | 8 | 15 | 6 | 618 | 776 | 5 |
| Future Volume (Veh/h) | 8 | 15 | 6 | 618 | 776 | 5 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 9 | 16 | 6 | 665 | 834 | 5 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | . 13110 | . 15110 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1514 | 836 | 839 | | | |
| vC1, stage 1 conf vol | 1014 | 030 | 000 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1514 | 836 | 839 | | | |
| | 6.4 | 6.7 | 4.5 | | | |
| tC, single (s) | 0.4 | 0.7 | 4.5 | | | |
| tC, 2 stage (s) | 0.5 | 2.7 | 0.0 | | | |
| tF (s) | 3.5 | 3.7 | 2.6 99 | | | |
| p0 queue free % | 93 | 95 | | | | |
| cM capacity (veh/h) | 129 | 308 | 654 | | | |
| Direction, Lane # | EB 1 | NB 1 | SB 1 | واريب | | |
| Volume Total | 25 | 671 | 839 | | | |
| Volume Left | 9 | 6 | 0 | | | |
| Volume Right | 16 | 0 | 5 | | | |
| cSH | 205 | 654 | 1700 | | | |
| Volume to Capacity | 0.12 | 0.01 | 0.49 | | | |
| Queue Length 95th (m) | 3.3 | 0.2 | 0.0 | | | |
| Control Delay (s) | 25.0 | 0.3 | 0.0 | | | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 25.0 | 0.3 | 0.0 | | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | | | | | التهولا |
| Average Delay | | | 0.5 | | | |
| Intersection Capacity Utiliza | ation | | 52.3% | 10 | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |
| , maryoto i oriod (mmi) | | | | | | |

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1: Highway 22 & Township Road 242 08-26-2020

AM Peak Hour 2040 Post Development - Twinned Hwy 22

| | • | * | 1 | Ť | Ţ | 1 |
|-------------------------------|----------|------|-------|------|----------|------------|
| Movement | EBL | EBR | NEL | NBT | SBT | SBR |
| Lane Configurations | W | | | 44 | 47> | |
| Traffic Volume (veh/h) | 5 | 9 | 7 | 811 | 579 | 15 |
| Future Volume (Veh/h) | 5 | 9 | 7 | 811 | 579 | 15 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 5 | 10 | 8 | 872 | 623 | 16 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1083 | 320 | 639 | | | |
| vC1, stage 1 conf vol | 1000 | 020 | 000 | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1083 | 320 | 639 | | | |
| tC, single (s) | 6.9 | 8.4 | 5.1 | | | |
| tC, 2 stage (s) | 0.0 | 0.1 | 0,1 | | | |
| tF (s) | 3.5 | 4.0 | 2.7 | | | |
| p0 queue free % | 98 | 98 | 99 | | | |
| cM capacity (veh/h) | 205 | 502 | 679 | | | |
| | | | | 05 4 | OD O | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | SB 2 | |
| Volume Total | 15 | 299 | 581 | 415 | 224 | |
| Volume Left | 5 | 8 | 0 | 0 | 0 | |
| Volume Right | 10 | 0 | 0 | 0 | 16 | |
| cSH | 338 | 679 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.04 | 0.01 | 0.34 | 0.24 | 0.13 | |
| Queue Length 95th (m) | 1.1 | 0.3 | 0.0 | 0.0 | 0.0 | |
| Control Delay (s) | 16.1 | 0.4 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | С | Α | | | | |
| Approach Delay (s) | 16.1 | 0.1 | | 0.0 | | |
| Approach LOS | C | | | | | |
| Intersection Summary | te jejan | | | | | Barrell I |
| Average Delay | | | 0.2 | | | |
| Intersection Capacity Utiliza | ation | | 38.1% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |
| raidiyolo i chica (ililii) | | | 10 | | | |

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1: Highway 22 & Township Road 242 08-26-2020

PM Peak Hour 2040 Post Development - Twinned Hwy 22

| | Þ | * | 4 | † | 1 | 1 |
|------------------------------|-------|--------|-------|--------|----------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | W | | | 44 | 44 | |
| Traffic Volume (veh/h) | 8 | 15 | 6 | 618 | 776 | 5 |
| Future Volume (Veh/h) | 8 | 15 | 6 | 618 | 776 | 5 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Hourly flow rate (vph) | 9 | 16 | 6 | 665 | 834 | 5 |
| Pedestrians | | | | | | |
| Lane Width (m) | | | | | | |
| Walking Speed (m/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | 140110 | 110110 | |
| Upstream signal (m) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1181 | 420 | 839 | | | |
| vC1, stage 1 conf vol | 1101 | 420 | 009 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vCu, unblocked vol | 1181 | 420 | 839 | | | |
| | 6.9 | 7.8 | 4.9 | | | |
| tC, single (s) | 0.9 | 7.0 | 4.9 | | | |
| tC, 2 stage (s) | 2.5 | 2.0 | 0.0 | | | |
| tF(s) | 3.5 | 3.8 | 2.6 | | | |
| p0 queue free % | 95 | 97 | 99 | | | |
| cM capacity (veh/h) | 177 | 475 | 589 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | SB 2 | |
| Volume Total | 25 | 228 | 443 | 556 | 283 | |
| Volume Left | 9 | 6 | 0 | 0 | 0 | |
| Volume Right | 16 | 0 | 0 | 0 | 5 | |
| cSH | 296 | 589 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.08 | 0.01 | 0.26 | 0.33 | 0.17 | |
| Queue Length 95th (m) | 2.2 | 0.2 | 0.0 | 0.0 | 0.0 | |
| Control Delay (s) | 18.3 | 0.4 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | C | Α | | | | |
| Approach Delay (s) | 18.3 | 0.1 | | 0.0 | | |
| Approach LOS | C | | | | | |
| Intersection Summary | | 3 1 | | | | |
| Average Delay | | - 2.71 | 0.4 | | - 1 | |
| Intersection Capacity Utiliz | ation | | 32.2% | IC | CU Level | of Service |
| Analysis Period (min) | | | 15 | | | |
| raidiyələ i Gilou (illili) | | | 10 | | | |



APPENDIX E

Illumination Warrants

Illumination of Isolated Rural Intersections LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with Illumination of Isolated Rural Intersections, Transportation Association of Canada, February 2001.

Please enter information in the cells with yellow background

| INTERSECTION CHARACTERISTIC | S |
|-----------------------------|------------|
| Highway 22 | Main Road |
| Township Road 242 | Minor Road |
| Rocky View County | City/Town |

Other June 2, 2020
Horizon: Long

Horizon: Long Term Background

| GEOMETRIC FACTORS | | | | | | |
|--|-------------|--------|--------|---|--------------|-------|
| | Value | Rating | Weight | | Check | Score |
| Channelization Rating | Descriptive | 0 | | Refer to Table 1(A) to determine rating value | OK | |
| Presence of raised channelization? (Y/N) | n | | | | OK | |
| lighest operating speed on raised, channelized approach (km/h) | 0 | | 5 | | OK | |
| Channelization Factor | | | | | OK | 0 |
| Approach Sight Distance on most constrained approach (%) | 100 | 0 | 10 | Relative to the recommended minimum sight distance | ОК | 0 |
| Posted Speed limit (in 10's of km/h) | 100 | | | | ок | |
| Radius of Horizontal Curve (m) | T | | | Enter "T" for tangent (no horizontal curve at the intersection) | OK | |
| Posted Speed Category = | | 0 | | | | |
| Posted Speed Category = | В | 0 | | | | |
| Posted Speed Category = | | 0 | | | | |
| Posted Speed Category = | | 0 | | | | |
| Horizontal Curvature Factor | | 0 | 5 | | OK | 0 |
| Angle of Intersection (10's of Degrees) | 90 | 0 | 5 | | ОК | 0 |
| Downhill Approach Grade (x.x%) | 0.0 | 0 | 3 | Rounded to nearest tenth of a percent | ОК | 0 |
| Number of Intersection Legs | 4 | 2 | 3 | Number of legs = 3 or more | ОК | 6 |
| | | | | Geometric Facto | ors Subtotal | 6 |

| OPERATIONAL FACTORS | | | | | | |
|---|------------------------------|-------------|----------------|---|-----------|--------------|
| Is the intersection signalized ?(Y/ N) | N | | | Calculate the Signalization Warrant Factor | | |
| AADT on Major Road (2-way) AADT on Minor Road (2-way) Signalization Warrant | 12,200 115 Descriptive | 4 0 0 | 10 20 30 | Either Use the two AADT inputs OR the Descriptive Signalization Warrant (Unused values should be set to Zero) Refer to Table 1(B) for description and rating values for signalization warrant. | OK OK | 40 0 0 |
| Night-Time Hourly Pedestrian Volume | 0 | 0 | 10 | Refer to Table 1(B), note #2, to account for children and seniors | ок | 0 |
| Intersecting Roadway Classification | Descriptive | 1 | 5 | Refer to Table 1(B) for ratings. | ОК | 5 |
| Operating Speed or Posted Speed on Major Road (km/h) | 110 | 4 | 5 | Refer to Table 1(B), note #3 | ок | 20 |
| Operating Speed on Minor Road (km/h) | 60 | 1 | 5 | Refer to Table 1(B), note #3 | ОК | 5 |
| | | | | Operational Factor | s Subtota | I 70 |

| ENVIRONMENTAL FACTOR | | | | | | | |
|--|---|---|---|------------------------|----------------------|----------|---|
| Lighted Developments within 150 m radius of intersection | 0 | 0 | 5 | Maximum of 4 quadrants | | ок | 0 |
| | | | | | Environmental Factor | Subtotal | 0 |

| COLLISION HISTORY | | | N/I | | | | |
|---|-----|---|-----|--|--------------|----|---|
| Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole #) OR | 0.0 | 0 | 0 | Enter either the annual frequency (See Table 1(C), note #4) OR the number of collisions / MEV | ок | | 0 |
| Collision Rate over last 3 years, due to inadequate lighting (/MEV) | 0 | 0 | 0 | (Unused values should be set to Zero) | ок | | 0 |
| Is the average ratio of all night to day collisions >= 1.5 (Y/N) | n | 0 | | | OK | ок | |
| | | | | Collision History | ory Subtotal | | 0 |

Check Intersection Signalization: Intersection is not Signalized

LIGHTING IS NOT WARRANTED

| SUMMARY | |
|---------------------------------------|----|
| Geometric Factors Subtotal | 6 |
| Operational Factor Subtotal | 70 |
| Environmental Factor Subtotal | 0 |
| Collision History Subtotal | 0 |
| · · · · · · · · · · · · · · · · · · · | |
| TOTAL POINTS | 76 |

Illumination of Isolated Rural Intersections LIGHTING WARRANT SPREADSHEET

This spreadsheet is to be used in conjunction with Illumination of Isolated Rural Intersections, Transportation Association of Canada, February 2001.

| Please enter information in the cells with vellow backgroun | Please | e enter inform | ation in the | cells with vello | w background |
|---|--------|----------------|--------------|------------------|--------------|
|---|--------|----------------|--------------|------------------|--------------|

| Main Road |
|------------|
| Minor Road |
| City/Town |
| |

Date Other June 2, 2020
Horizon: Long Term Post Development

| GEOMETRIC FACTORS | | | | | | |
|--|-------------|--------|--------|---|--------------|-------|
| | Value | Rating | Weight | | Check | Score |
| Channelization Rating | Descriptive | 0 | | Refer to Table 1(A) to determine rating value | OK | |
| Presence of raised channelization? (Y/N) | n | | _ | | OK | |
| Highest operating speed on raised, channelized approach (km/h) | 0 | | 5 | | OK | |
| Channelization Factor | | | | | ок | 0 |
| Approach Sight Distance on most constrained approach (%) | 100 | 0 | 10 | Relative to the recommended minimum sight distance | ОК | 0 |
| Posted Speed limit (in 10's of km/h) | 100 | | | | ОК | |
| Radius of Horizontal Curve (m) | T | | | Enter "T" for tangent (no horizontal curve at the intersection) | OK | |
| Posted Speed Category = | | 0 | | - ' | | |
| Posted Speed Category = | В | 0 | | | | |
| Posted Speed Category = | | 0 | | | | |
| Posted Speed Category = | | 0 | | | | |
| Horizontal Curvature Factor | | 0 | 5 | | ок | 0 |
| Angle of Intersection (10's of Degrees) | 90 | 0 | 5 | | ок | 0 |
| | | | | | | |
| Downhill Approach Grade (x.x%) | 0.0 | 0 | 3 | Rounded to nearest tenth of a percent | OK | 0 |
| Number of Intersection Legs | 4 | 2 | 3 | Number of legs = 3 or more | ок | 6 |
| | | | | Geometric Fact | ors Subtotal | 6 |

| OPERATIONAL FACTORS | | E1 19 | | | 125 | |
|---|------------------------------|-------------|----------------|---|----------------|-------------------|
| s the intersection signalized ? (Y/ N) | N | | | Calculate the Signalization Warrant Factor | | |
| AADT on Major Road (2-way) AADT on Minor Road (2-way) Signalization Warrant | 12,300 210 Descriptive | 4 0 0 | 10 20 30 | Either Use the two AADT inputs OR the Descriptive Signalization Warrant (Unused values should be set to Zero) Refer to Table 1(B) for description and rating values for signalization warrant. | OK OK OK | 40 0 0 0 |
| light-Time Hourly Pedestrian Volume | 0 | 0 | 10 | Refer to Table 1(B), note #2, to account for children and seniors | ок | 0 |
| ntersecting Roadway Classification | Descriptive | 1 | 5 | Refer to Table 1(B) for ratings. | ок | 5 |
| Operating Speed or Posted Speed on Major Road (km/h) | 110 | 4 | 5 | Refer to Table 1(B), note #3 | ок | 20 |
| Operating Speed on Minor Road (km/h) | 60 | 1 | 5 | Refer to Table 1(B), note #3 | ок | 5 |
| | | | | Operational Factors | s Subtotal | 70 |

| ENVIRONMENTAL FACTOR | | | | | | |
|--|---|---|---|------------------------|-------------------------------|---|
| Lighted Developments within 150 m radius of intersection | 0 | 0 | 5 | Maximum of 4 quadrants | ОК | 0 |
| | | | | | Environmental Factor Subtotal | 0 |

| COLLISION HISTORY | | | | | | |
|--|-----|---|---|--|-----|---|
| Average Annual night-time collision frequency due to inadequate lighting (collisions/yr, rounded to nearest whole #) | 0.0 | 0 | 0 | Enter either the annual frequency (See Table 1(C), note #4) | | 0 |
| OR Collision Rate over last 3 years, due to inadequate lighting (/MEV) | 0 | n | 0 | OR the number of collisions / MEV (Unused values should be set to Zero) OK | | 0 |
| Is the average ratio of all night to day collisions >= 1.5 (Y/N) | n | ō | Ü | OK OK | | Ü |
| | | | | | OK | |
| | | | | Collision History Subto | tal | 0 |

Check Intersection Signalization: Intersection is not Signalized

LIGHTING IS NOT WARRANTED

| SUMMARY | |
|-------------------------------|----|
| Geometric Factors Subtotal | 6 |
| Operational Factor Subtotal | 70 |
| Environmental Factor Subtotal | 0 |
| Collision History Subtotal | 0 |
| TOTAL POINTS | 76 |